

TECHNICAL APPENDIX

APPENDIX A

MEMORANDUM OF UNDERSTANDING

May 21, 2019

Danny Johnson, AICP
Town of Rolesville
502 Southtown Circle
Rolesville, NC 27571
919.554.6517
danny.johnson@rolesville.nc.gov

Reference: Wheeler Tract
Rolesville, North Carolina

Subject: Memorandum of Understanding for TIA Report

Dear Mr. Johnson:

The following is a Memorandum of Understanding (MOU) outlining the proposed scope of work and assumptions related to the Traffic Impact Analysis (TIA) for the proposed Wheeler Tract residential development, to be along Rolesville Road, north of Mitchell Mill Road in Rolesville, North Carolina. This MOU reflects the assumptions outlined during initial coordination with Ramey Kemp & Associates (RKA), the Town of Rolesville (Town), and the North Carolina Department of Transportation (NCDOT). Refer to the attached site location map. Site access will be provided via one full movement driveway on Rolesville Road, one full movement connection on Mitchell Mill Road, and one connection to the existing subdivision along Taviswood Way. The proposed site, anticipated to be completed in 2026, is expected to consist of approximately 233 single family homes and 125 townhomes.

Study Area

Based on coordination with the Town and the NCDOT, the study area is proposed to consist of the following intersections:

- Mitchell Mill Road and Rolesville Road
- Rolesville Road and Fowler Road
- Rolesville Road and Taviswood Way

Existing Traffic Volumes and Traffic Signal Data

Peak hour turning movement counts were conducted by Ramey Kemp & Associates, Inc. at the existing study intersections in May of 2019 during weekday AM (7:00 to 9:00) and weekday PM (4:00 to 6:00) peak hours. Refer to the attached existing (2019) traffic volumes figure.

Background Traffic Volumes

Based on coordination with the Town and NCDOT, background traffic volumes will be determined by projecting existing (2019) traffic volumes to the year 2026 using a 2% annual growth rate and adding the traffic associated with the following two adjacent developments:

- East Young Street PUD (The Point)
- Kalas Property
- Watkins Family Property

Future Roadway Improvements

Based on coordination with the Town and NCDOT, there are no future roadway improvements to consider at the study intersections.

Trip Generation

Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 10th Edition for the 265 apartment units. Refer to Table 1 for a summary of the proposed site trip generation.

Table 1: Trip Generation Summary

Land Use (ITE Code)	Intensity	Daily Traffic (vpd)	AM Peak Hour Trips (vph)		PM Peak Hour Trips (vph)	
			Enter	Exit	Enter	Exit
Single Family Detached (210)	233 units	2,300	42	128	144	85
Multifamily Housing - Townhomes (220)	125 units	900	14	45	46	27
Total		3,200	56	173	190	112

It is estimated that the proposed development will generate approximately 3,200 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 229 trips (56 entering and 173 exiting) will occur during the weekday AM peak hour and 302 (190 entering and 112 exiting) will occur during the weekday PM peak hour.

Trip Distribution and Assignment

The primary site trips are distributed based on the locations of existing traffic patterns, population centers adjacent to the study area, and engineering judgment. A summary of the overall distributions is below:

- 50% to/from the north via Rolesville Road
- 10% to/from the south via Rolesville Road
- 40% to/from the west via Mitchell Mill Road

Refer to the attached site trip distribution figure.

Analysis Scenarios

All capacity analyses will be performed utilizing Synchro (Version 10.3). All study intersections will be analyzed during the weekday AM and PM peak hours under the following proposed traffic scenarios:

- Existing (2019)
- Background (2026)
- Combined (2026)

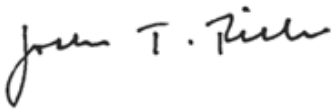
Report

The TIA report will be prepared based on the Town and NCDOT requirements.

If you find this memorandum of understanding acceptable, please let me know so that we may include it in the TIA report. If you have any questions or concerns, please do not hesitate to contact me.

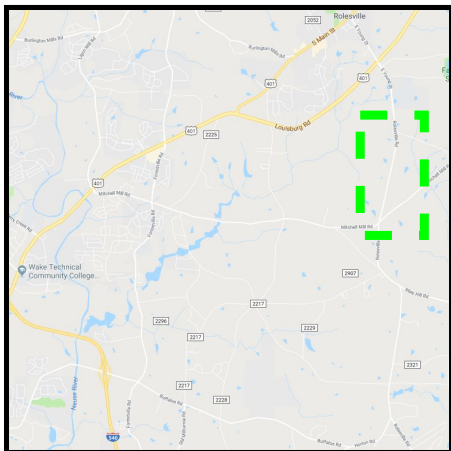
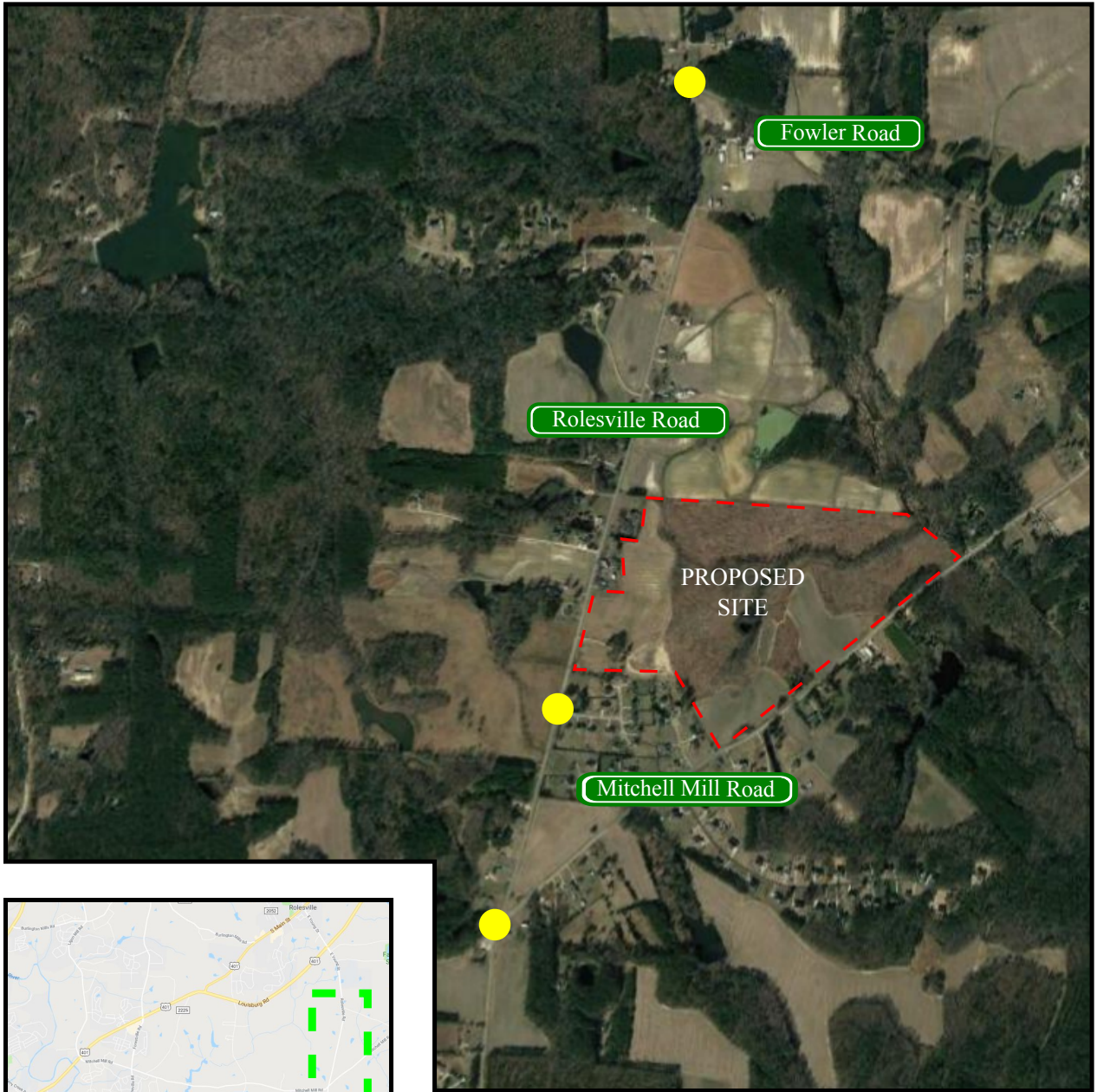
Sincerely,

Ramey Kemp & Associates, Inc.






Joshua Reinke, P.E.
Transportation Engineer

Attachments: Site Location Map
Site Plan
Existing (2019) Traffic Volumes Figure
Site Trip Distribution Figure



LEGEND

-  Proposed Site Location
-  Study Intersection
-  Study Area



Wheeler Tract
Rolesville, NC

Site Location Map

Scale: Not to Scale

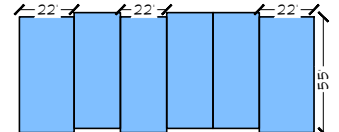
SITE DATA

PROPERTY OWNER	WHEELER
	7400 FOWLER ROAD
WAKE COUNTY PINS	1767483143, 1767586093
PROPOSED ZONING	REPUD
TRACT AREA	42.57 AC (COMPUTED)
AREA IN R/W	16.39 AC
NET TRACT AREA	76.19 AC
MINIMUM LOT WIDTH	50 FT
MINIMUM LOT SIZE	9500 SF
PROPOSED NUMBER OF LOTS	361
NUMBER OF 50' WIDE LOTS	148
NUMBER OF 60' WIDE LOTS	39
NUMBER TOWNHOMES	125
PROPOSED DENSITY	361 / 42.57 = 8.48 UN/AC
LENGTH OF 50' R/W	~ 12,000 LF

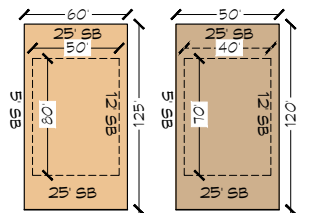


GENERAL NOTES

1. BOUNDARY IS TAKEN FROM GIS INFORMATION ON FILE WITH WAKE COUNTY.
2. TOPOGRAPHIC DATA IS TAKEN FROM GIS DATA AVAILABLE FROM WAKE COUNTY.
3. THIS PLAN IS CONCEPTUAL IN NATURE AND HAS NOT BEEN REVIEWED BY ANY AGENCY.
4. UNIT COUNT IS SUBJECT TO CHANGE PENDING WETLAND AND NEUSE BUFFER DETERMINATIONS AND PERMITTING, GRADING, SEWER AND STORMWATER DESIGN.
5. THIS SITE IS LOCATED IN THE NEUSE RIVER BASIN.
6. THIS SITE WILL REQUIRE TOWN OF ROLESVILLE ZONING. THIS LAYOUT ASSUMES REPUD.
7. FEMA DESIGNATED FLOOD ZONES ARE LOCATED ON THIS SITE.



TYPICAL LOT DIMENSIONS 'TOWNS'



TYPICAL LOT DIMENSIONS '60S'

WHEELER TRACT
 SINGLE FAMILY DETACHED (65' WIDE)
 TOWN OF ROLESVILLE NORTH CAROLINA

SCALE	1:100
DATE	11/15/2016
PROJECT NUMBER	TBC
CLIENT	HOPPER COMMUNITIES
PLAN TYPE	CONCEPT PLAN

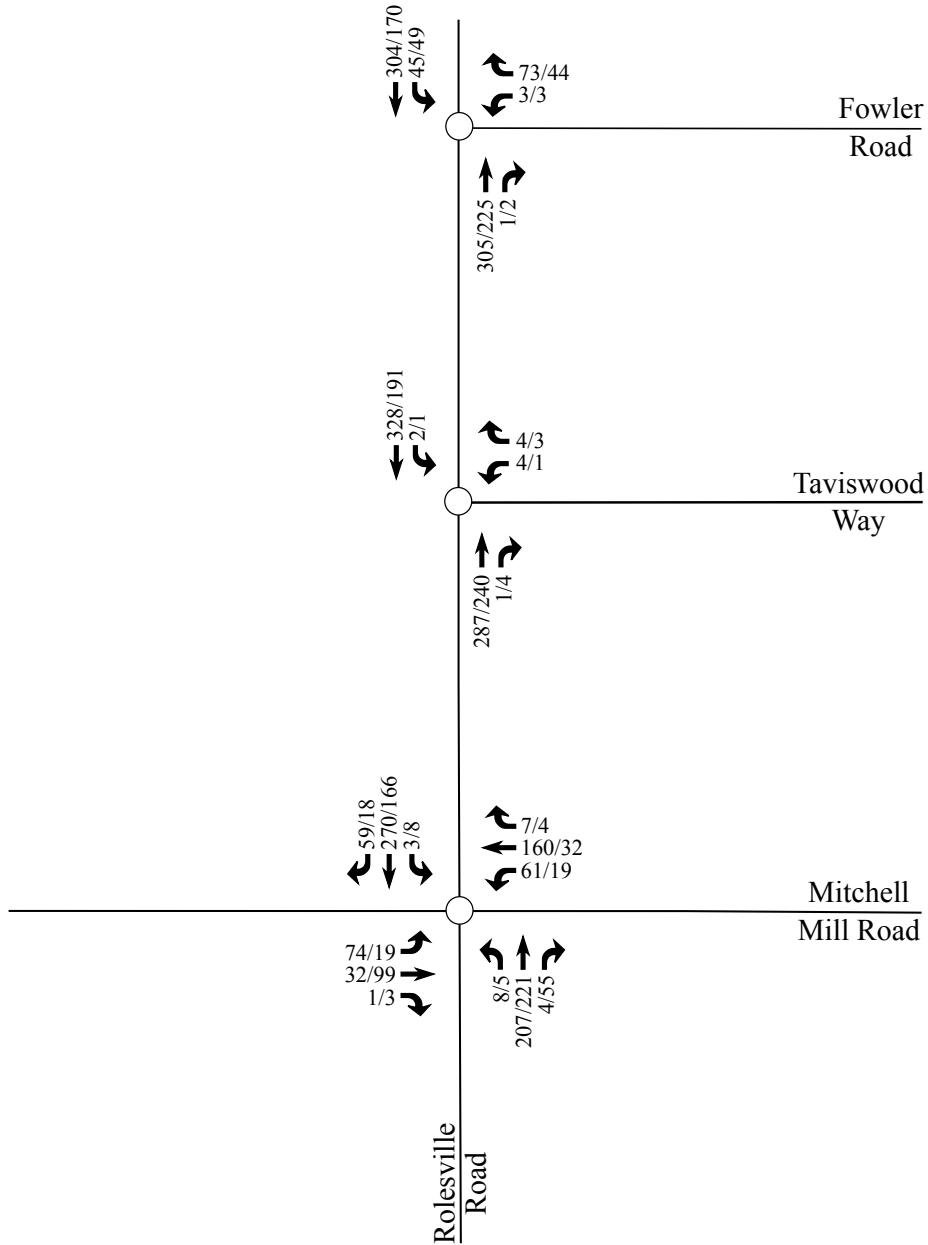
CONCEPT PLAN 3

RELEASED FOR CONSTRUCTION DATE (SEE COVER)	
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PRIEST, CRAVEN & ASSOCIATES, INC.
 LAND USE CONSULTANTS / PLANNERS / LANDSCAPE DESIGNERS / SURVEYORS / ENGINEERS
 3801-B Computer Drive, Suite 104 Raleigh, NC, 27609, Phone 919 / 781-8300, Fax 919 / 782-1288, Email PCRA@PriestCraven.com / Firm #: C0488

LEGEND

- Unsignalized Intersection
- X / Y → AM / PM Peak Hour Traffic



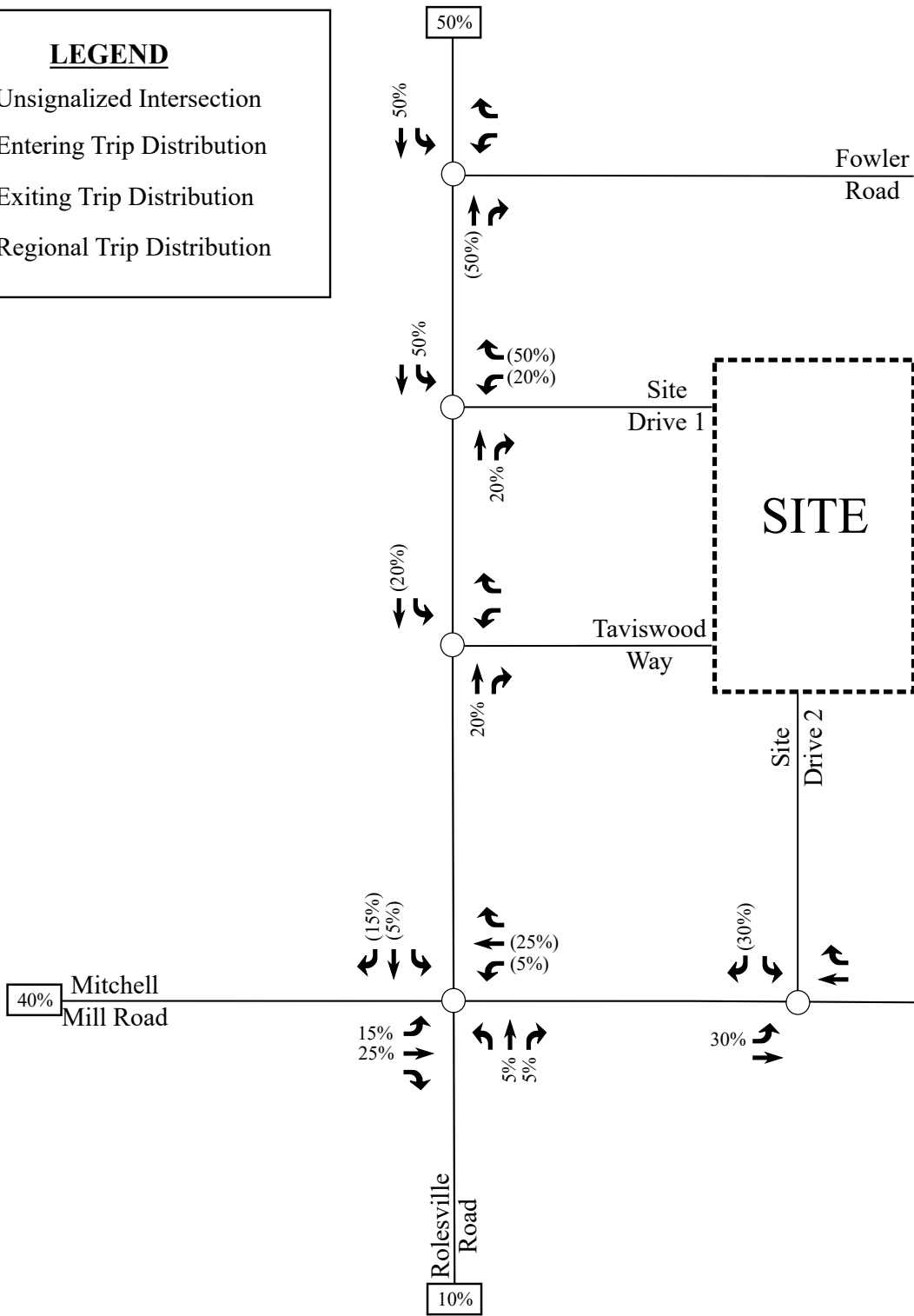
Wheeler Tract
Rolesville, NC

Existing (2019)
Peak Hour Traffic

Scale: Not to Scale

LEGEND

- Unsignalized Intersection
- X% → Entering Trip Distribution
- (Y%) → Exiting Trip Distribution
- XX% Regional Trip Distribution



Wheeler Tract
Rolesville, NC

Site Trip Distribution

Scale: Not to Scale

APPENDIX B

COUNT DATA



TRAFFIC DATA COLLECTION

File Name : Rolesville(Mitchell Mill and Rolesville) AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

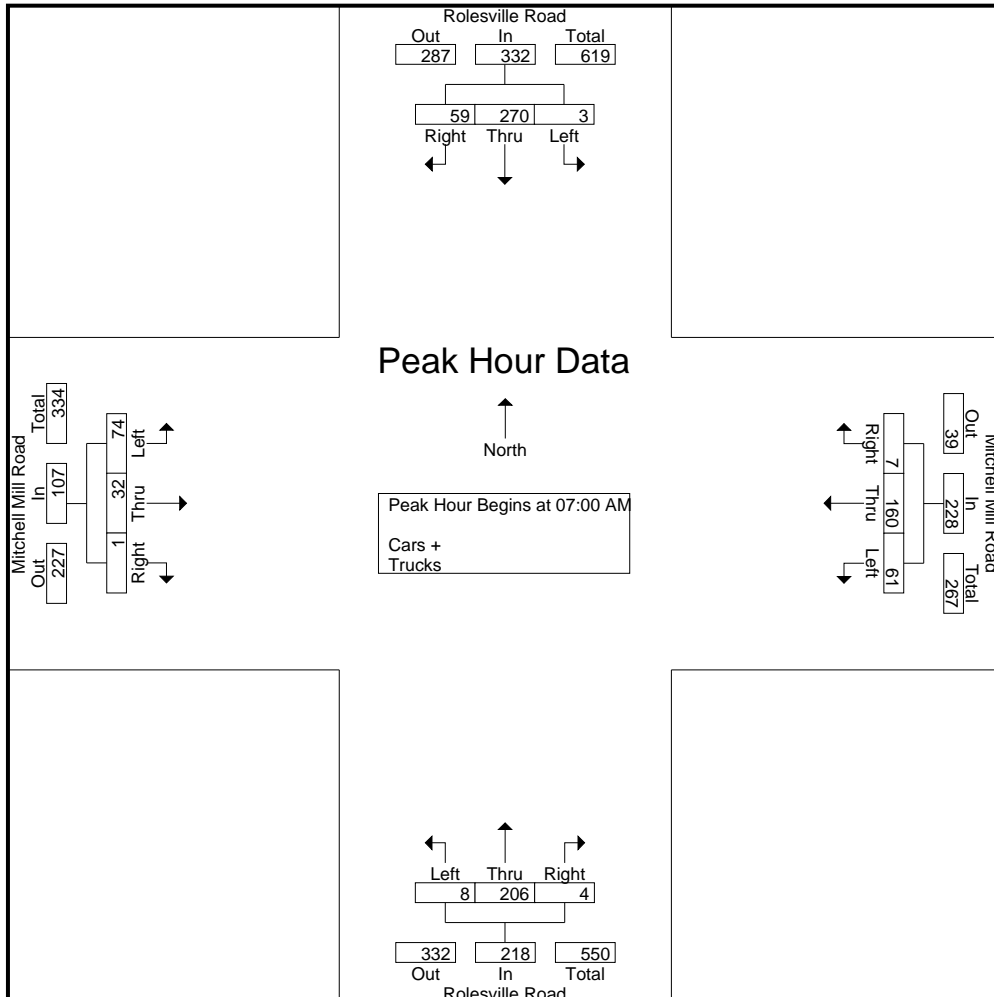
Start Time	Rolesville Road Southbound				Mitchell Mill Road Westbound				Rolesville Road Northbound				Mitchell Mill Road Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00 AM	24	61	0	85	0	36	12	48	2	60	3	65	1	9	55	65	263
07:15 AM	25	69	1	95	1	48	16	65	0	43	2	45	0	7	15	22	227
07:30 AM	6	75	1	82	2	44	17	63	1	60	3	64	0	7	3	10	219
07:45 AM	4	65	1	70	4	32	16	52	1	43	0	44	0	9	1	10	176
Total	59	270	3	332	7	160	61	228	4	206	8	218	1	32	74	107	885
08:00 AM	7	69	3	79	2	22	9	33	6	25	1	32	1	8	1	10	154
08:15 AM	6	41	1	48	2	19	9	30	3	23	2	28	1	5	0	6	112
08:30 AM	2	24	0	26	2	18	11	31	4	30	2	36	2	8	3	13	106
08:45 AM	0	32	0	32	2	11	8	21	4	22	0	26	1	4	1	6	85
Total	15	166	4	185	8	70	37	115	17	100	5	122	5	25	5	35	457
Grand Total	74	436	7	517	15	230	98	343	21	306	13	340	6	57	79	142	1342
Apprch %	14.3	84.3	1.4		4.4	67.1	28.6		6.2	90	3.8		4.2	40.1	55.6		
Total %	5.5	32.5	0.5	38.5	1.1	17.1	7.3	25.6	1.6	22.8	1	25.3	0.4	4.2	5.9	10.6	
Cars +	74	429	7	510	15	229	97	341	21	301	12	334	6	56	79	141	1326
% Cars +	100	98.4	100	98.6	100	99.6	99	99.4	100	98.4	92.3	98.2	100	98.2	100	99.3	98.8
Trucks	0	7	0	7	0	1	1	2	0	5	1	6	0	1	0	1	16
% Trucks	0	1.6	0	1.4	0	0.4	1	0.6	0	1.6	7.7	1.8	0	1.8	0	0.7	1.2



TRAFFIC DATA COLLECTION

File Name : Rolesville(Mitchell Mill and Rolesville) AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound				Mitchell Mill Road Westbound				Rolesville Road Northbound				Mitchell Mill Road Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00 AM																	
07:00 AM	24	61	0	85	0	36	12	48	2	60	3	65	1	9	55	65	263
07:15 AM	25	69	1	95	1	48	16	65	0	43	2	45	0	7	15	22	227
07:30 AM	6	75	1	82	2	44	17	63	1	60	3	64	0	7	3	10	219
07:45 AM	4	65	1	70	4	32	16	52	1	43	0	44	0	9	1	10	176
Total Volume	59	270	3	332	7	160	61	228	4	206	8	218	1	32	74	107	885
% App. Total	17.8	81.3	0.9		3.1	70.2	26.8		1.8	94.5	3.7		0.9	29.9	69.2		
PHF	.590	.900	.750	.874	.438	.833	.897	.877	.500	.858	.667	.838	.250	.889	.336	.412	.841





TRAFFIC DATA COLLECTION

File Name : Rolesville(Mitchell Mill and Rolesville) PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

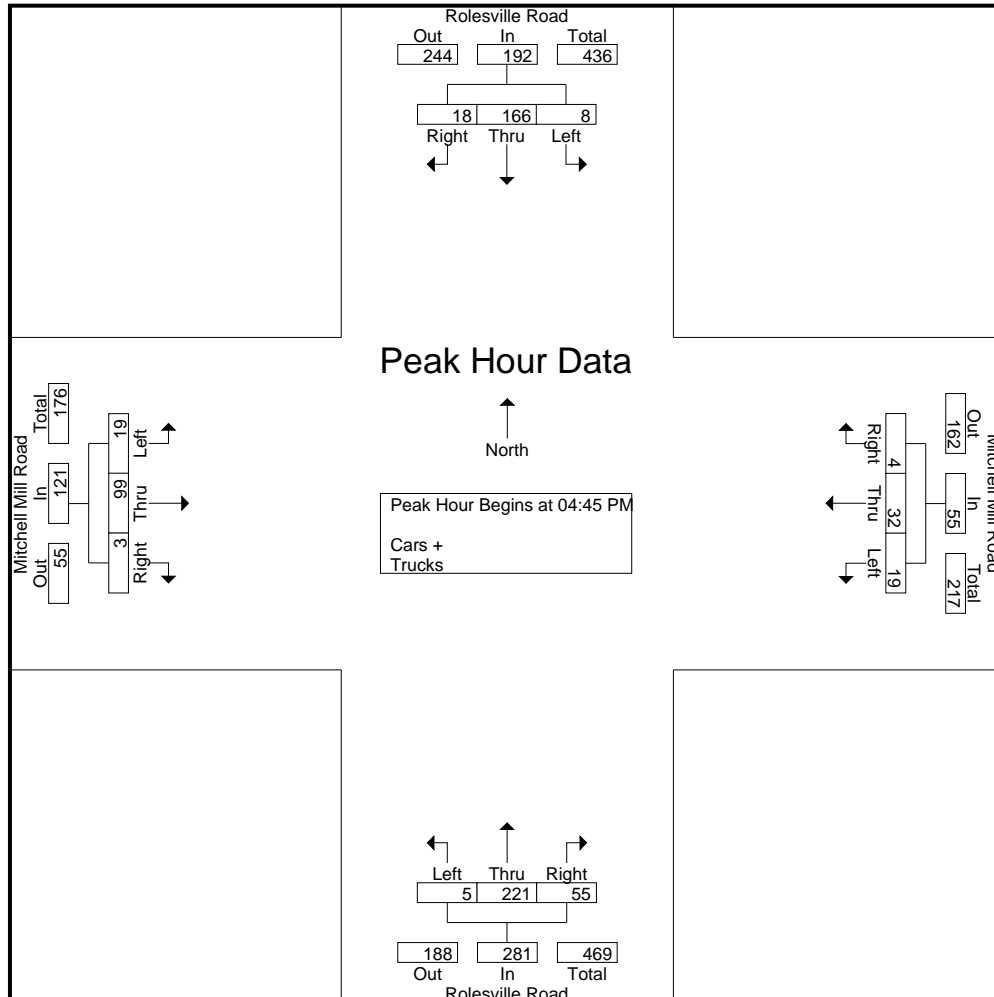
Start Time	Rolesville Road Southbound				Mitchell Mill Road Westbound				Rolesville Road Northbound				Mitchell Mill Road Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
04:00 PM	3	44	2	49	0	14	8	22	13	32	1	46	0	18	4	22	139
04:15 PM	5	30	1	36	1	7	6	14	11	41	0	52	3	31	6	40	142
04:30 PM	1	32	0	33	0	8	4	12	14	37	3	54	2	20	10	32	131
04:45 PM	5	47	4	56	3	7	5	15	11	54	2	67	0	19	3	22	160
Total	14	153	7	174	4	36	23	63	49	164	6	219	5	88	23	116	572
05:00 PM	6	39	1	46	1	9	6	16	10	52	1	63	1	29	5	35	160
05:15 PM	5	44	2	51	0	13	6	19	15	58	1	74	1	26	4	31	175
05:30 PM	2	36	1	39	0	3	2	5	19	57	1	77	1	25	7	33	154
05:45 PM	6	37	1	44	2	12	3	17	9	31	4	44	1	28	5	34	139
Total	19	156	5	180	3	37	17	57	53	198	7	258	4	108	21	133	628
Grand Total	33	309	12	354	7	73	40	120	102	362	13	477	9	196	44	249	1200
Apprch %	9.3	87.3	3.4		5.8	60.8	33.3		21.4	75.9	2.7		3.6	78.7	17.7		
Total %	2.8	25.8	1	29.5	0.6	6.1	3.3	10	8.5	30.2	1.1	39.8	0.8	16.3	3.7	20.8	
Cars +	33	305	12	350	7	73	38	118	100	361	13	474	9	195	44	248	1190
% Cars +	100	98.7	100	98.9	100	100	95	98.3	98	99.7	100	99.4	100	99.5	100	99.6	99.2
Trucks	0	4	0	4	0	0	2	2	2	1	0	3	0	1	0	1	10
% Trucks	0	1.3	0	1.1	0	0	5	1.7	2	0.3	0	0.6	0	0.5	0	0.4	0.8



TRAFFIC DATA COLLECTION

File Name : Rolesville(Mitchell Mill and Rolesville) PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound				Mitchell Mill Road Westbound				Rolesville Road Northbound				Mitchell Mill Road Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	5	47	4	56	3	7	5	15	11	54	2	67	0	19	3	22	160
05:00 PM	6	39	1	46	1	9	6	16	10	52	1	63	1	29	5	35	160
05:15 PM	5	44	2	51	0	13	6	19	15	58	1	74	1	26	4	31	175
05:30 PM	2	36	1	39	0	3	2	5	19	57	1	77	1	25	7	33	154
Total Volume	18	166	8	192	4	32	19	55	55	221	5	281	3	99	19	121	649
% App. Total	9.4	86.5	4.2		7.3	58.2	34.5		19.6	78.6	1.8		2.5	81.8	15.7		
PHF	.750	.883	.500	.857	.333	.615	.792	.724	.724	.953	.625	.912	.750	.853	.679	.864	.927





TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Fowler)AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

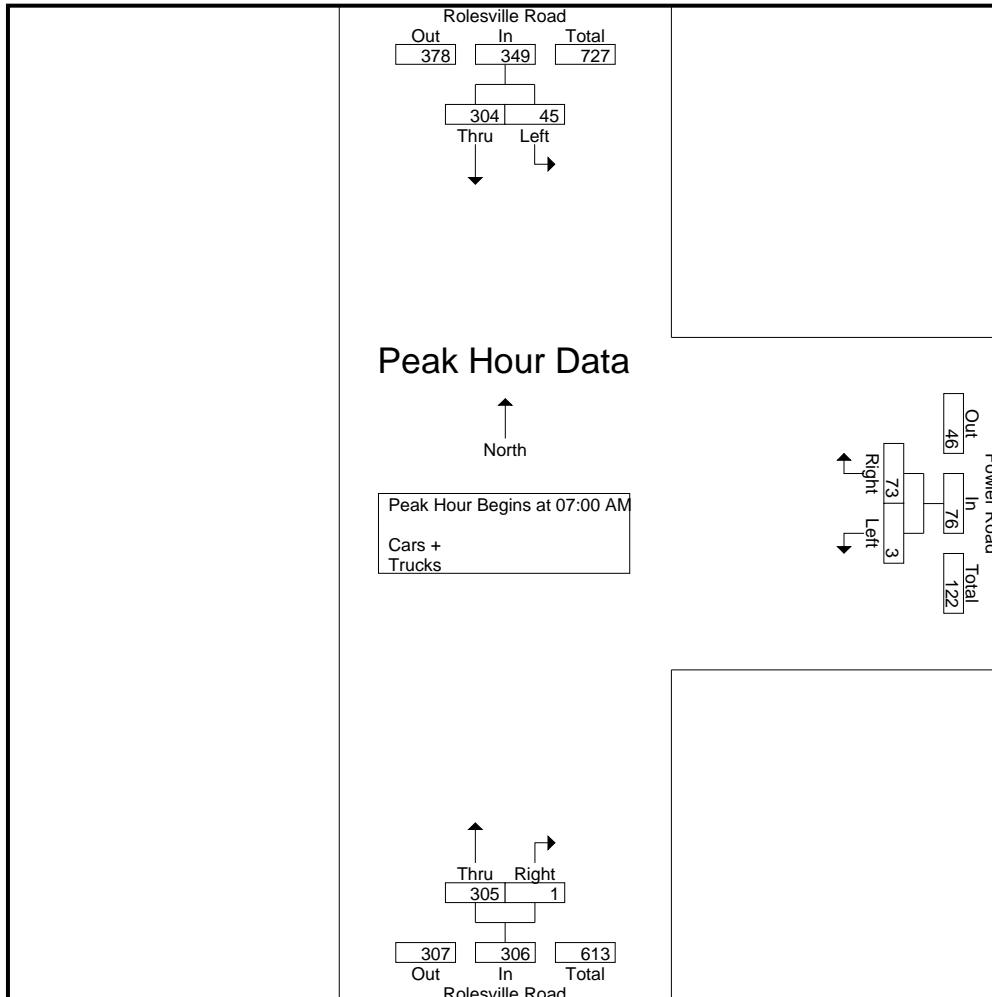
Start Time	Rolesville Road Southbound			Fowler Road Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
07:00 AM	69	6	75	25	0	25	0	107	107	207
07:15 AM	93	13	106	22	1	23	1	86	87	216
07:30 AM	75	12	87	11	0	11	0	53	53	151
07:45 AM	67	14	81	15	2	17	0	59	59	157
Total	304	45	349	73	3	76	1	305	306	731
08:00 AM	72	19	91	10	0	10	0	26	26	127
08:15 AM	38	14	52	9	1	10	0	27	27	89
08:30 AM	32	11	43	8	0	8	0	34	34	85
08:45 AM	29	5	34	18	0	18	0	25	25	77
Total	171	49	220	45	1	46	0	112	112	378
Grand Total	475	94	569	118	4	122	1	417	418	1109
Apprch %	83.5	16.5		96.7	3.3		0.2	99.8		
Total %	42.8	8.5	51.3	10.6	0.4	11	0.1	37.6	37.7	
Cars +	469	92	561	118	4	122	1	414	415	1098
% Cars +	98.7	97.9	98.6	100	100	100	100	99.3	99.3	99
Trucks	6	2	8	0	0	0	0	3	3	11
% Trucks	1.3	2.1	1.4	0	0	0	0	0.7	0.7	1



TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Fowler)AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound			Fowler Road Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:00 AM										
07:00 AM	69	6	75	25	0	25	0	107	107	207
07:15 AM	93	13	106	22	1	23	1	86	87	216
07:30 AM	75	12	87	11	0	11	0	53	53	151
07:45 AM	67	14	81	15	2	17	0	59	59	157
Total Volume	304	45	349	73	3	76	1	305	306	731
% App. Total	87.1	12.9		96.1	3.9		0.3	99.7		
PHF	.817	.804	.823	.730	.375	.760	.250	.713	.715	.846





TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Fowler)PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

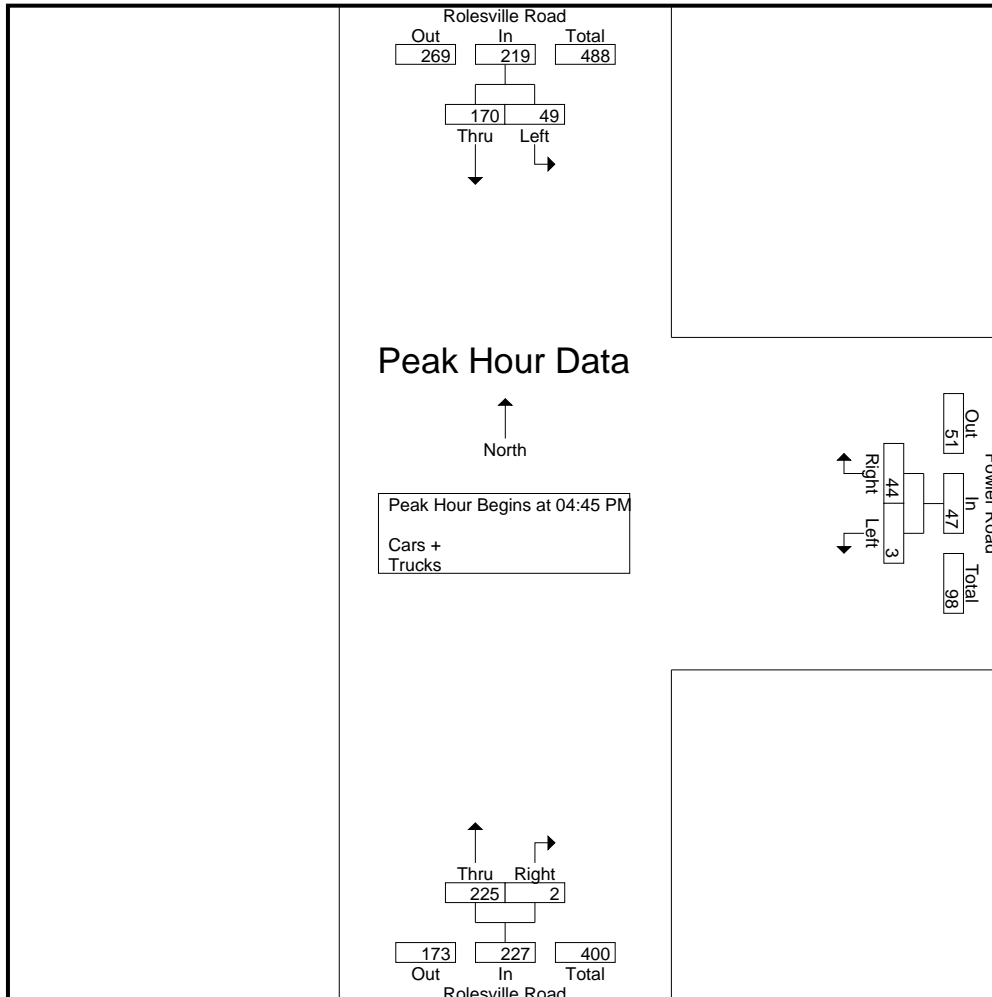
Start Time	Rolesville Road Southbound			Fowler Road Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
04:00 PM	38	17	55	10	1	11	1	31	32	98
04:15 PM	32	8	40	12	1	13	1	37	38	91
04:30 PM	33	13	46	7	0	7	0	32	32	85
04:45 PM	54	11	65	11	2	13	0	57	57	135
Total	157	49	206	40	4	44	2	157	159	409
05:00 PM	34	15	49	8	1	9	1	46	47	105
05:15 PM	43	8	51	14	0	14	1	55	56	121
05:30 PM	39	15	54	11	0	11	0	67	67	132
05:45 PM	41	14	55	8	0	8	0	38	38	101
Total	157	52	209	41	1	42	2	206	208	459
Grand Total	314	101	415	81	5	86	4	363	367	868
Apprch %	75.7	24.3		94.2	5.8		1.1	98.9		
Total %	36.2	11.6	47.8	9.3	0.6	9.9	0.5	41.8	42.3	
Cars +	311	100	411	80	5	85	4	362	366	862
% Cars +	99	99	99	98.8	100	98.8	100	99.7	99.7	99.3
Trucks	3	1	4	1	0	1	0	1	1	6
% Trucks	1	1	1	1.2	0	1.2	0	0.3	0.3	0.7



TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Fowler)PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound			Fowler Road Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	54	11	65	11	2	13	0	57	57	135
05:00 PM	34	15	49	8	1	9	1	46	47	105
05:15 PM	43	8	51	14	0	14	1	55	56	121
05:30 PM	39	15	54	11	0	11	0	67	67	132
Total Volume	170	49	219	44	3	47	2	225	227	493
% App. Total	77.6	22.4		93.6	6.4		0.9	99.1		
PHF	.787	.817	.842	.786	.375	.839	.500	.840	.847	.913





TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Traviswood)AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

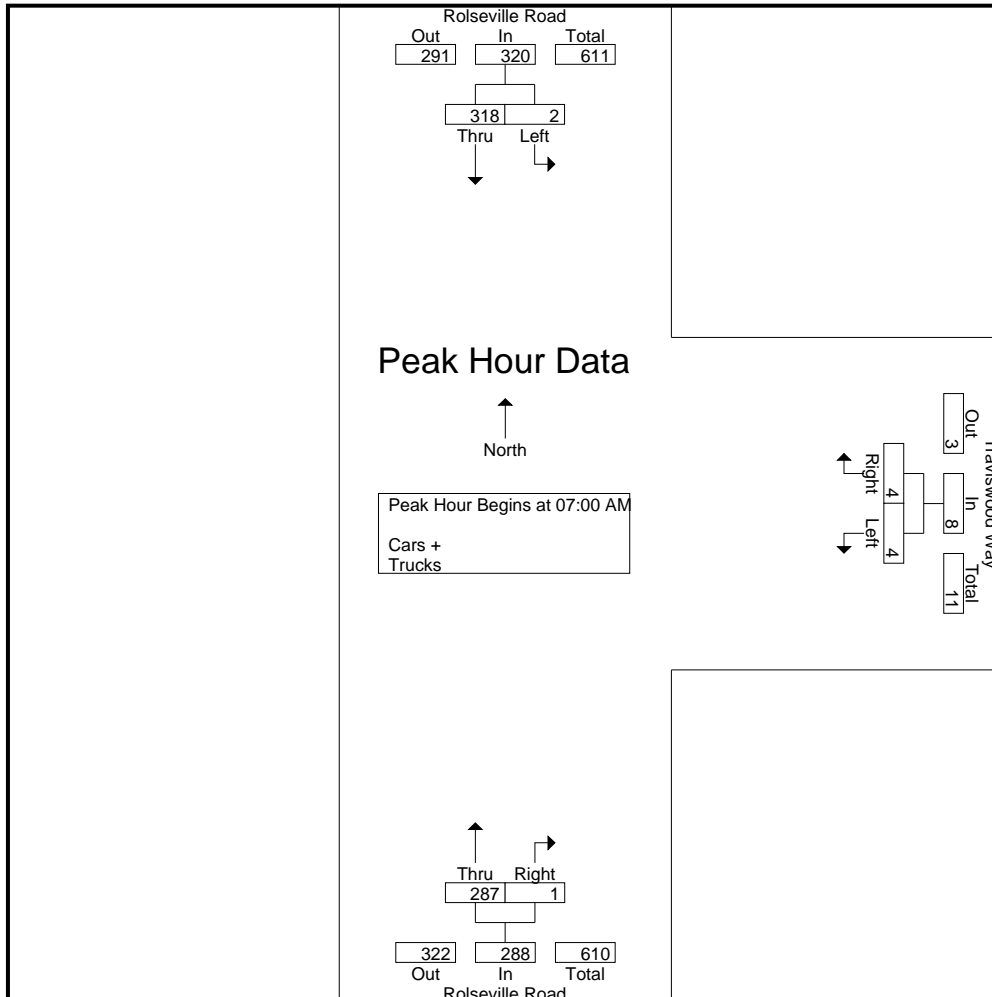
Start Time	Rolesville Road Southbound			Traviswood Way Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
07:00 AM	83	0	83	1	2	3	0	113	113	199
07:15 AM	90	1	91	2	0	2	0	63	63	156
07:30 AM	76	0	76	0	1	1	1	66	67	144
07:45 AM	69	1	70	1	1	2	0	45	45	117
Total	318	2	320	4	4	8	1	287	288	616
08:00 AM	71	0	71	0	0	0	1	25	26	97
08:15 AM	42	0	42	0	2	2	0	28	28	72
08:30 AM	28	0	28	0	0	0	0	33	33	61
08:45 AM	30	0	30	0	0	0	0	24	24	54
Total	171	0	171	0	2	2	1	110	111	284
Grand Total	489	2	491	4	6	10	2	397	399	900
Apprch %	99.6	0.4		40	60		0.5	99.5		
Total %	54.3	0.2	54.6	0.4	0.7	1.1	0.2	44.1	44.3	
Cars +	482	2	484	4	6	10	2	392	394	888
% Cars +	98.6	100	98.6	100	100	100	100	98.7	98.7	98.7
Trucks	7	0	7	0	0	0	0	5	5	12
% Trucks	1.4	0	1.4	0	0	0	0	1.3	1.3	1.3



TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Traviswood)AM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound			Traviswood Way Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:00 AM										
07:00 AM	83	0	83	1	2	3	0	113	113	199
07:15 AM	90	1	91	2	0	2	0	63	63	156
07:30 AM	76	0	76	0	1	1	1	66	67	144
07:45 AM	69	1	70	1	1	2	0	45	45	117
Total Volume	318	2	320	4	4	8	1	287	288	616
% App. Total	99.4	0.6		50	50		0.3	99.7		
PHF	.883	.500	.879	.500	.500	.667	.250	.635	.637	.774





TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Traviswood)PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 1

Groups Printed- Cars + - Trucks

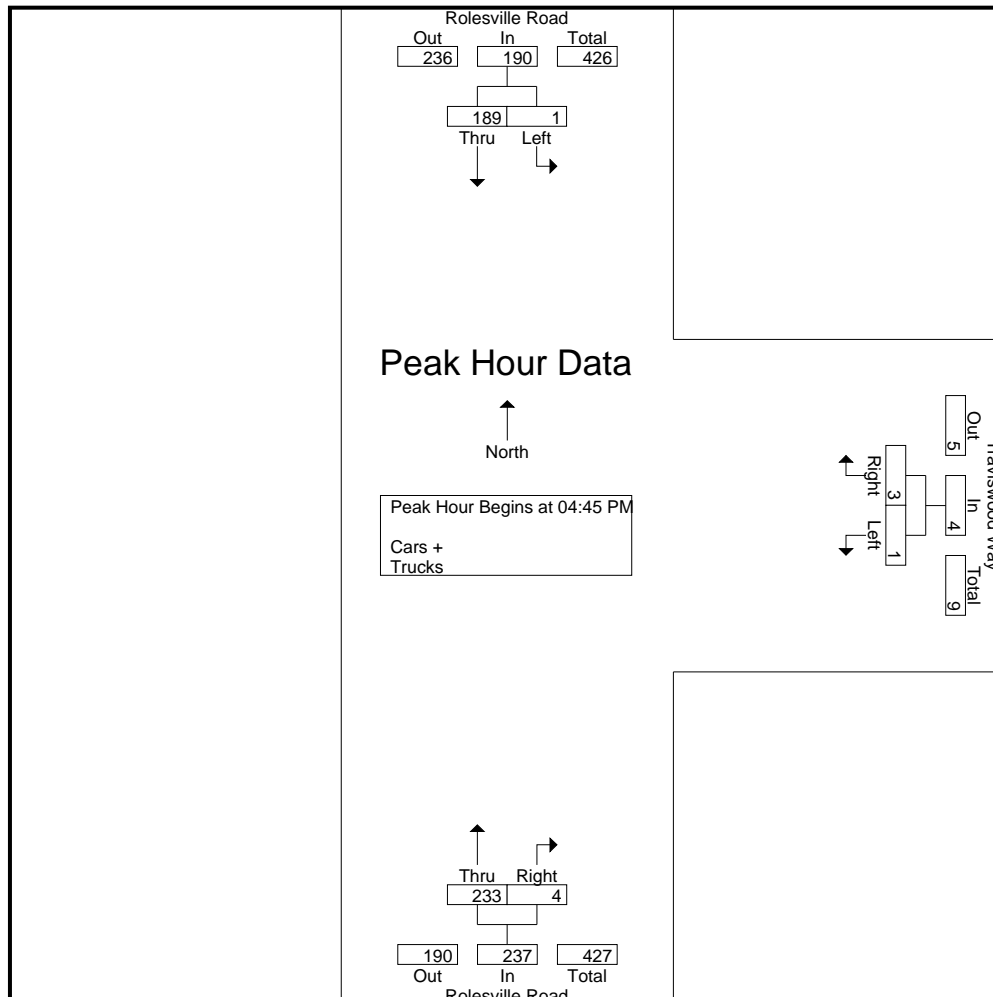
Start Time	Rolesville Road Southbound			Traviswood Way Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
04:00 PM	44	0	44	0	1	1	0	33	33	78
04:15 PM	37	0	37	0	0	0	1	38	39	76
04:30 PM	31	0	31	0	0	0	2	42	44	75
04:45 PM	58	0	58	0	0	0	0	55	55	113
Total	170	0	170	0	1	1	3	168	171	342
05:00 PM	39	1	40	0	1	1	1	54	55	96
05:15 PM	50	0	50	2	0	2	1	64	65	117
05:30 PM	42	0	42	1	0	1	2	60	62	105
05:45 PM	41	0	41	0	1	1	0	38	38	80
Total	172	1	173	3	2	5	4	216	220	398
Grand Total	342	1	343	3	3	6	7	384	391	740
Apprch %	99.7	0.3		50	50		1.8	98.2		
Total %	46.2	0.1	46.4	0.4	0.4	0.8	0.9	51.9	52.8	
Cars +	339	1	340	3	3	6	7	383	390	736
% Cars +	99.1	100	99.1	100	100	100	100	99.7	99.7	99.5
Trucks	3	0	3	0	0	0	0	1	1	4
% Trucks	0.9	0	0.9	0	0	0	0	0.3	0.3	0.5



TRAFFIC DATA COLLECTION

File Name : Rolesville(Rolesville and Traviswood)PM Peak
 Site Code :
 Start Date : 5/2/2019
 Page No : 2

Start Time	Rolesville Road Southbound			Traviswood Way Westbound			Rolesville Road Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	58	0	58	0	0	0	0	55	55	113
05:00 PM	39	1	40	0	1	1	1	54	55	96
05:15 PM	50	0	50	2	0	2	1	64	65	117
05:30 PM	42	0	42	1	0	1	2	60	62	105
Total Volume	189	1	190	3	1	4	4	233	237	431
% App. Total	99.5	0.5		75	25		1.7	98.3		
PHF	.815	.250	.819	.375	.250	.500	.500	.910	.912	.921



APPENDIX C

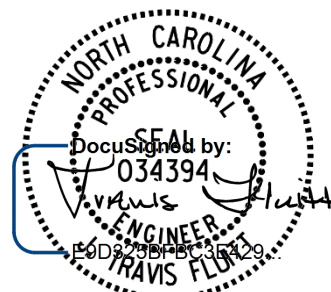
ADJACENT DEVELOPMENT INFORMATION

Traffic Impact Analysis for
Young Street PUD
Rolesville, North Carolina

Prepared for:
Ashton Woods
Raleigh, North Carolina

Prepared by:
Kimley-Horn and Associates, Inc.
NC License #F-0102
421 Fayetteville Street, Suite 600
Raleigh, NC 27601
(919) 677-2000

April 2019
015956012



4/4/2019

North of Axl Byway

RTS PUD	
STREETS	20 FT
FRONT	3 FT (2ND UNIT)
SIDE	25 FT
REAR	22 FT
MIN WIDTH	42.74 ACRES
AREA	211 PROPOSED TOWNHOME UNITS
DENSITY	4.73 DENSITY
ALLOWED	8 DENSITY
OS	30.71 ACRES (PROPOSED OPEN SPACE)
	46.8% OPEN SPACE
	30% MIN REQUIRED FOR TOWN HOMES
REGULATORY OPEN SPACE	7.74 ACRES HOUSE BUFFER
	7.08 ACRES LANDSCAPE BUFFER
	15.4 ACRES VEG. OPEN SPACE
	5.45 ACRES (25% ACTUAL OPEN SPACE)
	3.47 ACRES ACTUAL OPEN SPACE PROVIDED
PARKING	
TOWNHOMES	2 SPACES/UNIT + 0.25 SPACES/UNIT VISITOR PARKING
	+ 5 SPACES/UNIT OVER 3000 SQ. FT.
REQUIRED SPACES	
2 SP/UNIT	422 SPACES
PROPOSED SPACES	212 GARAGE SPACES
0.5 SP/UNIT	106 SPACES
211 GARAGE SPACES	
0.5 SP/UNIT	53 SPACES
211 ADDITIONAL SPACES	
TOTAL	581 ADDITIONAL PARKING
	128 SPACES
	133 SPACES

South of Axl Byway

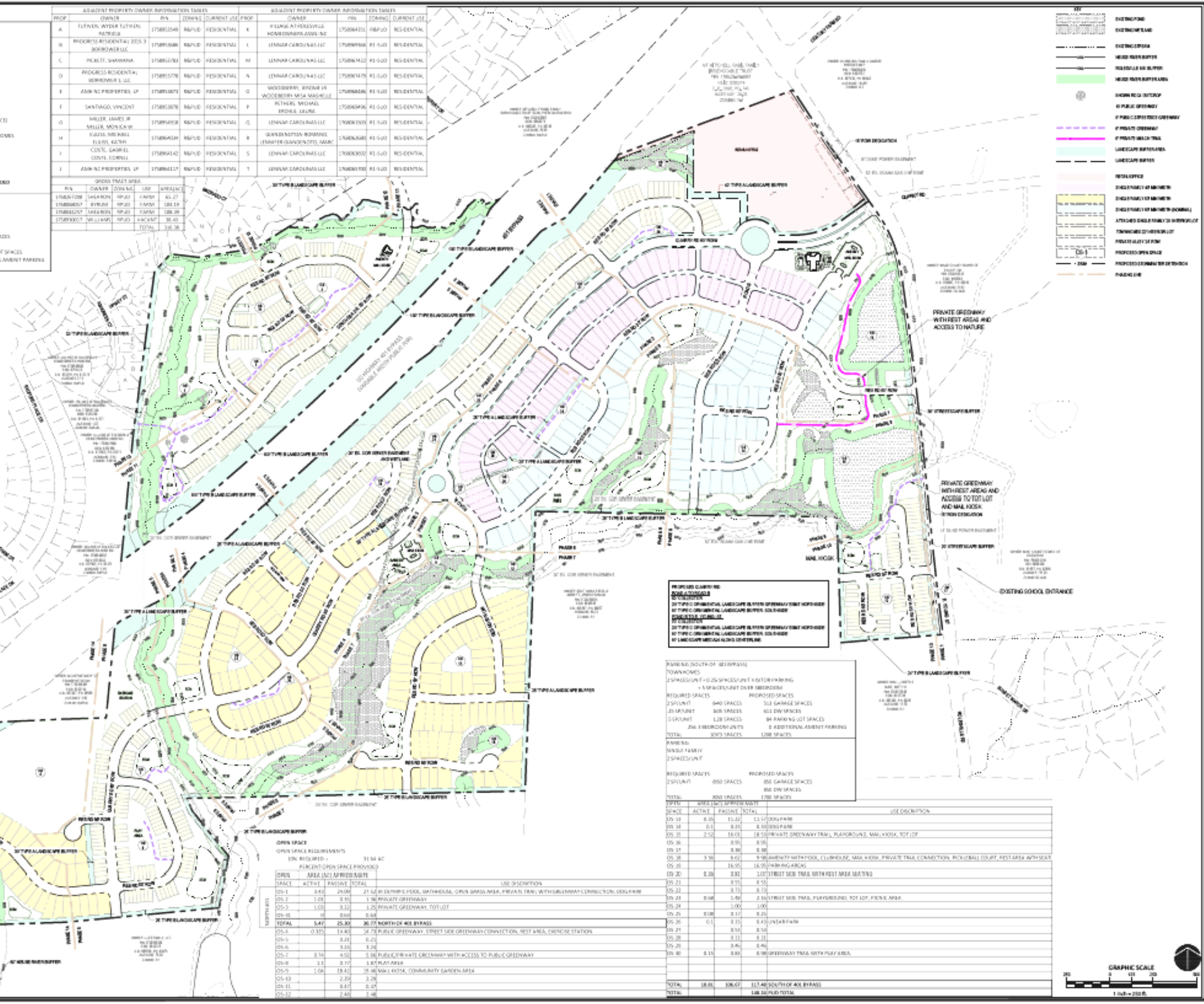
RTS PUD	
STREETS	20 FT
FRONT	3 FT
SIDE	30 FT
CORNER SIDE	30 FT
REAR	20 FT
MIN WIDTH	50 FT (20' WIDE NOTED)
MIN LOT AREA	6,300 SF
MIN LOT AREA	4,300 SF (ACT. LOT)
RTS PUD	
STREETS	20 FT
FRONT	3 FT (2ND UNIT)
SIDE	25 FT
REAR	22 FT
MIN WIDTH	42.74 ACRES
AREA TOTAL	204.21 ACRES
AREA	12.26 ACRES
RESIDENTIAL	212.95 ACRES
UNITS (TOWNHOME)	430 PROPOSED TOWNHOME UNITS
UNITS (SINGLE FAMILY)	300 SINGLE FAMILY
UNITS (SINGLE FAMILY)	100-40 SINGLE FAMILY
TOTAL UNITS	830
OVERALL DENSITY	6.81 DENSITY
ALLOWED DENSITY	6.0 DENSITY
TOWNHOME AREA	68.34 ACRES
DENSITY	6.1 DENSITY/TOWNHOME
ALLOWED DENSITY	6.0 DENSITY
SINGLE FAMILY AREA	202.60 ACRES
DENSITY	2.1 DENSITY
ALLOWED DENSITY	6.0 DENSITY
OPEN SPACE REQUIRED	
25%	25.1 ACRES REQUIRED
	117.46 ACRES PROVIDED
46.8% PROVIDED	
TOWNHOME	
25%	17.25 ACRES REQUIRED
	117.46 ACRES PROVIDED
83% ACTUAL	
25% ACTUAL	16 ACRES REQUIRED
	133.81 ACRES PROVIDED

ADJACENT PROPERTY OWNER INFORMATION TABLE

PROP	OWNER	RSN	ZONING	CURRENT USE	PROP	OWNER	RSN	ZONING	CURRENT USE
A	ETERNAL WATER ETHERAL	175803548	MP/UD	RESIDENTIAL	K	ETERNAL WATER ETHERAL	175803548	MP/UD	RESIDENTIAL
B	PROGRESS RESIDENTIAL 2023-3	175803548	MP/UD	RESIDENTIAL	L	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
C	ROCKY, SHAMMA	175803548	MP/UD	RESIDENTIAL	M	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
D	PROGRESS RESIDENTIAL 2023-3	175803548	MP/UD	RESIDENTIAL	N	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
E	ADIR-NC PROPERTIES LP	175803548	MP/UD	RESIDENTIAL	O	WOODBERRY, BRUCE JR	175803548	R1-5UD	RESIDENTIAL
F	SANTAGO VINCENT	175803548	MP/UD	RESIDENTIAL	P	PETERS, MICHAEL	175803548	R1-5UD	RESIDENTIAL
G	MILLER, JAMES R	175803548	MP/UD	RESIDENTIAL	Q	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
H	MILLER, MONSIEUR	175803548	MP/UD	RESIDENTIAL	R	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
I	COOTE, SARAH	175803548	MP/UD	RESIDENTIAL	S	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL
J	ADIR-NC PROPERTIES LP	175803548	MP/UD	RESIDENTIAL	T	LENNAR CAROLINAS LLC	175803548	R1-5UD	RESIDENTIAL

OPEN SPACE TABLE

RSN	OWNER	AREA	TYPE	DATE
175803548	ETERNAL WATER ETHERAL	30.71	OS	01/21/2023
175803548	ETERNAL WATER ETHERAL	12.26	OS	01/21/2023
175803548	ETERNAL WATER ETHERAL	15.4	OS	01/21/2023
175803548	ETERNAL WATER ETHERAL	5.45	OS	01/21/2023
175803548	ETERNAL WATER ETHERAL	3.47	OS	01/21/2023
TOTAL		77.29		



WithersRavenel
Engineers | Planners | Surveyors

ROLESVILLE YOUNG ST. PUD
ROLESVILLE, NC

PUD MASTERPLAN

PRELIMINARY
NOT APPROVED FOR
CONSTRUCTION
DATE: 08/28/23

MP

ROLESVILLE YOUNG ST. PUD
ROLESVILLE, NC

PUD MASTERPLAN

ROLESVILLE YOUNG ST. PUD
ROLESVILLE, NC

PUD MASTERPLAN

ROLESVILLE YOUNG ST. PUD
ROLESVILLE, NC

PUD MASTERPLAN

FIGURE 2

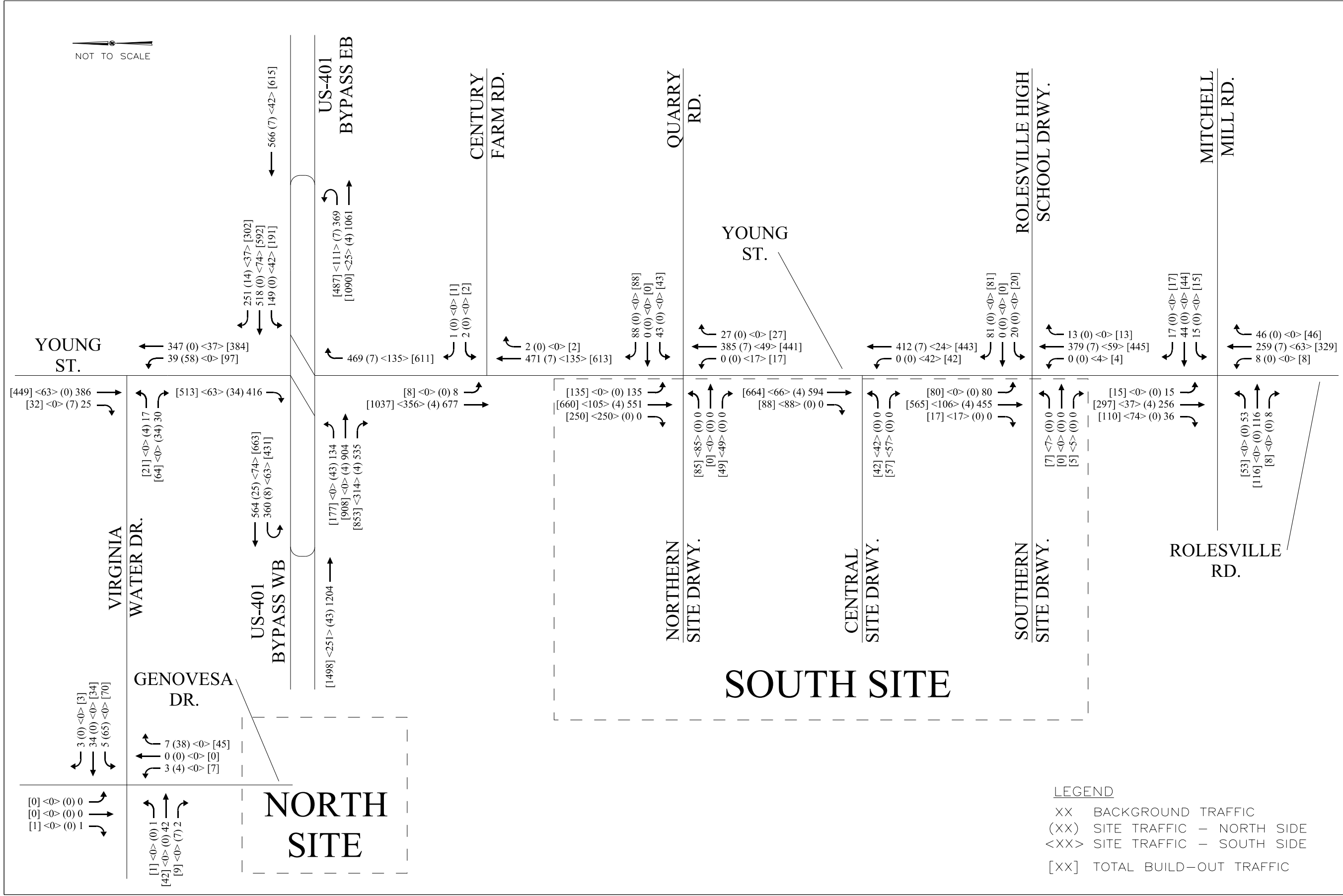
PRELIMINARY SITE PLAN

YOUNG STREET PUD
ROLESVILLE, NC
TRAFFIC IMPACT ANALYSIS



THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY KIMLEY-HORN AND ASSOCIATES, INC. SHALL BE WITHOUT LIABILITY TO KIMLEY-HORN AND ASSOCIATES, INC.

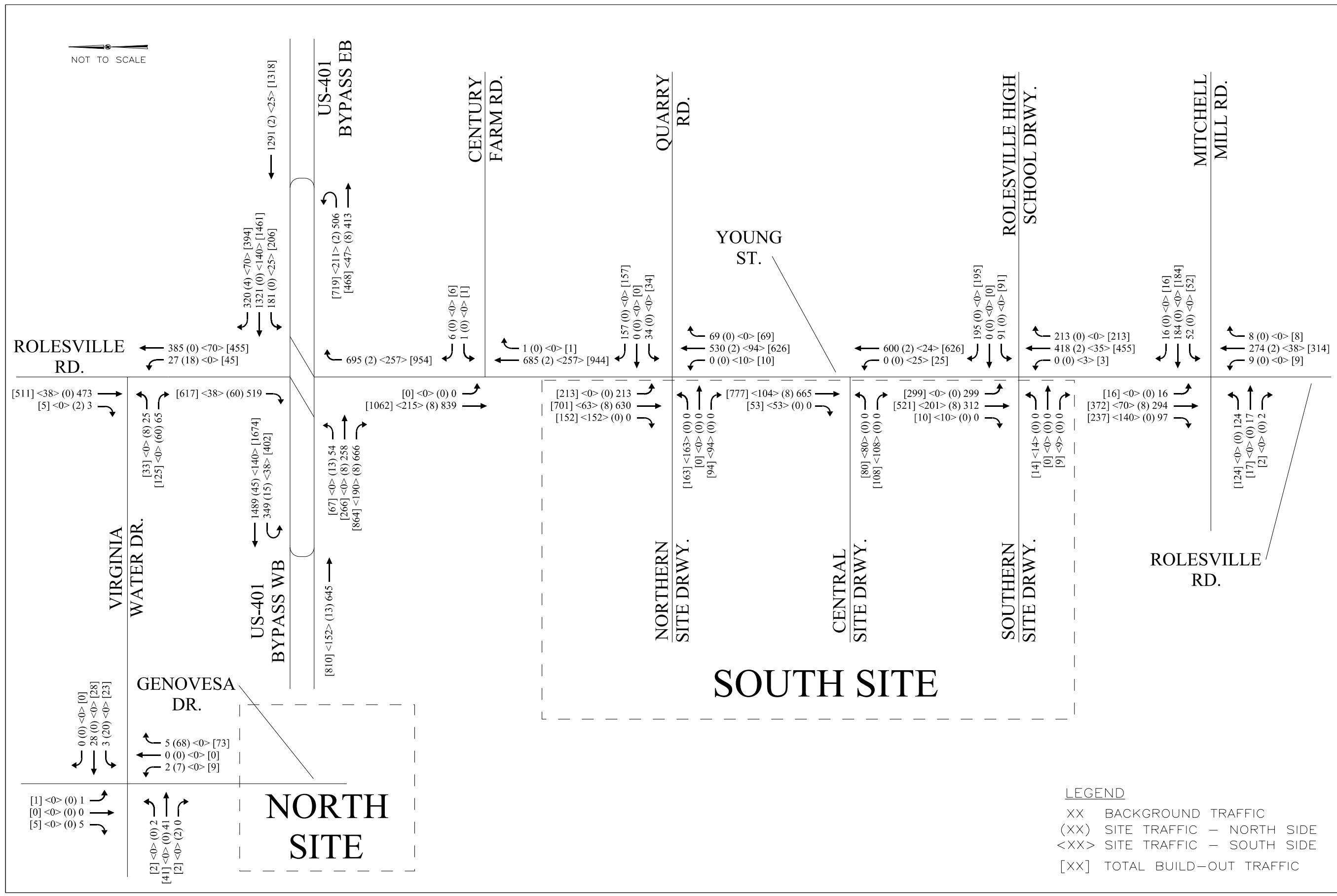
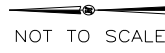
NOT TO SCALE



YOUNG STREET PUD
ROLESVILLE, NC
TRAFFIC IMPACT ANALYSIS

PROJECTED (2025)
BUILD-OUT PM PEAK HOUR
TRAFFIC VOLUMES -
RESIDENTIAL BUILD-OUT

FIGURE
13



PROJECTED (2025)
BUILD-OUT AM PEAK HOUR
TRAFFIC VOLUMES –
COMMERCIAL BUILD-OUT

YOUNG STREET PUD
ROLESVILLE, NC
TRAFFIC IMPACT ANALYSIS

FIGURE
14



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Kalas Property Traffic Impact Analysis

Prepared for:
Mitchell Mill Road Investors LLC
PO Box 3557
Cary, NC, 27519

Prepared by:
Stantec Consulting Services Inc.
801 Jones Franklin Road
Suite 300
Raleigh, NC 27606



January 19, 2016

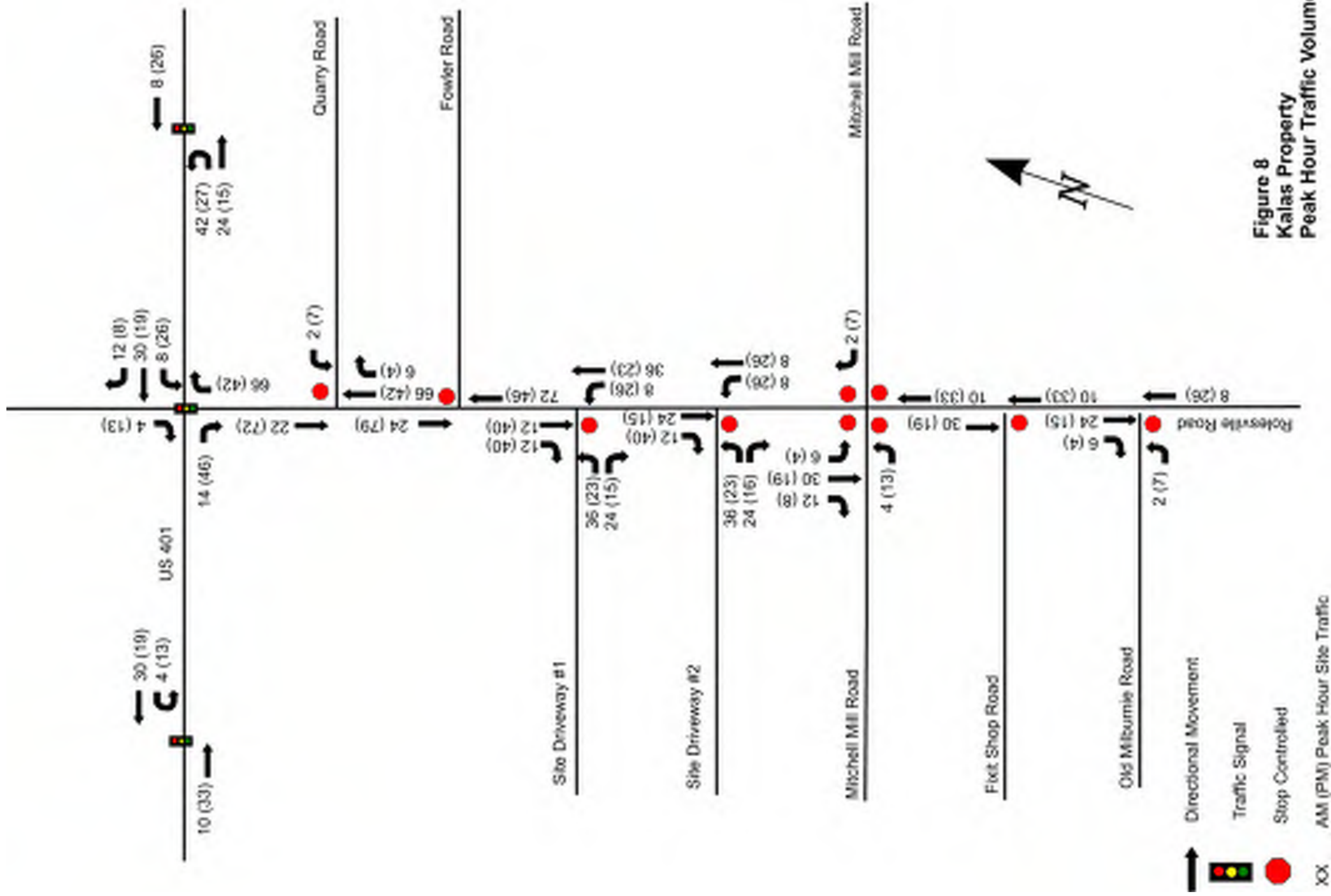


Figure 8
 Kalas Property
 Peak Hour Traffic Volumes

KALAS PROPERTY TRAFFIC IMPACT ANALYSIS

Projected Traffic Volumes
January 2016

6.2 APPROVED DEVELOPMENT TRAFFIC

Approved development traffic is generated by specific approved, but not yet constructed, projects within the vicinity of the subject project. Based on coordination with the Town of Rolesville and NCDOT, the following approved developments were considered in the analyses of the Kalas Property development:

- Rogers Farm Subdivision

A traffic impact analysis report was not required by the Town of Rolesville for the Rogers Farm subdivision. With the information provided regarding the future development, trips generated by the development during the AM and PM peak hours were distributed in the same manner as the site traffic distributed in this report (see Figure 6).

The following is the ITE trip generation information used for the Rogers Farm subdivision:

Land Use	ITE Site Code	Size	24 Hour Two-Way Volume	AM Peak		PM Peak	
				Enter	Exit	Enter	Exit
Single-Family Detached Housing	210	98 units	1,030	20	59	65	38

A summary of all traffic from the above mentioned approved developments is illustrated in Figure 6A.

KALAS PROPERTY TRAFFIC IMPACT ANALYSIS

Projected Traffic Volumes

January 2016

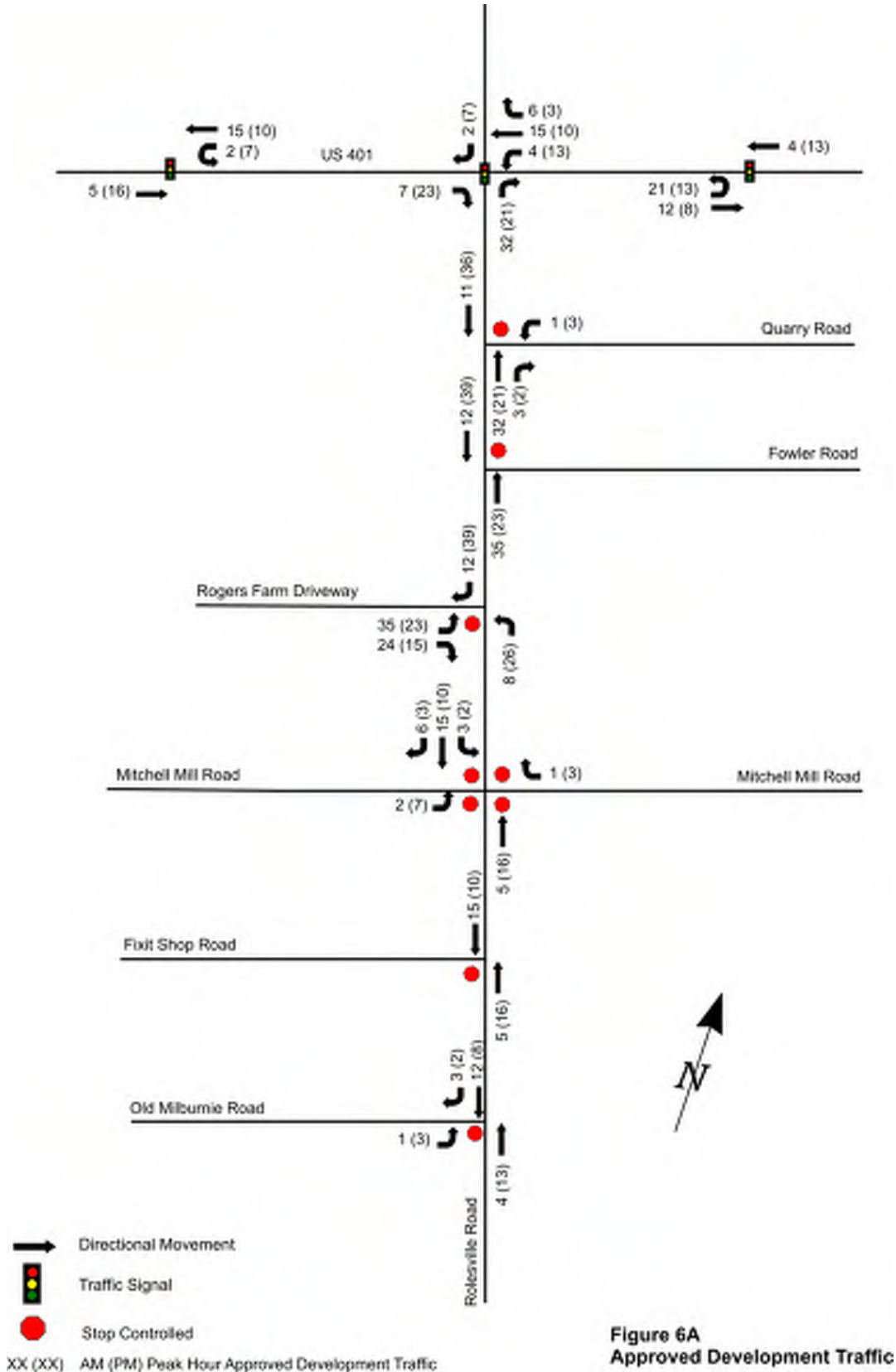


Figure 6A
Approved Development Traffic

Executive Summary

The proposed Watkins Family Property Development is located on the west side of Rolesville Road just north of Mitchell Mill Road in Rolesville, NC. The proposed development will consist of a maximum 145 single family homes. At full build out, the development is expected to generate 1464 new trips per average weekday. In the AM and PM peak hours, the development will generate approximately 108 AM peak hour trips (27 entering and 81 exiting) and 145 PM peak hour trips (91 entering and 54 exiting), respectively. The project completion date is expected to be 2025.

Access to the site is envisioned to be provided via the approved but not yet constructed access points for the Kalas Property on Rolesville Road. The first access is approximately 1,500 feet and the other approximately 2,300 feet north of the intersection of Rolesville Road at Mitchell Mill Road.

The purpose of this report is to evaluate the proposed development in terms of projected traffic conditions, evaluate the ability of the adjacent roadways to accommodate the additional traffic volumes, and to recommend transportation improvements needed to mitigate congestion that may result from the additional site traffic. This report presents trip generation, trip distribution, traffic analyses, and recommendations for transportation improvements needed to meet anticipated traffic demands. This report examines the following scenarios for the AM and PM peak hours:

- 2018 Existing
- 2025 No-Build
- 2025 Build Out
- 2025 Build Out with Improvements

Capacity analyses for the AM and PM peak hours in each scenario were performed for the following intersections:

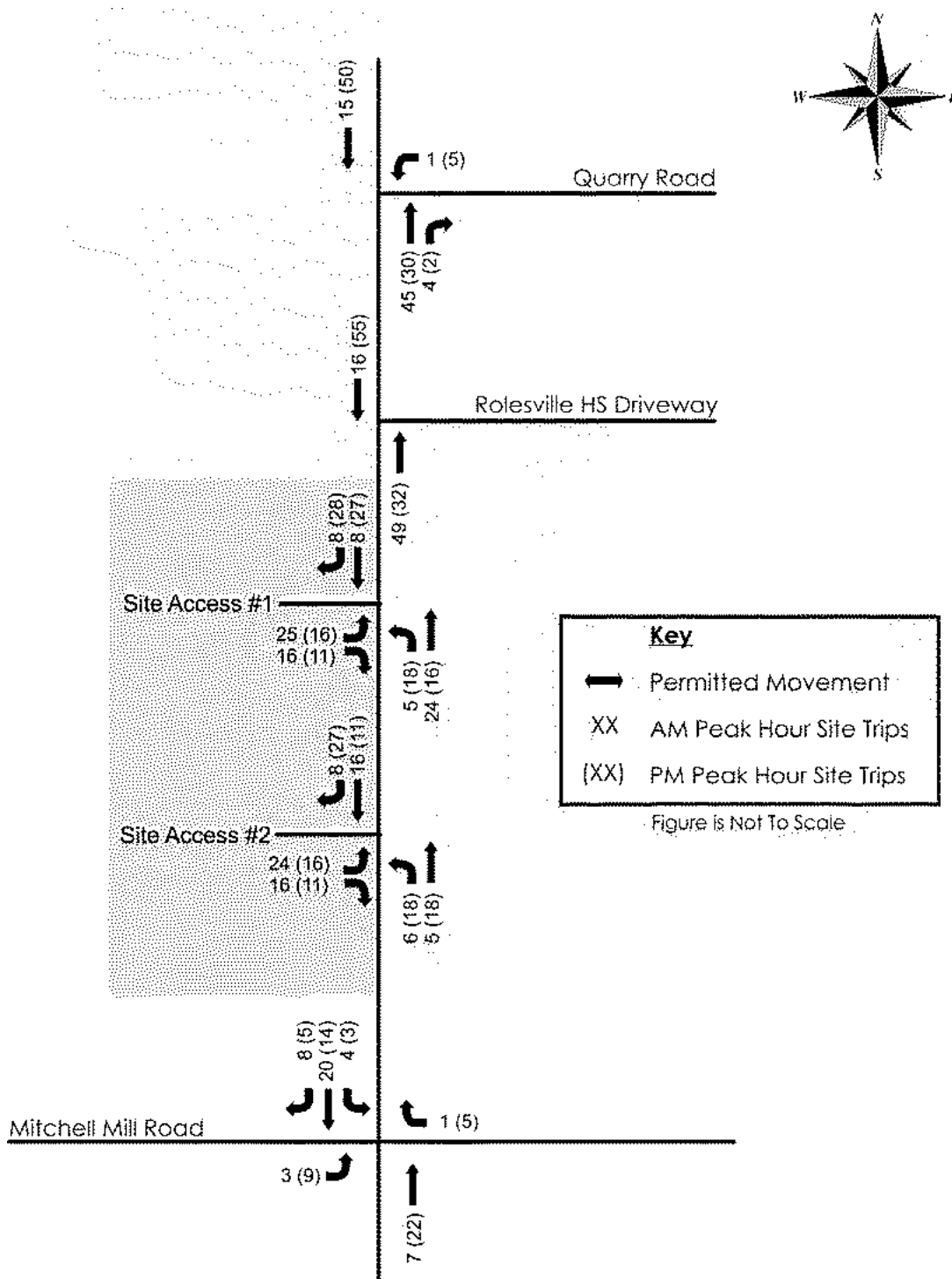
- Rolesville Road at Mitchell Mill Road;
- Rolesville Road at Rolesville High School Driveway;
- Rolesville Road at Quarry Road;
- Rolesville Road at Site Access 1; and
- Rolesville Road at Site Access 2.

Table ES-1 shows a summary of the capacity analyses results included in this Traffic Impact Analysis (TIA).

WATKINS FAMILY PROPERTY DEVELOPMENT TRAFFIC IMPACT ANALYSIS

Traffic Generation
 October 29, 2018

Figure 6: Proposed Site Trips



APPENDIX D

CAPACITY ANALYSIS CALCULATIONS

ROLESVILLE ROAD

&

MITCHELL MILL ROAD

Intersection

Intersection Delay, s/veh	14
Intersection LOS	B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	74	32	1	61	160	7	8	207	4	3	270	59
Future Vol, veh/h	74	32	1	61	160	7	8	207	4	3	270	59
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	82	36	1	68	178	8	9	230	4	3	300	66
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			1		
HCM Control Delay	11.3			14.5			12.6			15.4		
HCM LOS	B			B			B			C		

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1
Vol Left, %	4%	69%	28%	0%	1%
Vol Thru, %	95%	30%	72%	0%	81%
Vol Right, %	2%	1%	0%	100%	18%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	219	107	221	7	332
LT Vol	8	74	61	0	3
Through Vol	207	32	160	0	270
RT Vol	4	1	0	7	59
Lane Flow Rate	243	119	246	8	369
Geometry Grp	2	5	7	7	2
Degree of Util (X)	0.391	0.214	0.45	0.012	0.561
Departure Headway (Hd)	5.78	6.472	6.599	5.745	5.476
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	618	550	543	619	657
Service Time	3.856	4.565	4.372	3.518	3.544
HCM Lane V/C Ratio	0.393	0.216	0.453	0.013	0.562
HCM Control Delay	12.6	11.3	14.7	8.6	15.4
HCM Lane LOS	B	B	B	A	C
HCM 95th-tile Q	1.9	0.8	2.3	0	3.5

Intersection

Intersection Delay, s/veh	10.2
Intersection LOS	B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔		↔			↔	
Traffic Vol, veh/h	19	99	3	19	32	4	5	221	55	8	166	18
Future Vol, veh/h	19	99	3	19	32	4	5	221	55	8	166	18
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	21	110	3	21	36	4	6	246	61	9	184	20
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	2	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	2
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	2	1
HCM Control Delay	9.8	9.5	10.7	9.8
HCM LOS	A	A	B	A

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1
Vol Left, %	2%	16%	37%	0%	4%
Vol Thru, %	79%	82%	63%	0%	86%
Vol Right, %	20%	2%	0%	100%	9%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	281	121	51	4	192
LT Vol	5	19	19	0	8
Through Vol	221	99	32	0	166
RT Vol	55	3	0	4	18
Lane Flow Rate	312	134	57	4	213
Geometry Grp	2	5	7	7	2
Degree of Util (X)	0.4	0.2	0.096	0.006	0.284
Departure Headway (Hd)	4.614	5.361	6.107	5.209	4.788
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	774	663	581	679	745
Service Time	2.671	3.448	3.905	3.006	2.852
HCM Lane V/C Ratio	0.403	0.202	0.098	0.006	0.286
HCM Control Delay	10.7	9.8	9.6	8	9.8
HCM Lane LOS	B	A	A	A	A
HCM 95th-tile Q	1.9	0.7	0.3	0	1.2

Intersection

Intersection Delay, s/veh 108.7
 Intersection LOS F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔		↔			↔	
Traffic Vol, veh/h	94	37	1	70	184	12	9	299	5	16	448	225
Future Vol, veh/h	94	37	1	70	184	12	9	299	5	16	448	225
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	104	41	1	78	204	13	10	332	6	18	498	250
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			1		
HCM Control Delay	16.8			24.2			25.3			196.8		
HCM LOS	C			C			D			F		

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1
Vol Left, %	3%	71%	28%	0%	2%
Vol Thru, %	96%	28%	72%	0%	65%
Vol Right, %	2%	1%	0%	100%	33%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	313	132	254	12	689
LT Vol	9	94	70	0	16
Through Vol	299	37	184	0	448
RT Vol	5	1	0	12	225
Lane Flow Rate	348	147	282	13	766
Geometry Grp	2	5	7	7	2
Degree of Util (X)	0.677	0.335	0.619	0.026	1.369
Departure Headway (Hd)	7.674	9.229	8.803	7.933	6.44
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	473	392	412	454	567
Service Time	5.674	7.229	6.503	5.633	4.464
HCM Lane V/C Ratio	0.736	0.375	0.684	0.029	1.351
HCM Control Delay	25.3	16.8	24.8	10.8	196.8
HCM Lane LOS	D	C	C	B	F
HCM 95th-tile Q	5	1.4	4	0.1	34.1

Intersection

Intersection Delay, s/veh 26.7
 Intersection LOS D

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔		↔			↔	
Traffic Vol, veh/h	51	114	3	22	37	20	6	405	63	18	291	144
Future Vol, veh/h	51	114	3	22	37	20	6	405	63	18	291	144
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	57	127	3	24	41	22	7	450	70	20	323	160
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			1		
HCM Control Delay	14.6			11.8			32.5			27.8		
HCM LOS	B			B			D			D		

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1	
Vol Left, %		1%	30%	37%	0%	4%
Vol Thru, %		85%	68%	63%	0%	64%
Vol Right, %		13%	2%	0%	100%	32%
Sign Control		Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane		474	168	59	20	453
LT Vol		6	51	22	0	18
Through Vol		405	114	37	0	291
RT Vol		63	3	0	20	144
Lane Flow Rate		527	187	66	22	503
Geometry Grp		2	5	7	7	2
Degree of Util (X)		0.845	0.374	0.148	0.045	0.8
Departure Headway (Hd)		5.777	7.217	8.131	7.216	5.721
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes
Cap		627	498	441	495	634
Service Time		3.819	5.275	5.896	4.981	3.763
HCM Lane V/C Ratio		0.841	0.376	0.15	0.044	0.793
HCM Control Delay		32.5	14.6	12.3	10.3	27.8
HCM Lane LOS		D	B	B	B	D
HCM 95th-tile Q		9.2	1.7	0.5	0.1	8

Intersection

Intersection Delay, s/veh 151.4
 Intersection LOS F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	102	51	1	79	227	12	9	302	8	16	457	251
Future Vol, veh/h	102	51	1	79	227	12	9	302	8	16	457	251
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	113	57	1	88	252	13	10	336	9	18	508	279
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			1		
HCM Control Delay	20.2			36.7			32.5			282.1		
HCM LOS	C			E			D			F		

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1
Vol Left, %	3%	66%	26%	0%	2%
Vol Thru, %	95%	33%	74%	0%	63%
Vol Right, %	3%	1%	0%	100%	35%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	319	154	306	12	724
LT Vol	9	102	79	0	16
Through Vol	302	51	227	0	457
RT Vol	8	1	0	12	251
Lane Flow Rate	354	171	340	13	804
Geometry Grp	2	5	7	7	2
Degree of Util (X)	0.74	0.411	0.773	0.027	1.564
Departure Headway (Hd)	8.644	10.228	9.403	8.538	7
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	422	354	388	422	523
Service Time	6.644	8.228	7.103	6.238	5.062
HCM Lane V/C Ratio	0.839	0.483	0.876	0.031	1.537
HCM Control Delay	32.5	20.2	37.7	11.5	282.1
HCM Lane LOS	D	C	E	B	F
HCM 95th-tile Q	5.9	1.9	6.4	0.1	42.9

Intersection

Intersection Delay, s/veh 55.2
 Intersection LOS F


















Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	79	161	3	28	65	20	6	415	73	18	297	160
Future Vol, veh/h	79	161	3	28	65	20	6	415	73	18	297	160
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	88	179	3	31	72	22	7	461	81	20	330	178
Number of Lanes	0	1	0	0	1	1	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			1		
HCM Control Delay	22.6			14.5			76.7			59.2		
HCM LOS	C			B			F			F		

Lane	NBLn1	EBLn1	WBLn1	WBLn2	SBLn1
Vol Left, %	1%	33%	30%	0%	4%
Vol Thru, %	84%	66%	70%	0%	63%
Vol Right, %	15%	1%	0%	100%	34%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	494	243	93	20	475
LT Vol	6	79	28	0	18
Through Vol	415	161	65	0	297
RT Vol	73	3	0	20	160
Lane Flow Rate	549	270	103	22	528
Geometry Grp	2	5	7	7	2
Degree of Util (X)	1.042	0.598	0.26	0.05	0.975
Departure Headway (Hd)	6.837	8.171	9.27	8.384	6.82
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	532	445	390	430	538
Service Time	4.837	6.171	6.97	6.084	4.82
HCM Lane V/C Ratio	1.032	0.607	0.264	0.051	0.981
HCM Control Delay	76.7	22.6	15.2	11.5	59.2
HCM Lane LOS	F	C	C	B	F
HCM 95th-tile Q	15.8	3.8	1	0.2	13.1

Lanes, Volumes, Timings
1: Rolesville Road & Mitchell Mill Road

Combined (2026) AM - with Improvements

05/25/2019

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	102	51	1	79	227	12	9	302	8	16	457	251
Future Volume (vph)	102	51	1	79	227	12	9	302	8	16	457	251
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		325	0		0	0		0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999				0.850		0.997			0.953	
Flt Protected		0.968			0.987			0.999			0.999	
Satd. Flow (prot)	0	1801	0	0	1839	1583	0	1855	0	0	1773	0
Flt Permitted		0.448			0.874			0.975			0.988	
Satd. Flow (perm)	0	834	0	0	1628	1583	0	1811	0	0	1754	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1309			1418			1475			1783	
Travel Time (s)		19.8			21.5			22.3			27.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	113	57	1	88	252	13	10	336	9	18	508	279
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	171	0	0	340	13	0	355	0	0	805	0
Turn Type	Perm	NA		Perm	NA	Free	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		Free	2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		12.0	12.0		12.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		19.0	19.0		19.0	19.0	
Total Split (s)	22.0	22.0		22.0	22.0		38.0	38.0		38.0	38.0	
Total Split (%)	36.7%	36.7%		36.7%	36.7%		63.3%	63.3%		63.3%	63.3%	
Maximum Green (s)	15.0	15.0		15.0	15.0		31.0	31.0		31.0	31.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		-2.0			-2.0			-2.0			-2.0	
Total Lost Time (s)		5.0			5.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)		16.0			16.0	56.1		29.9			29.9	
Actuated g/C Ratio		0.29			0.29	1.00		0.53			0.53	
v/c Ratio		0.72			0.73	0.01		0.37			0.86	
Control Delay		40.2			30.7	0.0		9.0			23.3	
Queue Delay		0.0			0.0	0.0		0.0			0.0	
Total Delay		40.2			30.7	0.0		9.0			23.3	
LOS		D			C	A		A			C	
Approach Delay		40.2			29.6			9.0			23.3	
Approach LOS		D			C			A			C	

Lanes, Volumes, Timings
 1: Rolesville Road & Mitchell Mill Road

Combined (2026) AM - with Improvements

05/25/2019

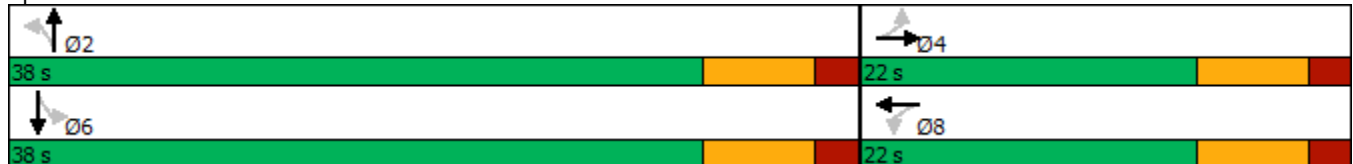
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		55			110	0		65			218	
Queue Length 95th (ft)		#145			#224	0		111			#442	
Internal Link Dist (ft)		1229			1338			1395			1703	
Turn Bay Length (ft)						325						
Base Capacity (vph)		257			502	1583		1085			1050	
Starvation Cap Reductn		0			0	0		0			0	
Spillback Cap Reductn		0			0	0		0			0	
Storage Cap Reductn		0			0	0		0			0	
Reduced v/c Ratio		0.67			0.68	0.01		0.33			0.77	

Intersection Summary

Area Type: Other
 Cycle Length: 60
 Actuated Cycle Length: 56.1
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 23.3
 Intersection Capacity Utilization 83.9%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Intersection LOS: C
 ICU Level of Service E


















Splits and Phases: 1: Rolesville Road & Mitchell Mill Road



Lanes, Volumes, Timings
1: Rolesville Road & Mitchell Mill Road

Combined (2026) PM - with Improvements

05/25/2019

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	79	161	3	28	65	20	6	415	73	18	297	160
Future Volume (vph)	79	161	3	28	65	20	6	415	73	18	297	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		325	0		0	0		0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.980			0.954	
Flt Protected		0.984			0.985			0.999			0.998	
Satd. Flow (prot)	0	1829	0	0	1835	1583	0	1824	0	0	1774	0
Flt Permitted		0.852			0.863			0.992			0.970	
Satd. Flow (perm)	0	1584	0	0	1608	1583	0	1811	0	0	1724	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1309			1418			1475			1783	
Travel Time (s)		19.8			21.5			22.3			27.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	88	179	3	31	72	22	7	461	81	20	330	178
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	270	0	0	103	22	0	549	0	0	528	0
Turn Type	Perm	NA		Perm	NA	Free	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		Free	2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		12.0	12.0		12.0	12.0	
Minimum Split (s)	14.0	14.0		14.0	14.0		19.0	19.0		19.0	19.0	
Total Split (s)	23.0	23.0		23.0	23.0		37.0	37.0		37.0	37.0	
Total Split (%)	38.3%	38.3%		38.3%	38.3%		61.7%	61.7%		61.7%	61.7%	
Maximum Green (s)	16.0	16.0		16.0	16.0		30.0	30.0		30.0	30.0	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		-2.0			-2.0			-2.0			-2.0	
Total Lost Time (s)		5.0			5.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)		14.5			13.5	47.2		22.3			22.3	
Actuated g/C Ratio		0.31			0.29	1.00		0.47			0.47	
v/c Ratio		0.56			0.22	0.01		0.64			0.65	
Control Delay		20.1			15.4	0.0		13.7			14.0	
Queue Delay		0.0			0.0	0.0		0.0			0.0	
Total Delay		20.1			15.4	0.0		13.7			14.0	
LOS		C			B	A		B			B	
Approach Delay		20.1			12.7			13.7			14.0	
Approach LOS		C			B			B			B	

Lanes, Volumes, Timings
 1: Rolesville Road & Mitchell Mill Road

Combined (2026) PM - with Improvements

05/25/2019

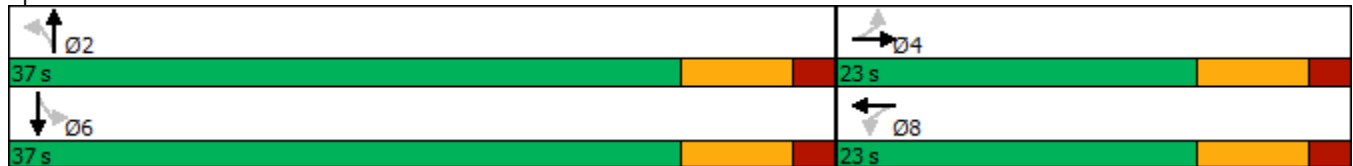
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		57			19	0		104			100	
Queue Length 95th (ft)		148			60	0		202			198	
Internal Link Dist (ft)		1229			1338			1395			1703	
Turn Bay Length (ft)						325						
Base Capacity (vph)		626			636	1583		1273			1212	
Starvation Cap Reductn		0			0	0		0			0	
Spillback Cap Reductn		0			0	0		0			0	
Storage Cap Reductn		0			0	0		0			0	
Reduced v/c Ratio		0.43			0.16	0.01		0.43			0.44	

Intersection Summary

Area Type: Other
 Cycle Length: 60
 Actuated Cycle Length: 47.2
 Natural Cycle: 45
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.65
 Intersection Signal Delay: 14.9
 Intersection Capacity Utilization 64.7%
 Analysis Period (min) 15

Intersection LOS: B
 ICU Level of Service C

Splits and Phases: 1: Rolesville Road & Mitchell Mill Road



APPENDIX E

CAPACITY ANALYSIS CALCULATIONS

ROLESVILLE ROAD

&

FOWLER ROAD

HCM 6th TWSC
2: Rolesville Road & Fowler Road

Existing (2019) AM
05/23/2019

Intersection




Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	R	T	R	L	T
Traffic Vol, veh/h	3	73	305	1	45	304
Future Vol, veh/h	3	73	305	1	45	304
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	81	339	1	50	338

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	778	340	0	0	340
Stage 1	340	-	-	-	-
Stage 2	438	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	365	702	-	-	1219
Stage 1	721	-	-	-	-
Stage 2	651	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	347	702	-	-	1219
Mov Cap-2 Maneuver	347	-	-	-	-
Stage 1	721	-	-	-	-
Stage 2	618	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.1	0	1
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	675	1219
HCM Lane V/C Ratio	-	-	0.125	0.041
HCM Control Delay (s)	-	-	11.1	8.1
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.4	0.1

Intersection

Int Delay, s/veh	1.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	44	225	2	49	170
Future Vol, veh/h	3	44	225	2	49	170
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	49	250	2	54	189

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	548	251	0	0	252
Stage 1	251	-	-	-	-
Stage 2	297	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	497	788	-	-	1313
Stage 1	791	-	-	-	-
Stage 2	754	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	474	788	-	-	1313
Mov Cap-2 Maneuver	474	-	-	-	-
Stage 1	791	-	-	-	-
Stage 2	719	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	10.1	0	1.8
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	756	1313
HCM Lane V/C Ratio	-	-	0.069	0.041
HCM Control Delay (s)	-	-	10.1	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1

Intersection

Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	84	424	1	52	657
Future Vol, veh/h	3	84	424	1	52	657
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	93	471	1	58	730

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1318	472	0	0	472
Stage 1	472	-	-	-	-
Stage 2	846	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	173	592	-	-	1090
Stage 1	628	-	-	-	-
Stage 2	421	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	157	592	-	-	1090
Mov Cap-2 Maneuver	157	-	-	-	-
Stage 1	628	-	-	-	-
Stage 2	383	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.1	0	0.6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	540	1090
HCM Lane V/C Ratio	-	-	0.179	0.053
HCM Control Delay (s)	-	-	13.1	8.5
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.6	0.2

Intersection

Int Delay, s/veh	1.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	51	453	2	56	427
Future Vol, veh/h	3	51	453	2	56	427
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	57	503	2	62	474

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1102	504	0	0	505
Stage 1	504	-	-	-	-
Stage 2	598	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	234	568	-	-	1060
Stage 1	607	-	-	-	-
Stage 2	549	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	216	568	-	-	1060
Mov Cap-2 Maneuver	216	-	-	-	-
Stage 1	607	-	-	-	-
Stage 2	506	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.8	0	1
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	521	1060
HCM Lane V/C Ratio	-	-	0.115	0.059
HCM Control Delay (s)	-	-	12.8	8.6
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.4	0.2

Intersection

Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	84	510	1	52	685
Future Vol, veh/h	3	84	510	1	52	685
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	93	567	1	58	761

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1445	568	0	0	568
Stage 1	568	-	-	-	-
Stage 2	877	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	145	522	-	-	1004
Stage 1	567	-	-	-	-
Stage 2	407	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	131	522	-	-	1004
Mov Cap-2 Maneuver	131	-	-	-	-
Stage 1	567	-	-	-	-
Stage 2	366	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	14.6	0	0.6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	473	1004
HCM Lane V/C Ratio	-	-	0.204	0.058
HCM Control Delay (s)	-	-	14.6	8.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.8	0.2

Intersection

Int Delay, s/veh	1.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	51	509	2	56	522
Future Vol, veh/h	3	51	509	2	56	522
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	57	566	2	62	580

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1271	567	0	0	568
Stage 1	567	-	-	-	-
Stage 2	704	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	185	523	-	-	1004
Stage 1	568	-	-	-	-
Stage 2	490	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	168	523	-	-	1004
Mov Cap-2 Maneuver	168	-	-	-	-
Stage 1	568	-	-	-	-
Stage 2	445	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.8	0	0.9
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	468	1004
HCM Lane V/C Ratio	-	-	0.128	0.062
HCM Control Delay (s)	-	-	13.8	8.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.4	0.2

APPENDIX F

CAPACITY ANALYSIS CALCULATIONS

ROLESVILLE ROAD

&

TAVISWOOD WAY

HCM 6th TWSC
3: Rolesville Road & Taviswood Way

Existing (2019) AM
05/23/2019

Intersection

Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔		↔	↔
Traffic Vol, veh/h	4	4	287	1	2	328
Future Vol, veh/h	4	4	287	1	2	328
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	4	319	1	2	364

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	688	320	0	0	320
Stage 1	320	-	-	-	-
Stage 2	368	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	412	721	-	-	1240
Stage 1	736	-	-	-	-
Stage 2	700	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	411	721	-	-	1240
Mov Cap-2 Maneuver	411	-	-	-	-
Stage 1	736	-	-	-	-
Stage 2	699	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	524	1240
HCM Lane V/C Ratio	-	-	0.017	0.002
HCM Control Delay (s)	-	-	12	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.1	0

HCM 6th TWSC
3: Rolesville Road & Taviswood Way

Existing (2019) PM
05/23/2019

Intersection

Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	1	3	240	4	1	191
Future Vol, veh/h	1	3	240	4	1	191
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	3	267	4	1	212

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	483	269	0	0	271
Stage 1	269	-	-	-	-
Stage 2	214	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	542	770	-	-	1292
Stage 1	776	-	-	-	-
Stage 2	822	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	541	770	-	-	1292
Mov Cap-2 Maneuver	541	-	-	-	-
Stage 1	776	-	-	-	-
Stage 2	821	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	10.2	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	696	1292
HCM Lane V/C Ratio	-	-	0.006	0.001
HCM Control Delay (s)	-	-	10.2	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

HCM 6th TWSC
3: Rolesville Road & Taviswood Way

Background (2026) AM
05/23/2019

Intersection

Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	5	5	404	1	2	685
Future Vol, veh/h	5	5	404	1	2	685
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	6	449	1	2	761

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1215	450	0	0	450
Stage 1	450	-	-	-	-
Stage 2	765	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	200	609	-	-	1110
Stage 1	642	-	-	-	-
Stage 2	459	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	200	609	-	-	1110
Mov Cap-2 Maneuver	200	-	-	-	-
Stage 1	642	-	-	-	-
Stage 2	458	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	17.4	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	301	1110
HCM Lane V/C Ratio	-	-	0.037	0.002
HCM Control Delay (s)	-	-	17.4	8.2
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	0.1	0

Intersection

Int Delay, s/veh 0.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	1	3	471	5	1	451
Future Vol, veh/h	1	3	471	5	1	451
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	3	523	6	1	501

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	1029	526	0
Stage 1	526	-	-
Stage 2	503	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	259	552	-
Stage 1	593	-	-
Stage 2	607	-	-
Platoon blocked, %			
Mov Cap-1 Maneuver	259	552	-
Mov Cap-2 Maneuver	259	-	-
Stage 1	593	-	-
Stage 2	606	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.5	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	430	1038
HCM Lane V/C Ratio	-	-	0.01	0.001
HCM Control Delay (s)	-	-	13.5	8.5
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

HCM 6th TWSC
3: Rolesville Road & Taviswood Way

Combined (2026) AM
05/23/2019

Intersection

Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔		↔	↔
Traffic Vol, veh/h	5	5	415	1	2	720
Future Vol, veh/h	5	5	415	1	2	720
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	6	461	1	2	800

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1266	462	0	0	462
Stage 1	462	-	-	-	-
Stage 2	804	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	187	600	-	-	1099
Stage 1	634	-	-	-	-
Stage 2	440	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	187	600	-	-	1099
Mov Cap-2 Maneuver	187	-	-	-	-
Stage 1	634	-	-	-	-
Stage 2	439	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	18.1	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	285	1099
HCM Lane V/C Ratio	-	-	0.039	0.002
HCM Control Delay (s)	-	-	18.1	8.3
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	0.1	0

Intersection

Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	1	3	509	5	1	473
Future Vol, veh/h	1	3	509	5	1	473
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	3	566	6	1	526

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1097	569	0	0	572
Stage 1	569	-	-	-	-
Stage 2	528	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	236	522	-	-	1001
Stage 1	566	-	-	-	-
Stage 2	592	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	236	522	-	-	1001
Mov Cap-2 Maneuver	236	-	-	-	-
Stage 1	566	-	-	-	-
Stage 2	591	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	14.1	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	401	1001
HCM Lane V/C Ratio	-	-	0.011	0.001
HCM Control Delay (s)	-	-	14.1	8.6
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

APPENDIX G

CAPACITY ANALYSIS CALCULATIONS

ROLESVILLE ROAD

&

SITE DRIVE 1

Intersection

Int Delay, s/veh	2.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↑		↔	↑
Traffic Vol, veh/h	35	86	404	11	28	685
Future Vol, veh/h	35	86	404	11	28	685
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	96	449	12	31	761

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1278	455	0	0	461
Stage 1	455	-	-	-	-
Stage 2	823	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	183	605	-	-	1100
Stage 1	639	-	-	-	-
Stage 2	431	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	178	605	-	-	1100
Mov Cap-2 Maneuver	178	-	-	-	-
Stage 1	639	-	-	-	-
Stage 2	419	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	21	0	0.3
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	357	1100
HCM Lane V/C Ratio	-	-	0.377	0.028
HCM Control Delay (s)	-	-	21	8.4
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	1.7	0.1

Intersection

Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↑		↔	↑
Traffic Vol, veh/h	22	56	471	38	95	451
Future Vol, veh/h	22	56	471	38	95	451
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	62	523	42	106	501

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1257	544	0	0	565
Stage 1	544	-	-	-	-
Stage 2	713	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	189	539	-	-	1007
Stage 1	582	-	-	-	-
Stage 2	486	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	169	539	-	-	1007
Mov Cap-2 Maneuver	169	-	-	-	-
Stage 1	582	-	-	-	-
Stage 2	435	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	19.6	0	1.6
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	333	1007
HCM Lane V/C Ratio	-	-	0.26	0.105
HCM Control Delay (s)	-	-	19.6	9
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	1	0.3

APPENDIX H

CAPACITY ANALYSIS CALCULATIONS

MITCHELL MILL ROAD

&

SITE DRIVE 2

Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	4		4	
Traffic Vol, veh/h	17	58	266	4	4	52
Future Vol, veh/h	17	58	266	4	4	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	19	64	296	4	4	58

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	300	0	400
Stage 1	-	-	298
Stage 2	-	-	102
Critical Hdwy	4.12	-	6.42
Critical Hdwy Stg 1	-	-	5.42
Critical Hdwy Stg 2	-	-	5.42
Follow-up Hdwy	2.218	-	3.518
Pot Cap-1 Maneuver	1261	-	606
Stage 1	-	-	753
Stage 2	-	-	922
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1261	-	596
Mov Cap-2 Maneuver	-	-	596
Stage 1	-	-	741
Stage 2	-	-	922

Approach	EB	WB	SB
HCM Control Delay, s	1.8	0	10.4
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1261	-	-	-	728
HCM Lane V/C Ratio	0.015	-	-	-	0.085
HCM Control Delay (s)	7.9	0	-	-	10.4
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.3

Intersection

Int Delay, s/veh	2.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Traffic Vol, veh/h	57	195	78	4	4	34
Future Vol, veh/h	57	195	78	4	4	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	63	217	87	4	4	38

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	91	0	-	0	432 89
Stage 1	-	-	-	-	89 -
Stage 2	-	-	-	-	343 -
Critical Hdwy	4.12	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.218	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1504	-	-	-	581 969
Stage 1	-	-	-	-	934 -
Stage 2	-	-	-	-	719 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1504	-	-	-	553 969
Mov Cap-2 Maneuver	-	-	-	-	553 -
Stage 1	-	-	-	-	889 -
Stage 2	-	-	-	-	719 -

Approach	EB	WB	SB
HCM Control Delay, s	1.7	0	9.2
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1504	-	-	-	898
HCM Lane V/C Ratio	0.042	-	-	-	0.047
HCM Control Delay (s)	7.5	0	-	-	9.2
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1