

- 1. THIS ALTA/NSPS LAND TITLE SURVEY WAS PREPARED FOR THE BENEFIT OF WALLBROOK LANDCO, LLC, ITS SUCCESSORS AND/OR ASSIGNS AS THEIR INTERESTS MAY APPEAR, AND INVESTORS TITLE INSURANCE COMPANY.
- 2. THE PROPERTY AS SHOWN HEREON IS BASED ON A FIELD-RUN BOUNDARY SURVEY WITH A RAW CLOSURE OF 1:34,600.
- 3. THE IMPROVEMENTS SHOWN HEREON ARE BASED ON A FIELD—RUN PLANIMETRIC SURVEY PERFORMED BY JOHNSON, MIRMIRAN & THOMPSON FROM DECEMBER 2019 THROUGH MARCH 2020 AND REFLECTS SITE CONDITIONS AS OF THAT DATE.
- 4. ELEVATIONS ARE BASED ON NAVD88 DATUM.
- 5. THE SURVEY IS REFERENCED TO THE NORTH CAROLINA STATE PLANE COORDINATE SYSTEM (NCSPCS), NORTH AMERICAN DATUM, 1983, 2001 ADJUSTMENT, NAD83(2001).
- 6. THE USE OF THE WORD CERTIFY OR CERTIFICATION CONSTITUTES AN EXPRESSION OF PROFESSIONAL OPINION REGARDING THOSE FACTS OR FINDINGS WHICH ARE THE SUBJECT OF THE UNDERSIGNED PROFESSIONAL'S KNOWLEDGE, INFORMATION AND BELIEF, AND IN ACCORDANCE WITH THE COMMONLY ACCEPTED PROCEDURE CONSISTENT WITH THE APPLICABLE STANDARDS OF PRACTICE AND DOES NOT CONSTITUTE A WARRANTY OR GUARANTEE EITHER EXPRESSED OR IMPLIED.
- 7. THE SUBJECT PROPERTY IS LOCATED IN FLOOD ZONE X, AREA OF MINIMAL FLOODING, AS SHOWN ON NATIONAL FLOOD INSURANCE RATE MAP (FIRM), WAKE COUNTY, NORTH CAROLINA, PANEL 1758, MAP NO. 3720175800J, EFFECTIVE DATE: MAY 2, 2006.
- 8. AT THE TIME OF THE SURVEY, THERE WERE NO PARKING SPACES.
- 9. AT THE TIME OF THE SURVEY, THERE WAS NO OBSERVABLE EVIDENCE OF THE SITE BEING USED AS A SOLID WASTE DUMP, SUMP OR LANDFILL.
- 10. AT THE TIME OF THE SURVEY, THERE WAS NO OBSERVABLE EVIDENCE OF A CEMETERY.
- 11. AT THE TIME OF THE SURVEY, THERE WAS NO OBSERVABLE EVIDENCE OF BUILDING CONSTRUCTION OR BUILDING ADDITIONS.

AREA TABULATION

PARCEL	PIN#	NET (AC.)	GROSS (AC.)
Α	1758-48-9229	0.828	0.828
В	1758-58-2090	10.742	11.168
С	1758-56-8976	42.324	44.100
D	1758-45-8905	15.024	15.024
	TOTALS:	68.918	71.120

RECORD LEGAL DESCRIPTIONS FOR TAX PARCELS 1758-48-9229 & 1758-58-2090:

PER INVESTORS TITLE INSURANCE COMPANY, TITLE COMMITMENT NO. 201800776CA2, WITH AN EFFECTIVE DATE OF SEPTEMBER 14, 2018 AT 5:00 P.M.

IN THE STATE OF NC. COUNTY OF WAKE,

1582. WAKE COUNTY REGISTRY.

PARCEL ONE (REID #: 0224145) (PIN #1758-58-2090) (PARCEL 'B'):
BEING LOT 2-3, CONTAINING 10.723 NET ACRES, AS THE SAME IS

SHOWN ON THAT PLAT RECORDED IN BOOK OF MAPS 1996, PAGE

PARCEL TWO (REID #: 0092211) (PIN #1758-48-9229) (PARCEL 'A'): BEING THAT PARCEL DESCRIBED AS FOLLOWS:

BEGINNING AT A STAKE IN THE EDGE OF THE ROLESVILLE—WALKERS CROSSROAD ROAD, CORNER OF O. V. WIGGINS; THENCE IN A SOUTHWESTERN DIRECTION ABOUT 300 FEET TO A STAKE AND W. H. MARSHALL'S LINE; THENCE IN A NORTHWESTERN DIRECTION 246 FEET WITH MARSHALL'S LINE TO THE EDGE OF THE ROAD; THENCE IN AN EASTERN DIRECTION WITH SAID ROAD ABOUT 410 FEET TO THE POINT OF BEGINNING, CONTAINING # OF AN ACRE BY ESTIMATION. BEING A PART OF THE LAND FORMERLY OWNED BY JAMES WALL.

RECORD LEGAL DESCRIPTIONS FOR TAX PARCELS 1758-56-8976 & 1758-45-8905:

PER INVESTORS TITLE INSURANCE COMPANY, TITLE COMMITMENT NO. 201800751CA2, WITH AN EFFECTIVE DATE OF SEPTEMBER 10, 2018 AT 5:00 P.M.

IN THE STATE OF NC, COUNTY OF WAKE,

TRACT 1 (PIN #1758-56-8976) (PARCEL 'C'):

BEING ALL OF TRACT 2A (2.894 ACRES) AND TRACT 3 (41.221 ACRES) AS SHOWN ON PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY" DATED NOVEMBER 17, 1995, PREPARED BY W. GRAHAM HAWTHORNE, JR., RLS AND RECORDED IN BOOK OF MAPS 1995, PAGE 2034, WAKE COUNTY REGISTRY.

LESS AND EXCEPT FROM TRACT 1 THAT 0.175 ACRE PORTION AS CONVEYED TO THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION IN DEED RECORDED IN BOOK 14395, PAGE 2080, WAKE COUNTY REGISTRY.

TRACT 2 (PIN #1758-45-8905) (PARCEL 'D'):

BEING ALL OF TRACT 2 (15.057 ACRE) AS SHOWN ON PLAT ENTITLED "RECOMBINATION MAP OF BOBBY L. MURRAY TRUST, TRACTS 1 & 2" DATED NOVEMBER 7, 2002, PREPARED BY MICHAEL D. GOODFRED, RLS, AND RECORDED IN BOOK OF MAPS 2005, PAGES 1195 AND 1196, WAKE COUNTY REGISTRY.

LESS AND EXCEPT FROM TRACT 2 THAT 0.03 ACRE PORTION AS CONVEYED TO CARLTON GROUP OF NORTH CAROLINA, LLC IN DEED RECORDED IN BOOK 13993, PAGE 2591, WAKE COUNTY REGISTRY.

SCHEDULE B. PART II EXCEPTIONS:

PER INVESTORS TITLE INSURANCE COMPANY, TITLE COMMITMENT NO. 201800776CA2, WITH AN EFFECTIVE DATE OF SEPTEMBER 14, 2018 AT 5:00 P.M.

- 1. (ITEM 3) MATTERS SHOWN ON RECORDED BOOK OF MAPS 1996 AT PAGE 1582 SHOWS THE FOLLOWING LOCATED ON THE LAND:
- (a) OVERHEAD LINES [PLOTTED HEREON]
 (b) POWER POLE [PLOTTED HEREON]
- (c) RIGHT OF WAY FOR U.S. HWY 401 LOUISBURG ROAD [PLOTTED HEREON]
- 2. (ITEM 4) EASEMENT(S) AND/OR RIGHT(S) OF WAY RECORDED IN BOOK 3868 AT PAGE 917 (PARCEL ONE). [PLOTTED HEREON]

SCHEDULE B. PART II EXCEPTIONS:

PER INVESTORS TITLE INSURANCE COMPANY, TITLE COMMITMENT NO. 201800751CA2, WITH AN EFFECTIVE DATE OF SEPTEMBER 10, 2018 AT 5:00 P.M.

- 3. (ITEM 2) TITLE TO THAT PORTION OF THE LAND WITHIN THE RIGHT-OF-WAY OF U.S. HIGHWAY 401 (LOUISBURG ROAD). [PLOTTED HEREON]
- 4. (ITEM 3) RIGHTS OF OTHERS THERETO ENTITLED IN AND TO THE CONTINUED UNINTERRUPTED FLOW OF THE CREEK, LOCATED ON THE LAND. [CREEK LOCATION PLOTTED HEREON]
- 5. (ITEM 4) ELECTRIC LINE RIGHT-OF-WAY EASEMENT TO WAKE ELECTRIC MEMBERSHIP CORPORATION RECORDED IN BOOK 3868 AT PAGE 917. [PLOTTED HEREON]
- AS TO TRACT 1 ONLY (PIN #1758-56-8976):
- 6. (ITEM 8) SUBJECT TO MATTERS SHOWN ON RECORDED BOOK OF MAPS 1995 AT PAGE 2034 SHOWS THE FOLLOWING LOCATED ON THE LAND:

(a) OVERHEAD LINE [PLOTTED HEREON]
(b) POWER POLE [PLOTTED HEREON]

- 7. (ITEM 9) EASEMENT(S) TO WAKE ELECTRIC MEMBERSHIP CORPORATION RECORDED IN BOOK 863 AT PAGES 211 AND 212. [BLANKET EASEMENT EXISTING ELECTRIC LINES PLOTTED HEREON]
- 8. (ITEM 10) RURAL LINE PERMIT TO CAROLINA TELEPHONE AND TELEGRAPH COMPANY RECORDED IN BOOK 1338 AT PAGES 143 AND 145. [LOCATIONS CANNOT BE DETERMINED FROM THE RECORD DOCUMENT, EXISTING POLES AND GUY WIRES PLOTTED HEREON]
- 9. (ITEM 11) SLOPE EASEMENT RECORDED IN BOOK 14395 AT PAGE 2087 AS SHOWN ON PLAT RECORDED IN BOOK OF MAPS 2011 AT PAGE 383. [PLOTTED HEREON]
- AS TO TRACT 2 ONLY (PIN #1758-45-8905):
- 10. SUBJECT TO MATTERS SHOWN ON RECORDED BOOK OF MAPS 2005 AT PAGES 1195 AND 1196 SHOWS THE FOLLOWING LOCATED ON THE LAND:
 - (a) UNDERGROUND SEWER MAIN MARKER [NOT FOUND]
 - (b) POWER BOX [NOT FOUND]
- (c) RIGHTS OF OTHERS IN AND TO THE USE OF THE SOIL PATH (TO BE ABANDONED) [PATH NO LONGER VISIBLE]
- 11. (ITEM 13) DEED OF EASEMENT WITH GENERAL WARRANTY FOR WATERLINE EASEMENT TEMPORARY CONSTRUCTION EASEMENT RECORDED IN BOOK 16679 AT PAGE 132. [WATERLINE EASEMENT PLOTTED HEREON, TEMPORARY CONSTRUCTION EASEMENT NOT PLOTTED]
- 12. (ITEM 14) SANITARY SEWER EASEMENT RECORDED IN BOOK 10071 AT PAGE 2179 AS SHOWN ON PLAT RECORDED IN BOOK OF MAPS 2003 AT PAGE 647. [PLOTTED HEREON]
- 13. (ITEM 15) RESERVATION OF EASEMENT RECORDED IN BOOK 13993 AT PAGE 2591. [PLOTTED HEREON]
- 14. (ITEM 16) TITLE TO THAT PORTION OF THE LAND WITHIN THE RIGHT-OF-WAY OF S.R. 2226. [RIGHT OF WAY OF S.R. 2226 PLOTTED HEREON PROPERTY DOES NOT EXTEND INTO RIGHT OF WAY]
- 15. (ITEM 17) RIGHT OF WAY AGREEMENT TO STATE HIGHWAY COMMISSION RECORDED IN BOOK 2052 AT PAGE 545. [RIGHT OF WAY OF S.R. 2226 PLOTTED HEREON]
- 16. (ITEM 18) RIGHTS OF OTHERS THERETO ENTITLED IN AND TO THE CONTINUED UNINTERRUPTED FLOW OF BRANCH/CREEK, LOCATED ON THE LAND. [BRANCH/CREEK LOCATION PLOTTED HEREON]

NEW LEGAL DESCRIPTIONS

PARCEL 'A' (PIN #1758-48-9229)

ALL THAT CERTAIN REAL PROPERTY SITUATED IN THE TOWN OF ROLESVILLE, COUNTY OF WAKE, STATE OF NORTH CAROLINA, DESCRIBED AS FOLLOWS:

BEGINNING AT A FOUND IRON PIPE MARKING THE NORTHWEST CORNER OF LOT 2-3 AS SAID LOT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY, TRACTS 2-2 & 2-3, TWITTY PROP.", BY W. GRAHAM CAWTHORNE, JR., RLS, DATED NOVEMBER 17, 1995, REVISED JANUARY 12, 1996 AND RECORDED IN BOOK OF MAPS 1996, PAGE 1582, WAKE COUNTY RECORDS, SAID PIPE ALSO BEING ON THE EAST LINE OF TRACT #3 AS SAID TRACT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "W. H. MARSHALL ESTATE", BY C. W. RUSSUM, RLS, DATED JULY, 1961 AND RECORDED IN BOOK OF MAPS 1961, PAGE 97, WAKE COUNTY RECORDS; THENCE ALONG SAID EAST LINE OF TRACT 3 NO1*11'04"W 240.24' TO AN IRON PIPE ON THE SOUTHWEST RIGHT OF WAY LINE OF BURLINGTON MILLS ROAD (S.R. 2051); THENCE ALONG SAID SOUTHWEST RIGHT OF WAY LINE THE FOLLOWING FOUR COURSES: (1) S47°12'51"E 130.81'; (2) ALONG THE ARC OF A TANGENT CURVE TO THE LEFT, CONCAVE TO THE NORTHEAST, HAVING A RADIUS OF 600.00', THROUGH A CENTRAL ANGLE OF 26'22'04", AN ARC LENGTH OF 276.12' AND BEING SUBTENDED BY A CHORD BEARING S60°23'53"E 273.69'; (3) \$75'02'10"E 50.12'; AND (4) \$77'31'31"E 22.85' TO THE NORTH LINE OF AFORESAID LOT 2-3; THENCE ALONG SAID NORTH LINE OF LOT 2-3 N89'45'02"W 399.74' TO THE POINT OF BEGINNING.

CONTAINING 0.828 ACRES, MORE OR LESS.

PARCEL 'B' (PIN #1758-58-2090)

ALL THAT CERTAIN REAL PROPERTY SITUATED IN THE TOWN OF ROLESVILLE, COUNTY OF WAKE, STATE OF NORTH CAROLINA, DESCRIBED AS FOLLOWS:

BEGINNING AT A FOUND IRON PIPE MARKING THE SOUTHWEST CORNER

OF PARCEL 2 AS SAID PARCEL IS DESCRIBED IN DEED BOOK 15498, PAGE 1302, WAKE COUNTY RECORDS; THENCE ALONG THE SOUTH LINE OF SAID PARCEL 2 S89°45'02"E 399.74' TO THE SOUTHWEST RIGHT OF WAY LINE OF BURLINGTON MILLS ROAD (S.R. 2051); THENCE CONTINUING S89'45'02"E 200.72' TO THE NORTHWEST CORNER OF LOT 2-4 AS SAID LOT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY, TRACTS 2-2 & 2-3, TWITTY PROP.". BY W. GRAHAM CAWTHORNE, JR., RLS, DATED NOVEMBER 17, 1995, REVISED JANUARY 12, 1996 AND RECORDED IN BOOK OF MAPS 1996. PAGE 1582. WAKE COUNTY RECORDS; THENCE ALONG THE SOUTHWEST LINE OF SAID LOT 2-4 S37'53'38"E 39.01' TO A FOUND BENT IRON PIPE AT AN ANGLE POINT IN THE SOUTH RIGHT OF WAY LINE OF BURLINGTON MILLS ROAD; THENCE CONTINUING \$37.53.38"E 454.70' TO A FOUND IRON PIPE AT AN ANGLE POINT IN THE NORTHWEST RIGHT OF WAY LINE OF LOUISBURG ROAD (U.S. 401); THENCE CONTINUING S37°53'38"E 45.31' TO THE CENTERLINE OF LOUISBURG ROAD: THENCE ALONG THE CENTERLINE OF SAID ROAD, ALONG THE ARC OF A CURVE TO THE LEFT, CONCAVE TO THE SOUTHEAST, HAVING A RADIUS OF 3,750.00' THROUGH A CENTRAL ANGLE OF 08°13'06", AN ARC LENGTH OF 537.89' AND BEING SUBTENDED BY A CHORD BEARING \$41*12'09"W 537.43': THENCE LEAVING SAID CENTERLINE, ALONG THE NORTHEAST LINE OF LOTS 1 AND 2 AS SAID LOTS ARE SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "SURVEY FOR GRAND PARK PROPERTIES. LOTS 1 AND 2", BY CAWTHORNE, MOSS & PANCIERA, P.C., DATED OCTOBER 7, 1998 AND RECORDED IN BOOK OF MAPS 1999, PAGE 1039, WAKE COUNTY RECORDS, N49°16'09"W 479.09' TO A FOUND IRON PIPE MARKING AN ANGLE POINT IN SAID LOT 2; THENCE CONTINUING ALONG THE NORTHEAST LINE OF SAID LOT 2 N66'40'58"W 215.40' TO A FOUND IRON PIPE AT THE NORTHWEST CORNER THEREOF, SAID PIPE ALSO BEING ON THE EAST LINE OF TRACT #3 AS SAID TRACT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "W. H. MARSHALL ESTATE", BY C. W. RUSSUM, RLS, DATED JULY, 1961 AND RECORDED IN BOOK OF MAPS 1961, PAGE 97, WAKE COUNTY RECORDS: THENCE ALONG SAID EAST LINE OF TRACT 3 NO1°36'18"W 428.68' TO THE POINT OF BEGINNING.

THIS PARCEL IS THE SAME AS LOT 2-3 AS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY, TRACTS 2-2 & 2-3, TWITTY PROP.", BY W. GRAHAM CAWTHORNE, JR., RLS, DATED NOVEMBER 17, 1995, REVISED JANUARY 12, 1996 AND RECORDED IN BOOK OF MAPS 1996, PAGE 1582, WAKE COUNTY RECORDS.

CONTAINING 11.168 ACRES, MORE OR LESS.

NEW LEGAL DESCRIPTIONS

PARCEL 'C' (PIN #1758-56-8976)

ALL THAT CERTAIN REAL PROPERTY SITUATED IN THE TOWN OF ROLESVILLE, COUNTY OF WAKE, STATE OF NORTH CAROLINA, DESCRIBED AS FOLLOWS:

BEGINNING AT A FOUND IRON PIPE MARKING THE NORTHEAST CORNER OF TRACT 2 AS SAID TRACT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION MAP OF BOBBY L. MURRAY TRUST, TRACTS 1+2", BY KENNETH CLOSE, INC., DATED NOVEMBER 7 2002 AND RECORDED IN BOOK OF MAPS 2005, PAGES 1195 AND 1196, WAKE COUNTY RECORDS; THENCE ALONG THE NORTH LINE OF SAID TRACT 2 N75"27"01"W 704.12" TO A FOUND IRON PIPE ON THE SOUTHEAST RIGHT OF WAY LINE OF LOUISBURG ROAD (U.S. 401); THENCE CONTINUING N75'27'01"W 32.34' TO THE CENTERLINE OF LOUISBURG ROAD; THENCE ALONG THE CENTERLINE OF SAID ROAD N36'33'22"E 1,116.98'; THENCE CONTINUING ALONG SAID CENTERLINE ALONG THE ARC OF A TANGENT CURVE TO THE RIGHT, CONCAVE TO THE SOUTHEAST, HAVING A RADIUS OF 3,750.00', THROUGH A CENTRAL ANGLE OF 18'18'05", AN ARC LENGTH OF 1,197.82' AND BEING SUBTENDED BY A CHORD BEARING N45'42'25"E 1,192.73'; THENCE LEAVING SAID CENTERLINE S89°44'38"E 134.33' TO AN ANGLE POINT IN THE SOUTHWEST LINE OF THAT CERTAIN PARCEL DESCRIBED IN DEED BOOK 6821, PAGE 005, WAKE COUNTY RECORDS; THENCE ALONG THE SOUTHWEST LINE OF SAID PARCEL AND ALONG THE WEST LINE OF WALI CREEK SUBDIVISION, PHASES 2. 4 AND 5-A AS RECORDED IN BOOK OF MAPS 1997, PAGE 1162, BOOK OF MAPS 2001, PAGE 628 AND BOOK OF MAPS 2002, PAGE 825, WAKE COUNTY RECORDS, S03°05'32"W 2,131.93' TO A POINT ON THE NORTH LINE OF CARLTON POINTE SUBDIVISION. PHASE I AS RECORDED IN BOOK OF MAPS 2008. PAGES 5 TO 10, WAKE COUNTY RECORDS; THENCE ALONG SAID NORTH LINE OF CARLTON POINTE SUBDIVISION N75°27'01"W 852.86' TO THE POINT OF BEGINNING.

THIS PARCEL IS THE SAME AS TRACTS 2A AND 3 AS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY", BY W. GRAHAM CAWTHORNE, JR., RLS, DATED NOVEMBER 17, 1995 AND RECORDED IN BOOK OF MAPS 1995, PAGE 2034, WAKE COUNTY RECORDS.

CONTAINING 44.100 ACRES, MORE OR LESS.

PARCEL 'D' (PIN #1758-45-8905)

ALL THAT CERTAIN REAL PROPERTY SITUATED IN THE TOWN OF ROLESVILLE, COUNTY OF WAKE, STATE OF NORTH CAROLINA, DESCRIBED AS FOLLOWS:

BEGINNING AT A FOUND IRON PIPE ON THE SOUTH LINE OF TRACT 3 AS SAID TRACT IS SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "RECOMBINATION SURVEY FOR TOMMY TWITTY", BY W. GRAHAM CAWTHORNE, JR., RLS, DATED NOVEMBER 17, 1995 AND RECORDED IN BOOK OF MAPS 1995, PAGE 2034, WAKE COUNTY RECORDS, SAID PIPE ALSO BEING AN ANGLE POINT IN THE NORTHWEST LINE OF CARLTON POINTE SUBDIVISION, PHASE I AS RECORDED IN BOOK OF MAPS 2008, PAGES 5 TO 10, WAKE COUNTY RECORDS; THENCE ALONG THE NORTHWEST LINE OF SAID CARLTON POINTE SUBDIVISION \$36.44.45, W 877.98' TO AN ANGLE POINT: THENCE CONTINUING ALONG SAID NORTHWEST LINE AND ALONG THE NORTH LINE OF LOTS 1, 2 AND 3 AS SAID LOTS ARE SHOWN AND SO DESIGNATED ON THAT CERTAIN PLAT ENTITLED "SUBDIVISION PLAT FOR BARRETT VENTURES, LLC", BY CAWTHORNE, MOSS & PANCIERA, P.C., DATED APRIL 4, 2007 AND RECORDED IN BOOK OF MAPS 2008, PAGE 702, WAKE COUNTY RECORDS, S87'10'58"W 737.24' TO AN ANGLE POINT IN THE EAST RIGHT OF WAY LINE OF JONESVILLE ROAD (S.R. 2226) AND THE SOUTHEAST CORNER OF THAT CERTAIN PARCEL DESCRIBED IN DEED BOOK 13993, PAGE 2591, WAKE COUNTY RECORDS; THENCE ALONG THE EAST LINE OF SAID PARCEL NO2'07'15"W 48.32': THENCE CONTINUING ALONG SAID EAST LINE, ALONG THE ARC OF A CURVE TO THE LEFT, CONCAVE TO THE WEST, HAVING A RADIUS OF 123.92' THROUGH A CENTRAL ANGLE OF 30°27'05". AN ARC LENGTH OF 65.86' AND BEING SUBTENDED BY A CHORD BEARING N17°21'49"W 65.09' TO THE EAST RIGHT OF WAY LINE OF JONESVILLE ROAD; THENCE ALONG SAID EAST RIGHT OF WAY LINE NO3*10'03"W 17.67' TO THE SOUTHEAST RIGHT OF WAY LINE OF LOUISBURG ROAD (U.S. 401); THENCE ALONG SAID SOUTHEAST RIGHT OF WAY LINE THE FOLLOWING EIGHT COURSES: (1) N41*10'16"E 41.30'; (2) N40*37'47"E 49.15'; (3) N39*31'43"E 50.83'; (4) N38'52'58"E 49.80'; (5) N38'22'04"E 50.40'; (6) N37*17'31"E 50.37'; (7) N36*44'45"E 304.20'; AND (8) N36*27'57"E 396.53' TO A FOUND IRON PIPE ON THE SOUTH LINE OF THE AFORESAID TRACT 3; THENCE ALONG SAID SOUTH LINE OF TRACT 3 S75'27'01"E 704.12' TO THE POINT OF BEGINNING.

CONTAINING 15.024 ACRES, MORE OR LESS.

10/1/2020

DATE

S.R. 2051
BURLINGTON
MILLS RD

SITE

SOLOWING WATER

DRIVE

LONNIE DR

BENDEMEER

LN

Cook

SITE

Solowing

ROLESMILE

RO

VICINITY MAP SCALE: 1"=2000'

LEGEND

(1) = RECORD DATA PER BM 1996 PG 1582
(2) = RECORD DATA PER BM 2011 PG 383
(3) = RECORD DATA PER BM 1995 PG 2034
(4) = RECORD DATA PER BM 2002 PG 825
(5) = RECORD DATA PER BM 2005 PG 1195−1196
(6) = RECORD DATA PER DB 13993 PG 2591

■ FOUND MONUMENT AS NOTED

O = SET IRON PIN

Δ = NCGS MONUMENT

■ DIMENSION POINT (NOTHING SET)

P = PROPERTY LINE

R/W = RIGHT OF WAY

C&G = CURB AND GUTTER

CATV = CABLE TV PEDESTAL

DI = DROP INLET

ELEC = ELECTRIC BOX

EM EM = ELECTRIC METER

F/O = FIBER OPTIC

□□ FH = FIRE HYDRANT

□□ GV = GAS VALVE

HBX = HAND BOX

↓ LP = LIGHT POLE

✓ PP = POWER POLE

← = GUY WIRE

RCP = REINFORCED CONCRETE PIPE

S.F. = SQUARE FEET (AREA)

= SIGN

SIGNAL

= TRAFFIC SIGNAL POLE

SS = SANITARY SEWER MANHOLE

= SANITARY SEWER FORCE MAIN VALVE

SD = STORM DRAIN MANHOLE

TEL = TELEPHONE PEDESTAL

TRAF = TRAFFIC BOX

WATER BOX

WM WM = WATER BOX

WM WM = WATER METER

W WMH = WATER MANHOLE

WV = WATER VALVE

= WELL

VALK

= PEDESTRIAN X-WALK POLE

= ELECTRIC LINE
= SANITARY SEWER FORCE MAIN
= FO = FIBER OPTIC LINE
= GAS LINE

G = GAS LINE

G = OVERHEAD ELECTRIC LINE

S = SANITARY SEWER LINE

TELEPHONE LINE

PHONE (SOA)

TH CARO

SESSION

SEAL.

L-4192

NO SURVE

JOHNSON, MIRMIRAN & THOMPSON

Engineering A Brighter Future ®
9201 Arboretum Parkway Suite 310 Richmond, Virginia 23236

REVISION #	DATE		REASO	ON FOR	REVISIO	N	
1 10/	01/2020	ADDED	RIPARIAN	BUFFER	ALONG	WALL	CREEK

ALTA/NSPS LAND TITLE SURVEY

PREPARED FOR WALLBROOK LANDCO, LLC

WAKE FOREST TOWNSHIP

TOWN OF ROLESVILLE, WAKE COUNTY, NORTH CAROLINA

DRAWN BY: JSZ PROJECT#:

CHECKED BY: WTR CONTRACT#: JMT#: 17-10946-001

DATE: 03/25/2020 SCALE: 1"=100' SHEET 1 OF 3

CURRENT ZONING SETBACK REQUIREMENTS:

TAX PARCEL 1758-48-9229: R-1 (SINGLE FAMILY RESIDENTIAL)
TAX PARCEL 1759-58-2090: R-1-SUD (SINGLE FAMILY RESIDENTIAL SPECIAL USE DISTRICT)
TAX PARCEL 1758-56-8976: CO-SUD (COMMERCIAL OUTLYING SPECIAL USE DISTRICT)

TAX PARCEL 1758-45-8905: CO-SUD

FRONT SIDE CORNER REAR

R-1 30' 12' 22' 25' (SETBACKS INCLUDE SPECIAL USE DISTRICTS)
CO 20' 15' 25' 35'

NOTE: ZONING INFORMATION BASED ON INFORMATION AS SUPPLIED BY CURRENT COUNTY ZONING DEPARTMENT, NO ZONING REPORT OR LETTER WAS PROVIDED TO SURVEYOR AT TIME OF SURVEY.

THIS IS TO CERTIFY THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH THE 2016 MINIMUM STANDARD DETAIL

WILLIAM T. ROBBINS, II

TO WALLBROOK LANDCO, LLC, ITS SUCCESSORS AND/OR ASSIGNS AS THEIR

REQUIREMENTS FOR ALTA/NSPS LAND TITLE SURVEYS, JOINTLY ESTABLISHED AND

ADOPTED BY ALTA AND NSPS, AND INCLUDES ITEMS 1, 2, 3, 4, 6A, 7A, 7B1, 8,

9, 11, 13, 16 AND 17 OF TABLE "A" THEREOF, THE FIELDWORK WAS COMPLETED

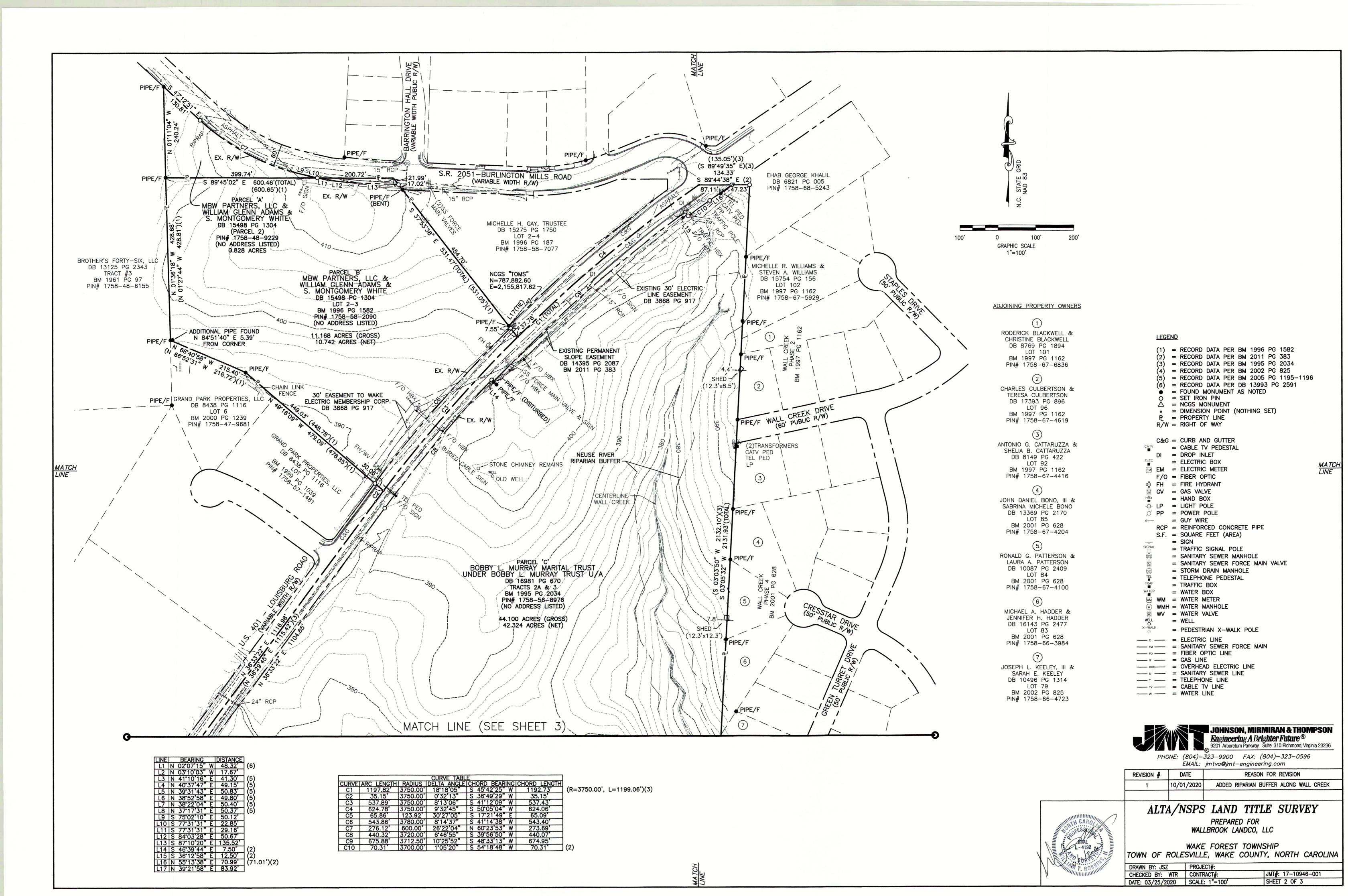
INTERESTS MAY APPEAR, AND INVESTORS TITLE INSURANCE COMPANY:

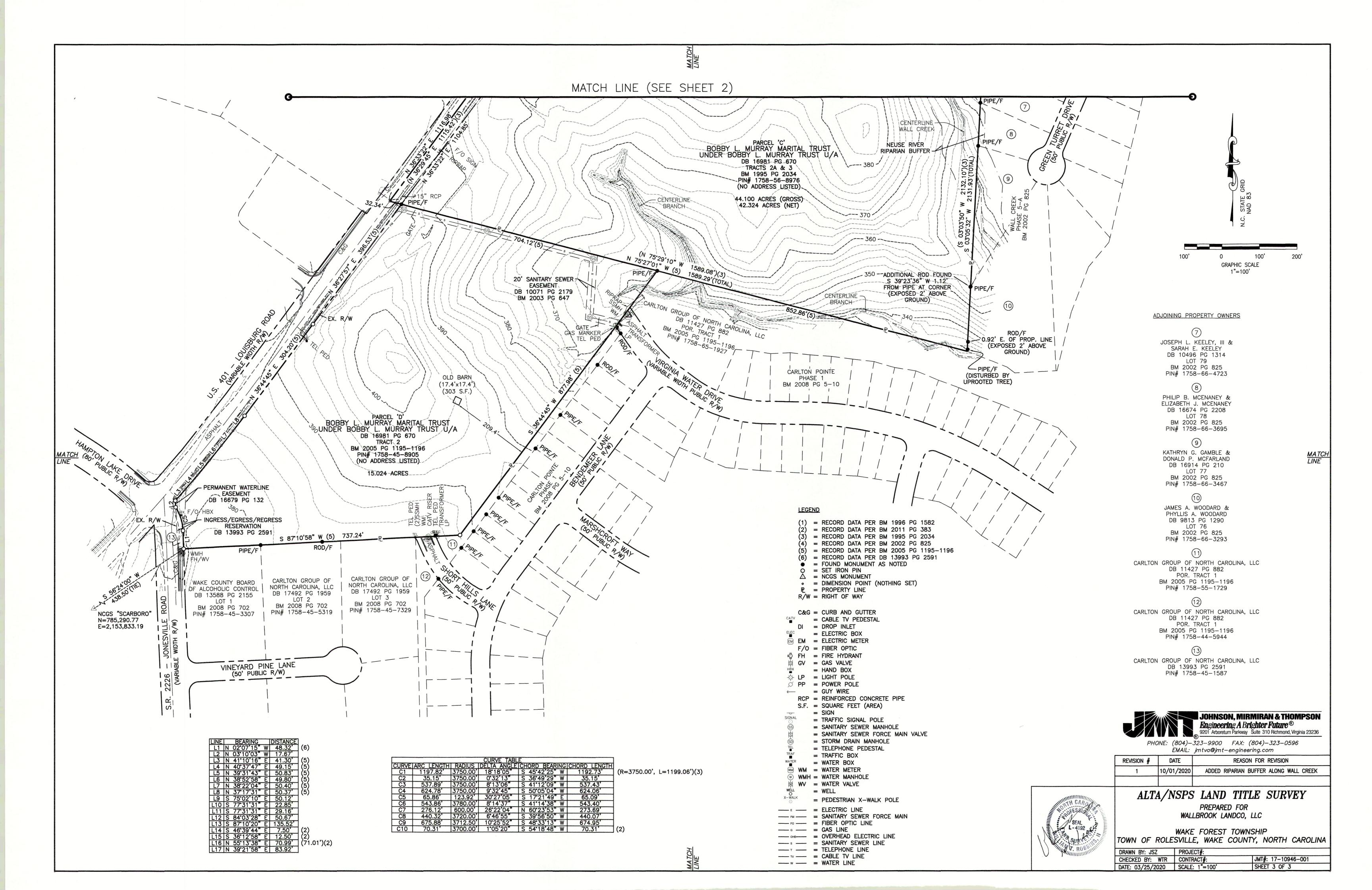
P.L.S. #L-4192

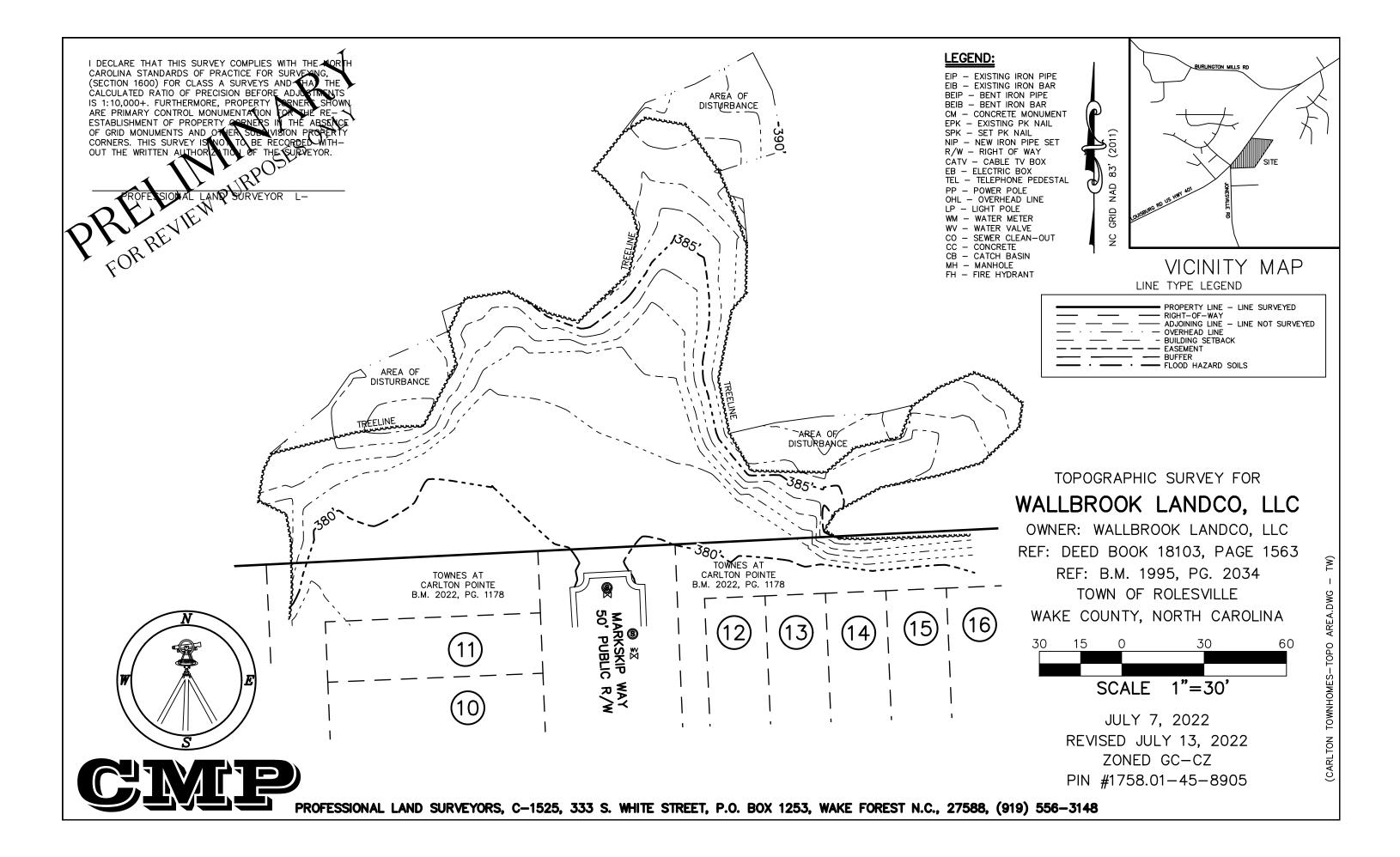
ON MARCH 6, 2020.

SURVEYOR'S CERTIFICATION:

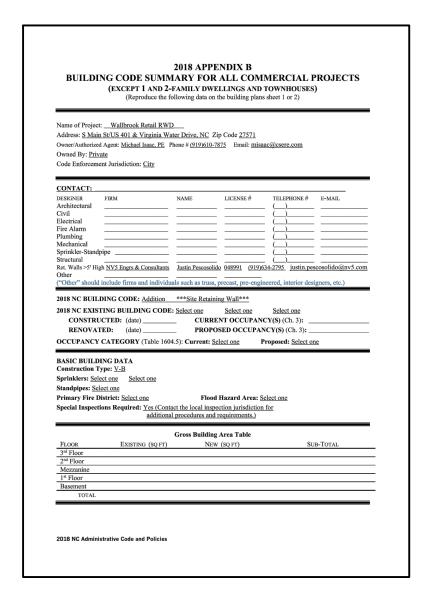
MATCH



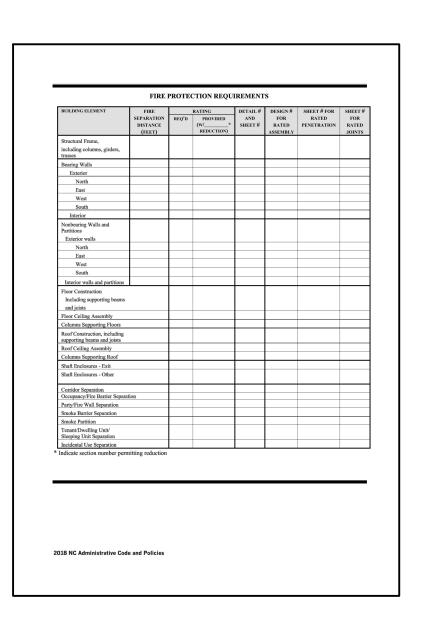


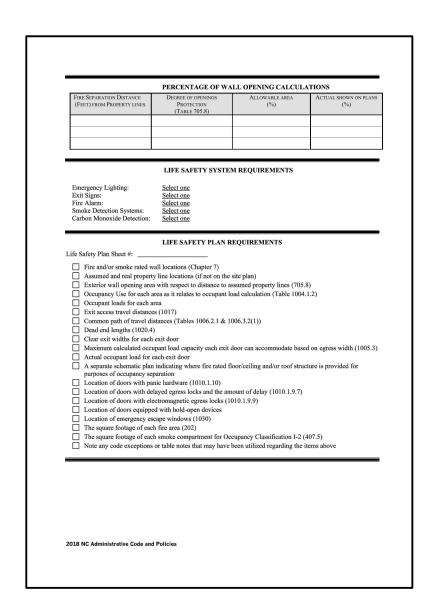


CONCRETE RETAINING WALL DESIGN FOR WALLBROOK SHOPPING CENTER ROLESVILLE, NORTH CAROLINA



Accessory Oc Incidental Us Special Uses	ecupancy Classificati es (Table 509):		Select one Sele	t one Select one S	elect one Select one
Special Uses	es (Table 509):	on(s):			
	(Chapter 4 – List Co sions: (Chapter 5 – 1		.alı		
	ancy: Select one			tion:	
Select on					
	tual Area of Occupan		ctual Area of Oc		
Allov	vable Area of Occupa	псу А Ано	wable Area of C	есирансу в	
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STORY	DESCRIPTION AND	(A)	(B)	(C)	(D)
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		STORY (ACTUAL)	AREA	INCREASE ^{1,5}	STORY OR UNLIMITED ^{2,3}
-					
		ALLOV	VABLE HEIGH	п	
		I AI	LLOWABLE	SHOWN ON PLANS	CODE REFERENCE 1
Building He	ight in Feet (Table 504.				
	ight in Stories (Table 50				
² The maximu	e reference if the "Sho m height of air traffic m height of open parl	control towers m	ust comply with	d on Table 504.3 or 50 Table 412.3.1. ble 406.5.4.	04.4.





RETAINING WALL CONSTRUCTION (IBC 1806.2)								
Item			Detailed Instructions and Frequencies					
Verify the properties of the in-place and proposed soil materials	☐ Continuous	✓ Periodic	See Retaining Wall Details and Specifications					
Verify the suitability of in-place and proposed soil materials	☐ Continuous	✓ Periodic	See Retaining Wall Details and Specifications					
Verify subgrade is adequate to achieve design bearing capacity	☐ Continuous	✓ Periodic	See Retaining Wall Details and Specifications					
Verify size, grade and layout of reinforcing steel	☐ Continuous	✓ Periodic	See Retaining Wall Details and Specifications					
Perform testing of concrete (slump, air, compressive strength)	☐ Continuous	☑ Periodic	See Retaining Wall Details and Specifications					
Verify installation of drainage system	☐ Continuous	✓ Periodic	See Retaining Wall Details and Specifications					
Verify density, moisture content, and lift thickness for fill	✓ Continuous	☐ Periodic	See Retaining Wall Details and Specifications					
A summary of testing services, si Registered Design Professional p 4905 Professional Court Raleigh, North Carolina 27609 Phone (919) 876-9799 Fax (919)	orior to issuand		red professional engineer shall be provided to aspection report. Registered Design Professional					

Plan Sheet Index:

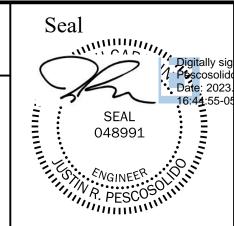
RW-1 Title Sheet

RW-2 Retaining Wall Layout

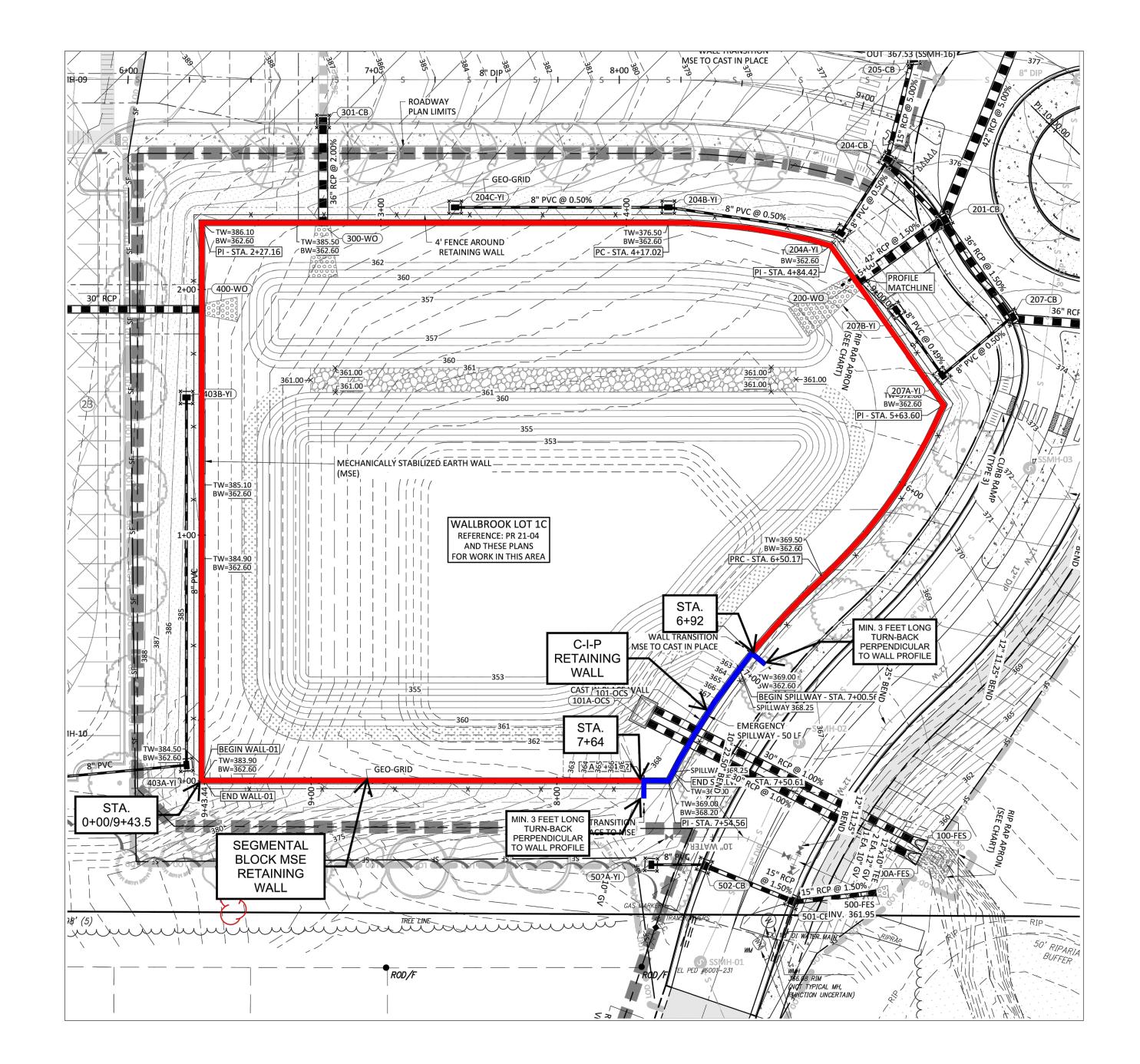
RW-3 Retaining Wall Details and Specifications

NV5 Engineers & Consultants, Inc.

4905 Professional Court Raleigh, North Carolina 27609 Phone: (919) 876-9799 Fax: (919) 876-8291 North Carolina Corporate License No. F-1333



2 1 2	DATE 8/9/2022 2/21/2023	DESCRIPTION ADDED TO RCW EXTENTS CHANGE TO RCW EXTENTS	BY JRP JRP	Title Sh	neet	
				Designed by: Justin R. Pescosolido, P.E.	Date: 5-26-22 Date: 5-26-22	SHEET RW-1
				Drawn By: Jalen Deatherage Reviewed By: Brock Horsley, P.E.	Date: 5-26-22	IX 44 - 1
				Retaining Wal	ll Design	



Note: This site plan was provided to NV5 Engineers & Consultants, Inc. by Crosland Southeast. This site plan is for general reference only and is not intended to be used for construction layout of the retaining walls. Actual retaining wall layout shall be staked in the field by a qualified surveyor and approved by the Site/Civil Engineer of Record prior to construction. Top-of-Wall elevations to be dictated by site/civil drawings, within the maximum height parameters described, herein.

General Notes

This retaining wall has been designed using active earth pressure theory. Therefore, outward movement at the top of the wall should be expected. Some cracks could develop at the ground surface due to lateral movement of the wall. These cracks should be filled in as soon as they are observed to help protect the soils below the ground surface from softening related to water infiltration that could affect the support characteristics for nearby construction.

External stability analyses for bearing capacity, global stability, and total and differential settlement was performed based on soil parameters derived from our previous Report of Subsurface Investigation and Geotechnical Engineering Evaluation.

The Engineer requests that representatives of the owner and/or general contractor arrange a pre-construction meeting with all pertinent parties involved for the construction of the retaining wall shown on these plans. The Engineer's responsibility is limited to providing only the design services of the project's retaining wall contained herein. Retaining wall construction monitoring and retaining wall certifying are beyond the scope of these design services. The Engineer shall not be required to sign any document, no matter by whom requested, in which the Engineer is required to certify, guarantee, or warrant conditions of which the Engineer has not or cannot ascertain.

This wall has been designed as a reinforced concrete wall considering the maximum expected soil backfill and lowest planned (during construction) bottom-of-wall grades (EL. 362.6). A live load surcharge of 250 psf was utlized at appropriate setbacks so as to model the planned asphalt pavement loading conditions. With the exception of these conditions no additional dead loading conditions, no live loading, and no additional lateral loading conditions were considered. Structures such as light poles, handrail, guardrail, or drainage structures to be installed in the vicinity of the retaining wall shall be designed and constructed to resist imposing additional lateral loads on the retaining wall. If future construction alters the assumed loading conditions of the retaining wall, NV5 Engineers & Consultants, Inc. shall be notified to review the design criteria for the imposed loads.

This reinforced concrete retaining wall shall include a turn-back at 90 degrees from the path of the wall at either end to facilitate connection with the adjacent MSE retaining wall. See Site Layout Figure for minimum acceptable turn-back distances. We understand that flow over the weir portion of this wall, from the adjacent SCM, is expected to occur periodically. It is imperative that the Site/Civil Engineer provide specifications for suitable scour protection at and around the weir portion of this wall so as to prevent loss of soils at any portion of the wall. We also note that this retaining wall design is intended to address ONLY the geotechnical and structural design characteristics of the wall. We have not performed design services related to the hydrologic components of the weir or SCM. It is also imperative that, prior to construction, the Site/Civil Engineer perform design substantiation to verify that this design submittal complies with the required storm water and hydrologic components of the weir wall system and pond, as a whole.

Construction Notes

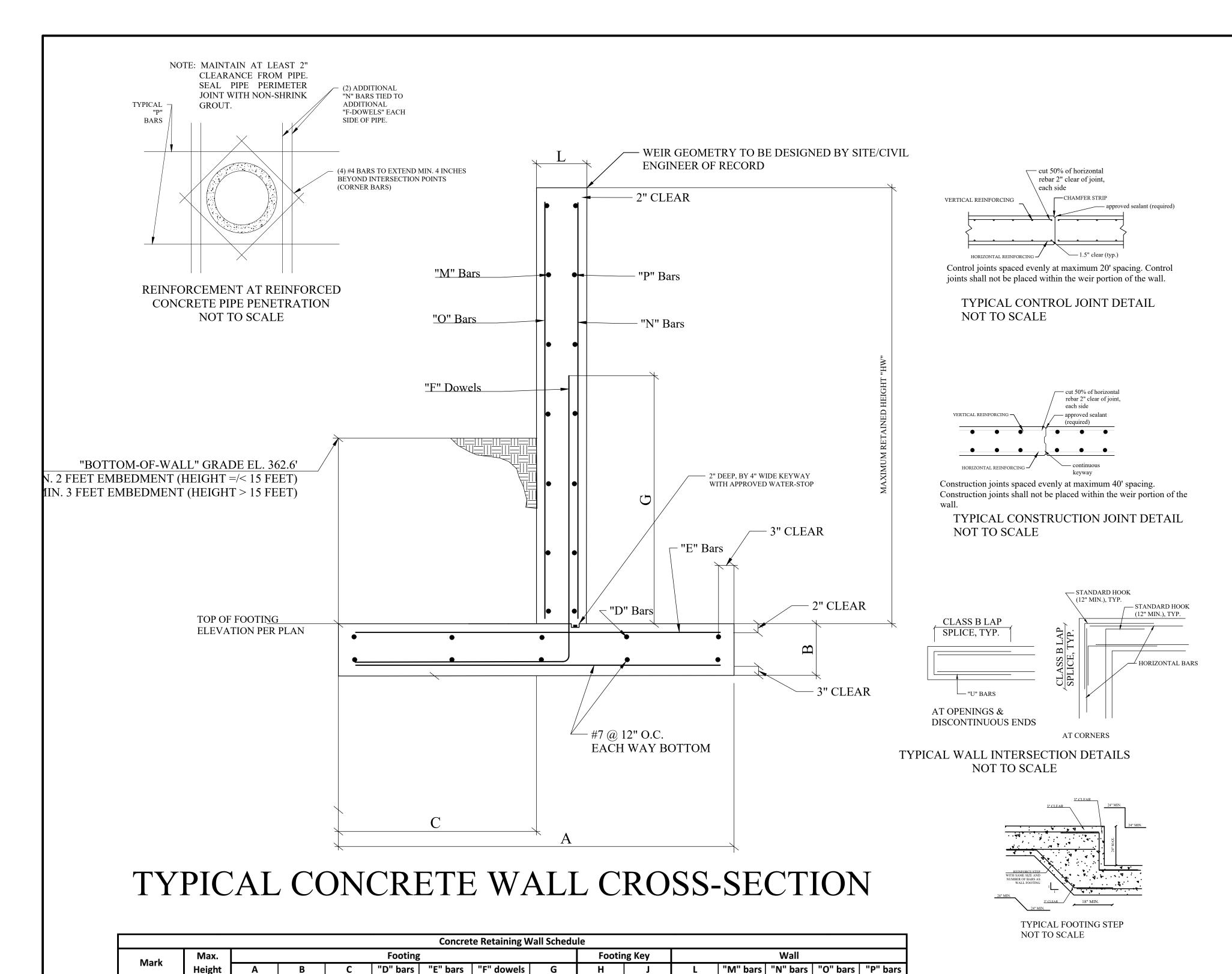
- Prior to construction, confirmation of the distances to property lines, buffers of any kind, curb and gutter, and/or buildings to the face of the proposed walls shall be performed.
- 2. Prior to construction, confirmation of existing utility line locations (Stormwater, Sewer, Water, Electrical, Communications, and Gas) and the locations of future utility lines shall be performed.
- 3. Prior to construction, confirmation of the in-situ and proposed grades shall be performed.
- 4. During construction, care must be exercised to prevent the undermining of any existing structures. Construction of nearby structures, after construction of this wall, shall be carried out in such a way as to avoid damaging and/or undermining of this wall system.
- 5. After construction, heavy equipment should not be operated within 3 feet of the completed walls.
- 6. After construction, surface water drainage from adjacent tyard areas and roadways shall be directed to the stormwater drainage system, and away from the retaining walls.
- 7. The project civil engineer shall be consulted for recommendations related to environmental requirements for finished slopes.

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<u>V</u>	8/9/2022 2/21/2023	ADDED TO RCW EXTENTS CHANGE TO RCW EXTENTS	JRP JRP	Retaining Wa	ll Layou	t
				Designed by: Justin R. Pescosolido, P.E.	Date: 5-26-22	SHEET
				Drawn By: Jalen Deatherage	Date: 5-26-22	RW-2
				Reviewed By: Brock Horsley, P.E.	Date: 5-26-22	
				Retaining Wal	ll Design	



General Specifications:

- 1. These drawings are the sole property of NV5 Engineers & Consultants, Inc. (NV5) for use on this project, and shall not be copied, reproduced or used for other purposes without permission of NV5.
- 2. This retaining wall has been designed as a reinforced concrete retaining wall. The details on this page shall be considered applicable to the construction of this retaining wall.
- 3. Dimensions and structural components shown on these drawings are based on information provided by others. Construction shall not begin until Site/Civil Engineer has provided design substantiation and Contractor has reviewed existing conditions and verified all items that may affect new construction. NV5 shall be made aware of all discrepancies.
- 4. The wall is only stable in its completed form. Contractor shall provide all temporary bracing during construction to stabilize the structure. Design of temporary shoring and bracing is not the responsibility of NV5 and is beyond the scope of these drawings.
- 5. These drawings should be used in conjunction with architectural, structural, and civil drawings. Any discrepancies should be brought to the attention of NV5 prior to construction.
- 6. All construction shall be in accordance with applicable sections of the 2018 North Carolina Building Code and local building codes, including Special Inspections.
- 7. Contractor is responsible for obtaining all required building permits or other required legal documents.
- 8. Both sides of control joints and construction joints shall be sealed with elastomeric sealant approved by NV5.
- 9. Waterproofing shall be applied to both sides of the wall along the entirety of the wall face.

Foundations:

- 1. The scope of services provided by NV5 begins from the bottom of the footing. The foundation systems shown on these drawings are based on an available bearing pressure of 4,000 pounds per square foot. Verification of bearing capacity is the responsibility of the contractor, owner and/or owner's agent.
- 2. Top of all footings shall be at least 24 inches below grade. See wall design details for embedment requirements. The bottom-of-wall elevation (lowest exposed elevation) for the concrete wall shall be maintained at Elevation 362.6' UNO.
- 3. Excavate to elevations specified on site civil drawings (or as described in item 2 above) and to the specified dimensions within a tolerance of 1 inch. Do not disturb soils in the bottom of footing excavations. Excavate by hand to final elevations. All excavations should be performed so as not to undermine existing footings or damage existing structures. The contractor is responsible for locating all underground utilities and other structures, and for maintaining excavations in a safe condition in accordance with all OSHA guidelines.
- 4. Excavation for footings shall be temporarily lined with a 6 MIL polyethylene if placement of concrete does not occur within 24 hours of excavation.
- 5. No concrete shall be placed on any subgrade containing standing water, ice, frost or loose soil.

Soils and Wall Backfill:

- 1. Any fill that will support the wall and wall backfill shall be placed under the direction or recommendation of a licensed professional engineer using suitable soils. Scarify, bench or break up sloped surfaces steeper than 1(V) to 4(H) so that soils bond with existing materials, except where these operations could undermine existing construction. Place fill soils in layers not more than 8 inches in loose depth for materials to be compacted by heavy compaction equipment and 4 inches loose depth for materials compacted by hand operated equipment. Compact soil to at least 95 percent of the standard Proctor (ASTM D-698) maximum dry density. Minimum soil wet density shall be 120 pounds per cubic foot.
- Retaining wall backfill shall have a minimum compacted unit weight of 120 pounds per cubic foot.
- 4. Minimum 12" wide drainage layer of washed No. 57 stone to be constructed at rear face of RW in maximum 8 inch lifts (loose measurement) and consolidated utilizing 2 passes (minimum) of suitable compaction equipment.
- 5. Geotextile fabric, such as Amoco 2006 or equivalent, shall be placed between soil and washed #57 stone when such materials are placed adjacent to each other.

Concrete

- . Concrete shall be proportioned, mixed, placed, consolidated and tested in accordance with ACI 318, ACI 301, ACI 350, and ACI 117.
- 2. Reinforcing steel shall comply with ASTM A615, grade 60, deformed.
- No admixtures or fly ash shall be added to wall concrete without written permission from NV5. A concrete mixture design (with a w/c ratio of < 0.45) shall be submitted to NV5 for approval prior to construction.
- 4. Concrete shall have the following properties: 28-day compressive strength of 4,000 psi; maximum slump of 4.5 inches; air content of 4% ±/-1%.
- 5. Comply with CRSI Manual of Standard Practice for fabricating, placing and supporting reinforcement.
- 6. Provide clear cover of reinforcement of at least 3 inches or as shown on drawings.
- 7. Splice reinforcement as detailed or authorized by NV5. Make bars continuous around corners. Splices shall be made by contact tension lap splices unless otherwise noted.
- 8. Welding of reinforcement not permitted.
- 9. Placing of sleeves through concrete elements not permitted unless shown on drawings.
- 10. Field bending of reinforcement partially embedded in concrete is not permitted unless approved by NV5.
- 11. Chamfer exposed concrete corners $\frac{3}{4}$ inch by $\frac{3}{4}$ inch, minimum, unless otherwise noted.
- 12. Water may be added to concrete at the project site, subject to the limitations of ACI 301.
- 13. See architectural drawings for finishing requirements of formed concrete surfaces.
- 14. Cure formed and unformed concrete for at least 7 days by one of the following methods; Moist curing, moisture retaining cover, application of curing compound, or application of curing and sealing compound.

NV5 Engineers & Consultants, Inc.

5'-6"

15-#7 | #9 @ 8" | #9 @ 16"

20-#9 | #10 @ 8" | #10 @ 16" | 72"

Type-B

Type-C

20'

27'

28'

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NO KEY

NO KEY

NO KEY

NO KEY

Wallbrook Shopping Center
Rolesville, North Carolina
Our Project Number: 121-22-108262

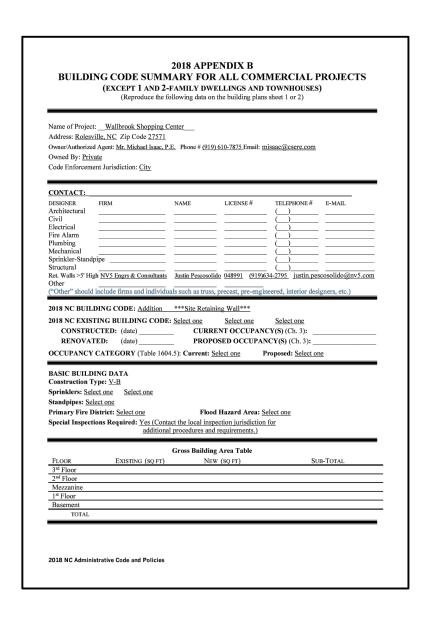
| #7 @ 12" | #7 @ 12" | #7 @ 18

#9 @ 9" | #9 @ 9" | #7 @ 18'

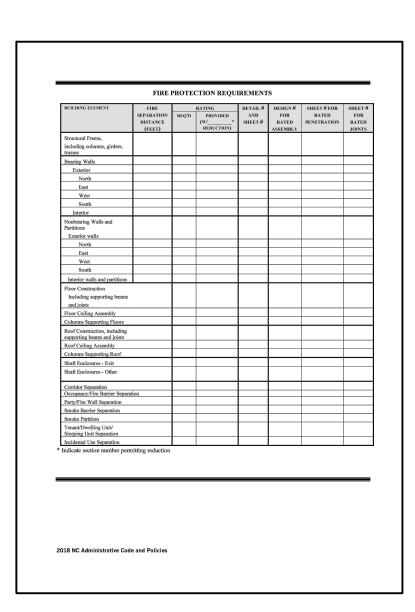
24" #7 @ 18" #9 @ 8" #9 @ 8" #7 @ 18'

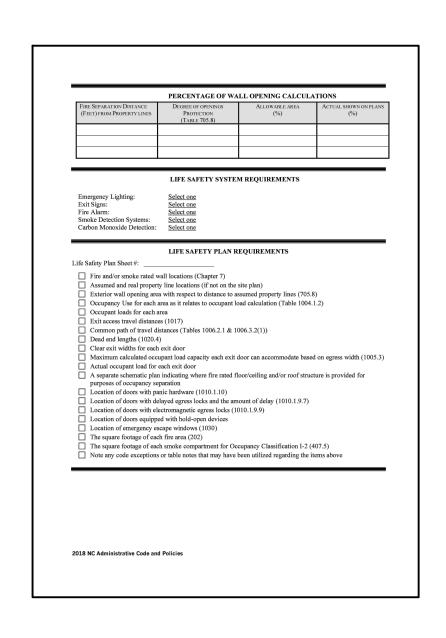
1 2	DATE 8/9/2022 2/21/2023	DESCRIPTION ADDED TO RCW EXTENTS CHANGE TO RCW EXTENTS	BY JRP JRP	Concrete Retaining	g Wall D	etails
				Designed by: Justin R. Pescosolido, P.E. Drawn By: Jalen Deatherage	Date: 5-26-22 Date: 5-26-22 Date: 5-26-22	SHEET RW-3
				Reviewed By: Brock Horsley, P.E. Retaining Wa		

RETAINING WALL DESIGN FOR WALLBROOK SHOPPING CENTER ROLESVILLE, NORTH CAROLINA



	cunancy Classificati	n(s): <u>Select one</u>			
	es (Table 509):				
	Chapter 4 – List Co	de Sections):			
	sions: (Chapter 5 – 1	, –			
Mixed Occup	ancy: Select one	Separation: Selection	ct one Excep	tion:	
Select one					
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Allow	able Area of Occupa	ncy A Auc	wanie Area of C	Эссирансу В	
		+ _		+	= ≤ 1.00
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		STORY (ACTUAL)	AREA	INCREASE ^{1,5}	STORY OR UNLIMITED ^{2,3}
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	increases from Sect	. 505.2	. 10	1	1
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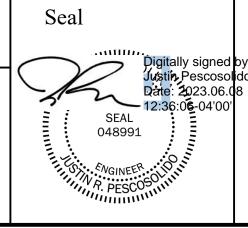
IT-4 MODULAR RETAINING WALLS (NCSBC SECTION 1610 AND 1704.13)						
Item				Detailed Instructions and Frequencies		
Verify the properties of the in-place and proposed soil materials	Continuous	7	Periodic	Materials shall meet the assumed design values in the "Back Fi Materials" and "Soil Properties" sections of the Specifications		
Verify the suitability of in-place and proposed soil materials	Continuous	V	Periodic	Materials shall meet the requirements of the "Soil Properties" "Unsuitable Material", 'Foundation Requirements", and "Structur Fill Placement" sections of the Specifications		
Verify subgrade is adequate to achieve design bearing capacity	Continuous	7 1	Periodic	Bearing capacity of foundation and reinforced zone shall be accordance with the "Foundation Load" and "Foundation Requirements" sections of the Specifications		
Verify leveling pad material and proper depth	Continuous	√ 1	Periodic	Materials shall meet requirements of the "Leveling Pad" section the Specifications		
Verify placement, setback, and connection method of units	✓ Continuous		Periodic	Retaining Wall Plans Detail and "Wall Batter", "Retaining W Shear Connections", "First Block Course" and "Retaining Wall C Units" sections of the Specifications		
Verify installation of drainage tile and drainage system	✓ Continuous	☐ F	Periodic	Retaining Wall Plans Detail and "Unit Fill" and "Hydrosta" Pressure Potential" sections of the Specifications		
Verify geogrid material, length, and elevation placement	✓ Continuous	☐ F	Periodic	Materials shall be as indicated on the RW Profile and meet t requirements of the "Geogrid Reinforcement", "Geogri Installation", & "Acceptable Geogrid" sections of the Specification		
Verify density, moisture content, and lift thickness for wall fill	✓ Continuous	✓ F	Periodic	Placement of fill shall meet requirements of the "Structural F Placement" and "Soil Testing" sections of the Specifications		
A summary of testing services, sign Design Professional prior to issuate 4905 Professional Court Raleigh, North Carolina 27609 Phone (919) 876-9799 Fax (919)	nce of the fina	•	_	ered professional engineer shall be provided to the Registere report. Registered Design Professional		
www.nv5.com	0/0-0291			Justin R. Pescosolido, P.E.		

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- RW-1 Title Sheet
- RW-2 Retaining Wall Layout
- RW-3 Retaining Wall Profile Part 1
- RW-4 Retaining Wall Profile Part 2
- RW-5 Retaining Wall Profile Part 3
- RW-6 Retaining Wall Profile Part 4
- RW-7 Retaining Wall Profile Part 5
- RW-8 Retaining Wall Profile Part 6
- RW-9 Retaining Wall Profile Part 7
- RW-10 Retaining Wall Profile Part 8
- RW-11 Anchor Diamond Pro PS Unit Details
- RW-12 Anchor Diamond Pro PS Unit Details (Cont'd)
- RW-13 Specifications

NV5 Engineers and Consultants, Inc.

4905 Professional Court Raleigh, North Carolina 27609 Phone: (919) 876-9799 Fax: (919) 876-8291 North Carolina Corporate License No. F-1333



REV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Title Sh	eet	
				Designed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	SHEET
				Designed by: Jalen G. Deatherage	Date: 6/7/23	RW-1
				Reviewed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	
				Segmental Retaining	g Wall De	sign

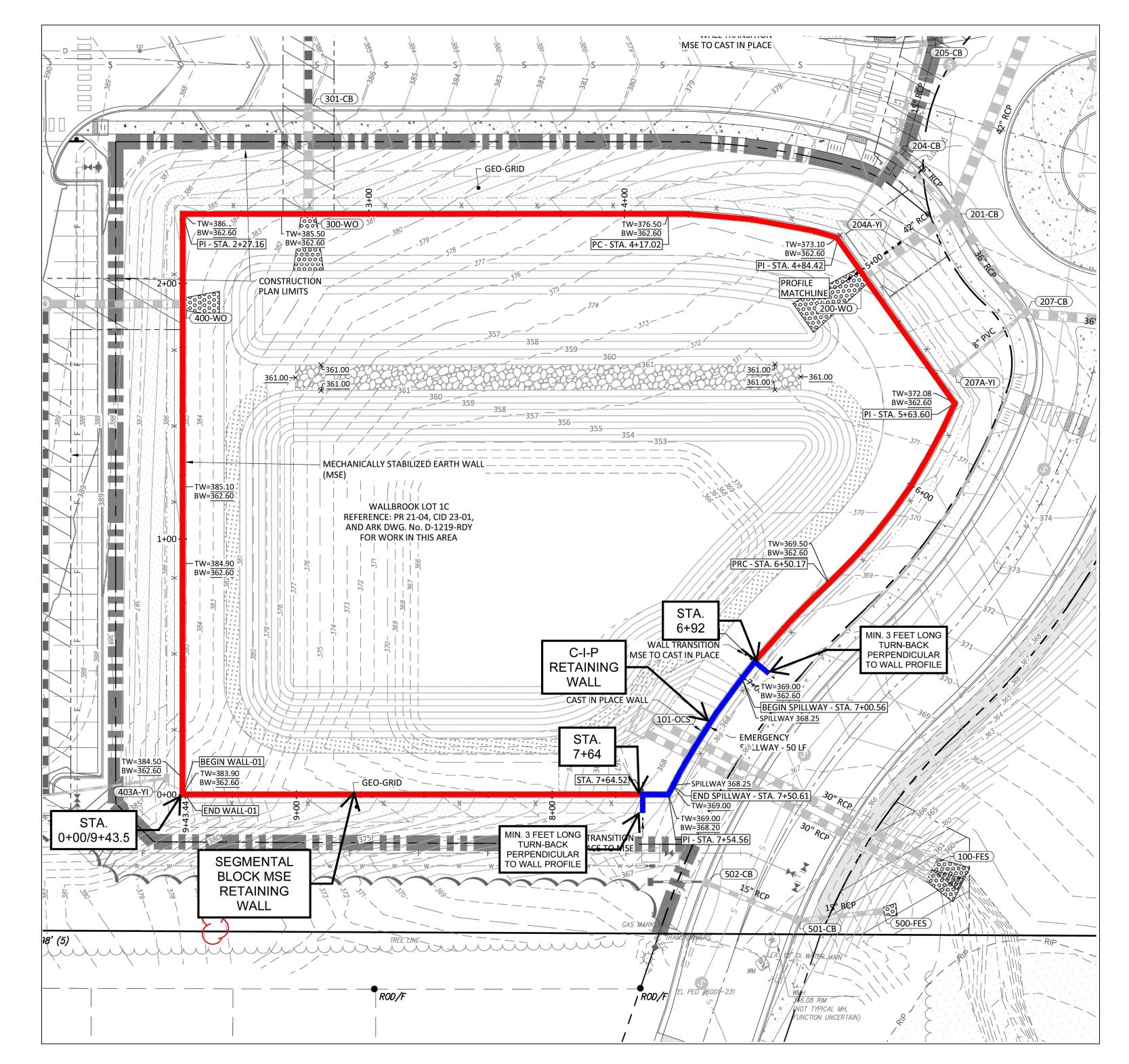


IMAGE FROM PROVIDED SITE PLAN. THIS SITE LAYOUT FIGURE IS APPROXIMATE. STAKING OF THE ACTUAL WALL LOCATIONS IN THE FIELD BASED ON SITE/CIVIL DRAWINGS IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

General Notes

The retaining wall(s) contained herein have been designed using active earth pressure theory. Therefore, outward movement at the top of the wall(s) should be expected. Some cracks could develop at the ground surface due to lateral movement of the wall(s). These cracks should be filled in as soon as they are observed to help protect the soils below the ground surface from softening related to water infiltration that could affect the support characteristics for adjacent construction.

Preliminary analyses for global stability and total and differential settlement were performed as part of the design services for the segmental retaining wall(s). Our analyses were based, in part, on assumed in-situ soil properties derived from our previous experience with similar conditions in close geographic proximity to this site. If soil conditions encountered during construction are significantly different than those assumed herein, NV5 Engineers and Consultants, Inc. shall be contacted immediately for review of and possible alterations to this design.

The Engineer requests that representatives of the owner and/or general contractor arrange a pre-construction meeting with all pertinent parties involved for the construction of the retaining wall(s) shown on these plans. The Engineer's responsibility is limited to providing only the design services of the project's retaining wall(s) contained herein. Retaining wall construction monitoring and retaining wall certifying are beyond the scope of these design services. The Engineer shall not be required to sign any document, no matter by whom requested, in which the Engineer is required to certify, guarantee, or warrant conditions of which the Engineer has not or cannot ascertain.

The retaining wall was designed using a live load surcharge of 250 pounds per square foot (psf) at the appropriate setback distances so as to model the planned roadway and parking lot pavement structures. Structures such as light poles, handrail, guardrail, or drainage structures to be installed in the vicinity of the retaining wall(s) shall be designed and constructed to resist imposing additional lateral loads on the retaining wall(s). If future construction alters the assumed loading conditions of the retaining wall(s), NV5 Engineers and Consultants, Inc. shall be notified to review the design criteria for the imposed loads.

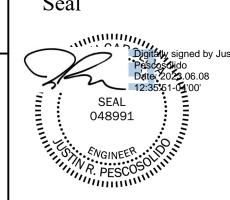
Scour protection shall be included at all bottom-of-wall areas at which water from the pond pool and/or surface flow over the top of the wall, is expected to be present/occur, even temporarily, during the service life of the wall. Scour protection design shall be specified by the site/civil engineer of record.

Construction Notes

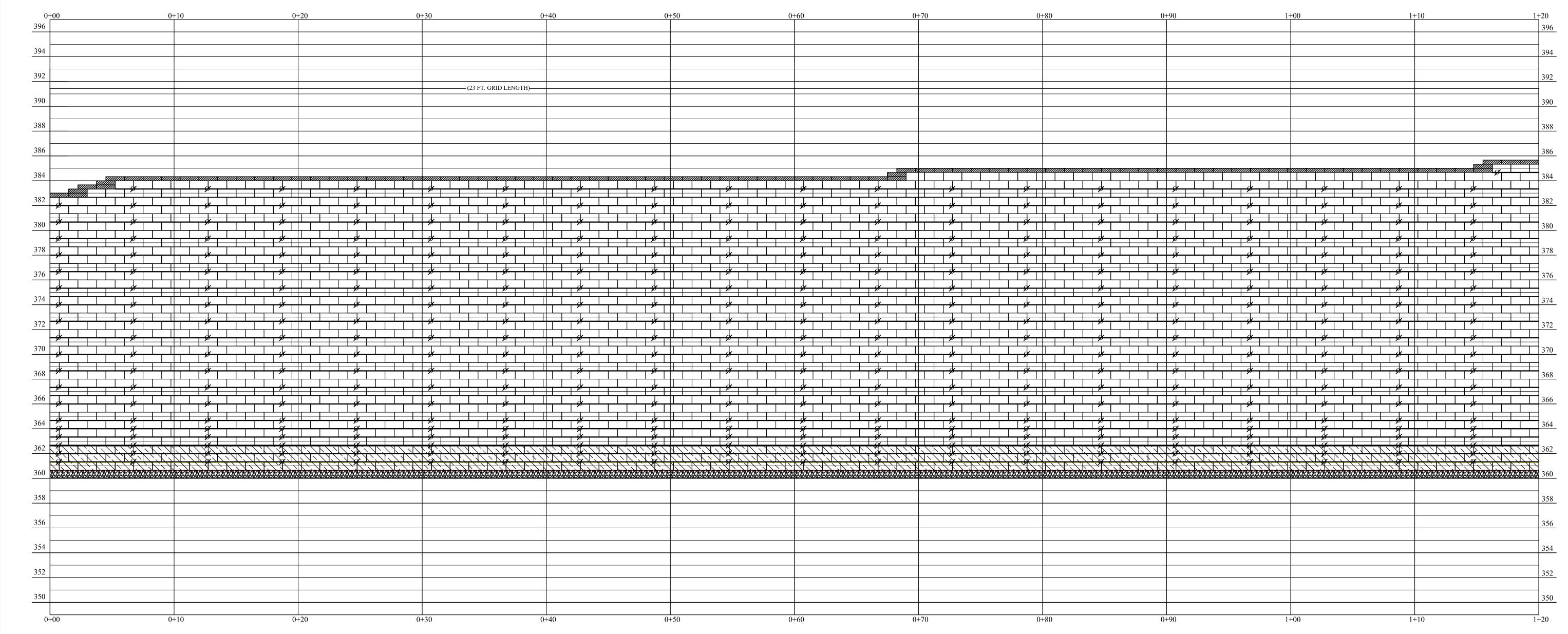
- 1. Prior to construction, confirmation of the distances to property lines, Tree Buffers, roadways, sidewalks, and/or curb and gutter to the face(s) of the proposed wall(s) shall be performed.
- 2. Prior to construction, confirmation of existing utility line locations (Stormwater, Sewer, Water, Electrical, and Gas) and the locations of future utility lines shall be performed.
- 3. Prior to construction, confirmation of the in-situ and proposed grades shall be performed by a qualified surveyor. NV5 Engineers and Consultants, Inc. shall be notified if the site grades are different than those shown on these drawings.
- 4. During construction, care must be exercised to prevent the undermining of any existing structures.
- 5. Utility structures and underground lines located within the reinforced zone of the retaining wall(s) shall be installed prior to or during construction of the retaining wall(s) to prevent damage to the reinforcement layers. If the presence of utility structures interferes with the integrity of the reinforcement, NV5 Engineers and Consultants, Inc. shall be notified during construction to recommend suitable remedial measures that will ensure proper operation of the retaining wall(s).
- 6. After construction, heavy equipment should not operate within 3 feet of the top portion of the wall(s) to prevent adverse impacts to the structural integrity of the retaining wall(s).
- 7. After construction, care must be exercised to prevent damage to the upper layers of reinforcement and degrading of the retained soils of the retaining wall(s). Installation of light poles, signs, handrails, guardrails, shrubs, or trees (etc.) in the reinforced zone of this retaining wall(s) shall not damage the upper layers of reinforcement. Any damaged reinforcement shall be repaired.
- 8. Surface water drainage shall be designed by others to discharge surface water away from the wall face(s) and away from the foundations of adjacent construction at all times during and after construction of the retaining wall(s). All downspouts from the surrounding structures should be directed away from the wall(s) and slope(s) above the wall(s).
- 9. Regular inspection and maintenance of the planned stormwater pond(s) is critical to long term performance of the retaining wall(s). Improper or irregular stormwater pond maintenance could negatively impact wall stability.

NV5 Engineers and Consultants, Inc.

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EV	DATE	DESCRIPTION	BY	
1	4-26-23	WALL TYPE/LAYOUT SCHEME 2	JRP	Retaining Wall Layout
				Tetaning wan Layout
				SHEET
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23
				DIV 1
				Designed by: Jalen G. Deatherage Date: 6/7/23 RW-2
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23
				Segmental Retaining Wall Design
				j segmentai Ketanning wan Design



NOTE: SIX (6) GRID LAYERS ARE UTILIZED WITHIN FIRST SEVEN (7) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 19 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

RETAINING WALL PROFILE - PART 1

LEGEND:

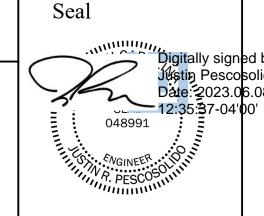
1). MIRAFI 3XT GEOGRID DESIGNATIO

2) EMBEDDED BLOCK DESIGNATION

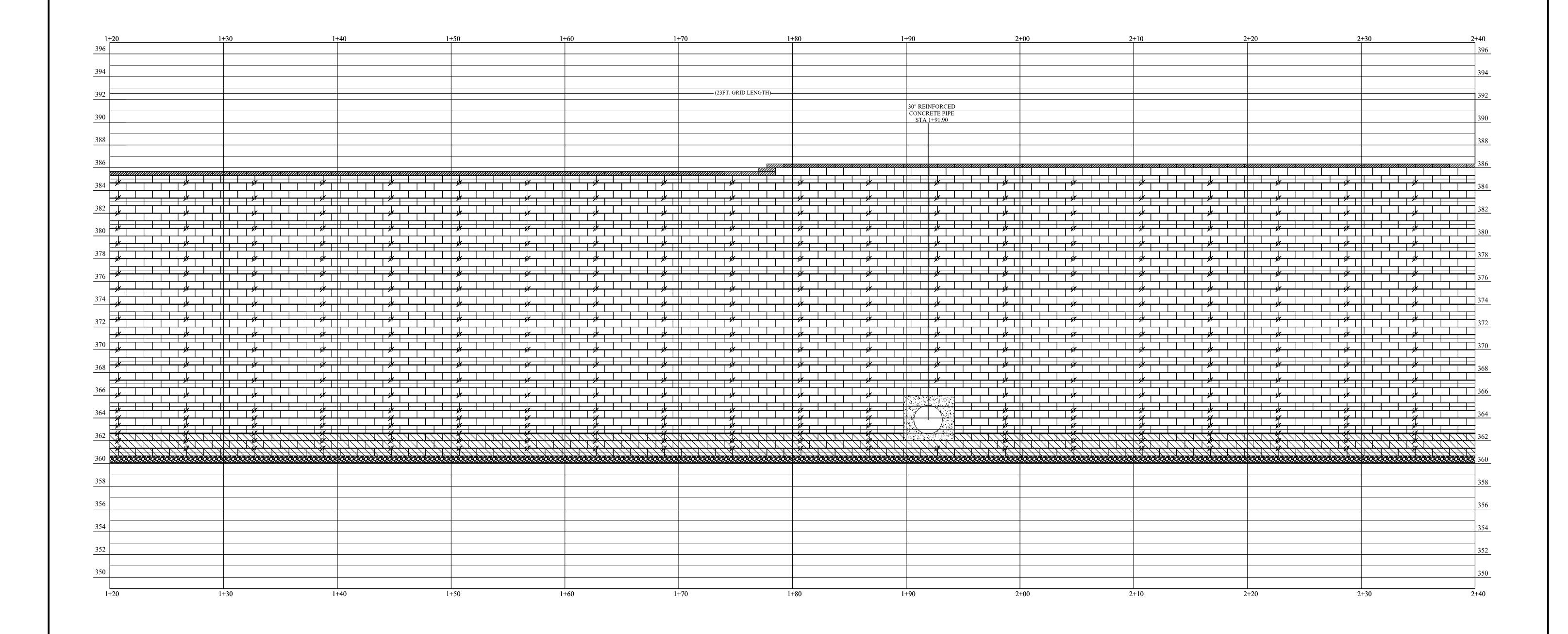
2) LEVEL BIG BAD DESIGNATION

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EV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	JRP	Retaining Wall Profile - Part 1
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23
				Designed by: Jalen G. Deatherage Date: 6/7/23 RW-3
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23
				Segmental Retaining Wall Design



NOTE: SIX (6) GRID LAYERS ARE UTILIZED WITHIN FIRST SEVEN (7) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 19 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

RETAINING WALL PROFILE - PART 2

EGEND:

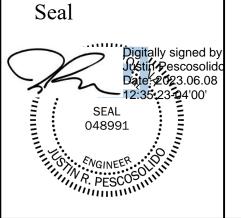
MIRAFI 3XT GEOGRID DESIGNATION

B

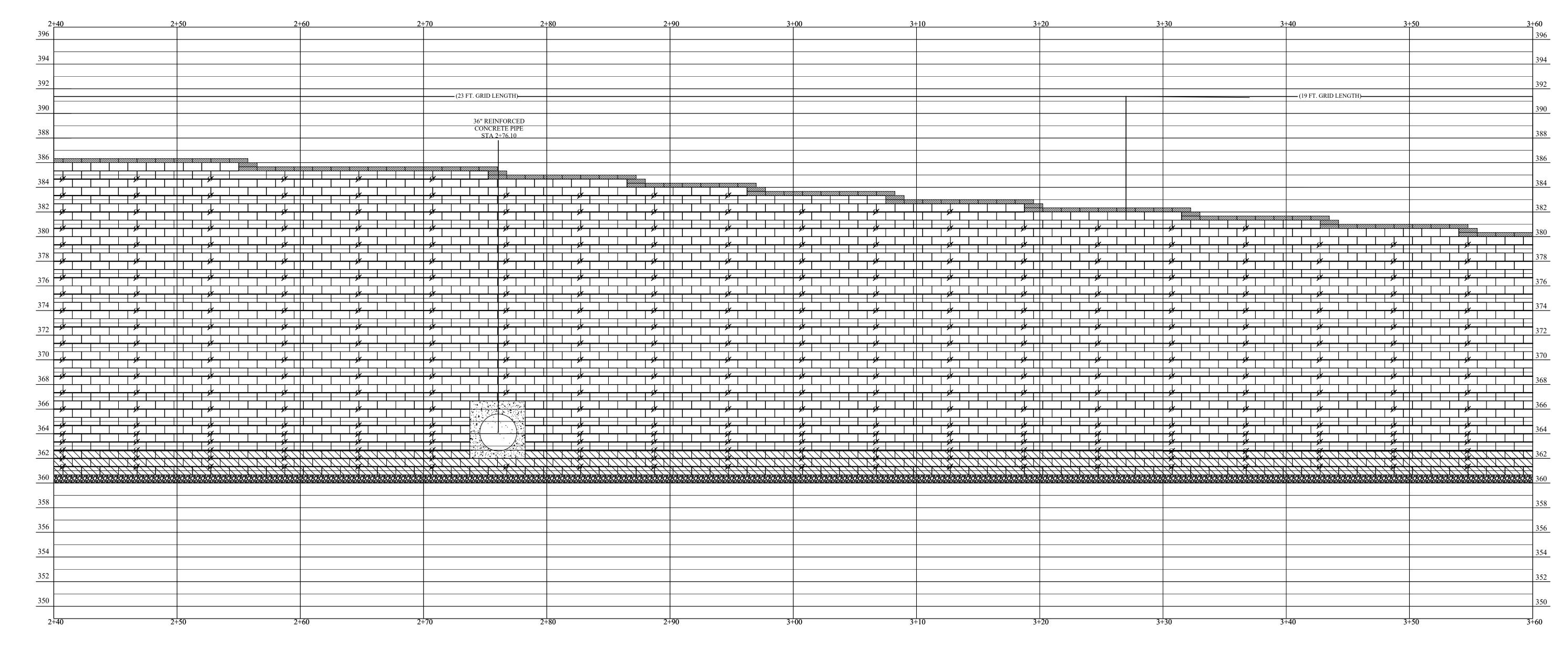
EMBEDDED BLOCK DESIGNATION

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1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	JRP	Retaining Wall Profile - Part 2
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23 RW-4
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23 Segmental Retaining Wall Design



NOTE: SIX (6) GRID LAYERS ARE UTILIZED WITHIN FIRST SEVEN (7) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 19 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

RETAINING WALL PROFILE - PART 3

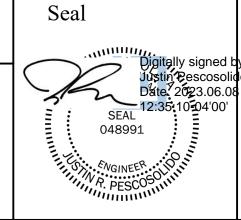
LEGEND:

). MIRAFI 3XT GEOGRID DESIGNATION

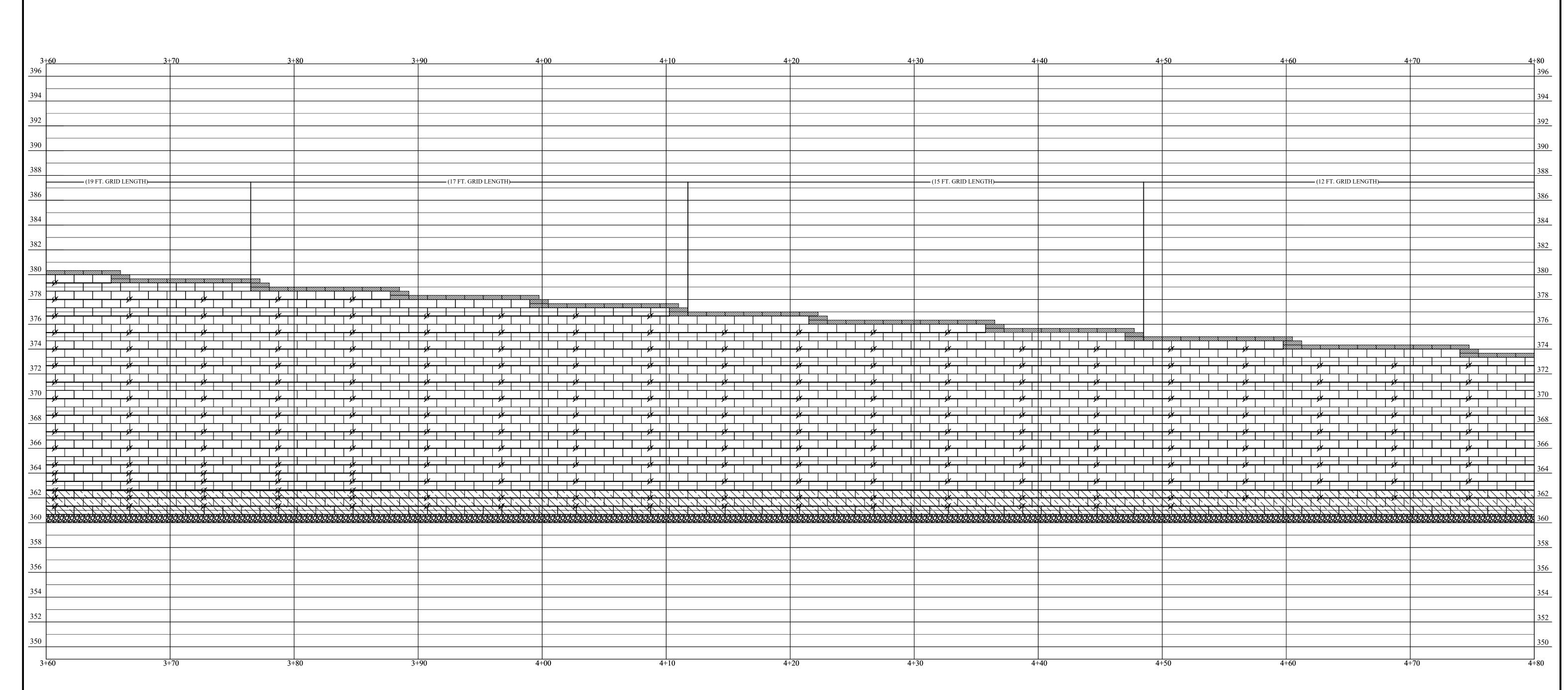
Comparison of the comparison of the

NV5 Engineers and Consultants, Inc.

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REV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	JRP	Retaining Wall Profile - Part 3	
	_			Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23	
				Designed by: Jalen G. Deatherage Date: 6/7/23 RW-	5
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23	
				Segmental Retaining Wall Design	



NOTE: TWO (2) GRID LAYERS ARE UTILIZED WITHIN FIRST THREE (3) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 15 FEET.

NOTE: SIX (6) GRID LAYERS ARE UTILIZED WITHIN FIRST SEVEN (7) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 19 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

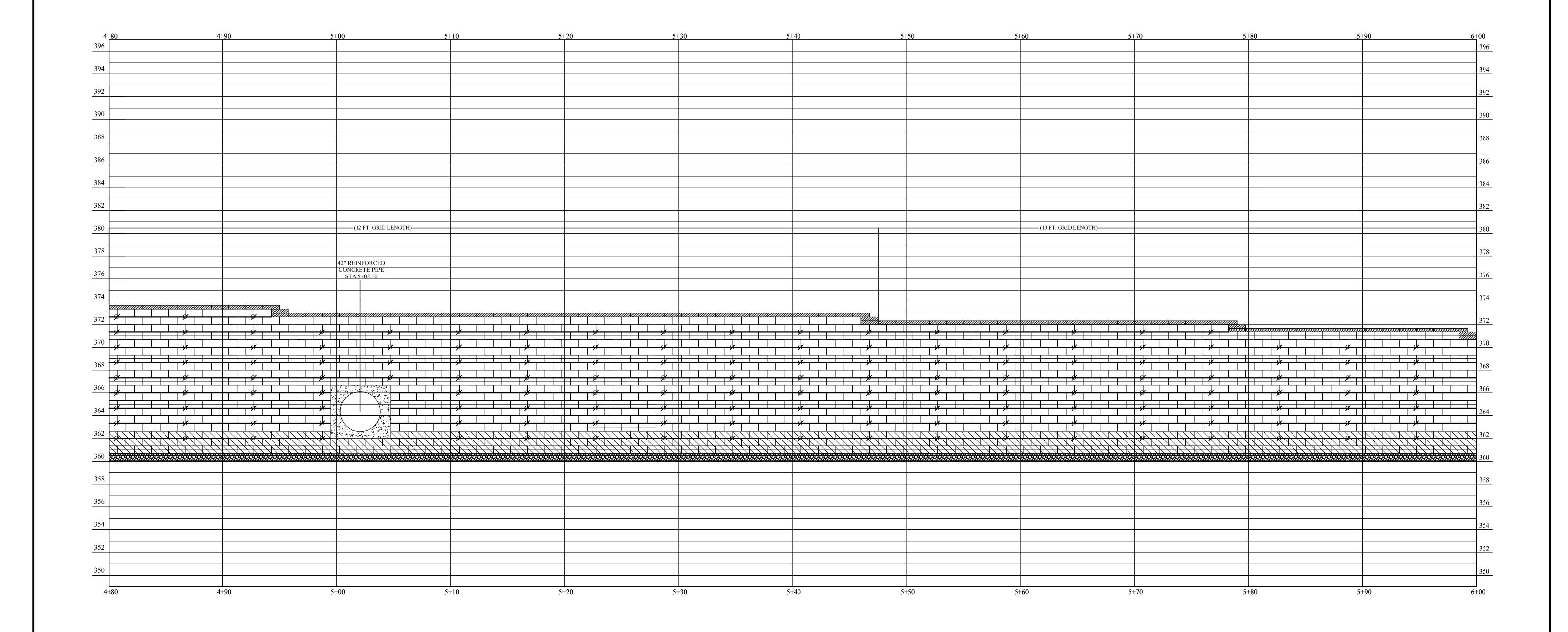
RETAINING WALL PROFILE - PART 4

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V	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Retaining Wall Pa	rofile - P	art 4
				Designed by: Justin R. Pescosolido, P.E. Designed by: Jalen G. Deatherage	Date: 6/7/23 Date: 6/7/23	SHEET RW-6
				Reviewed by: Justin R. Pescosolido, P.E. Segmental Retaining	Date: 6/7/23	sign

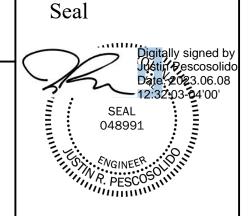


NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

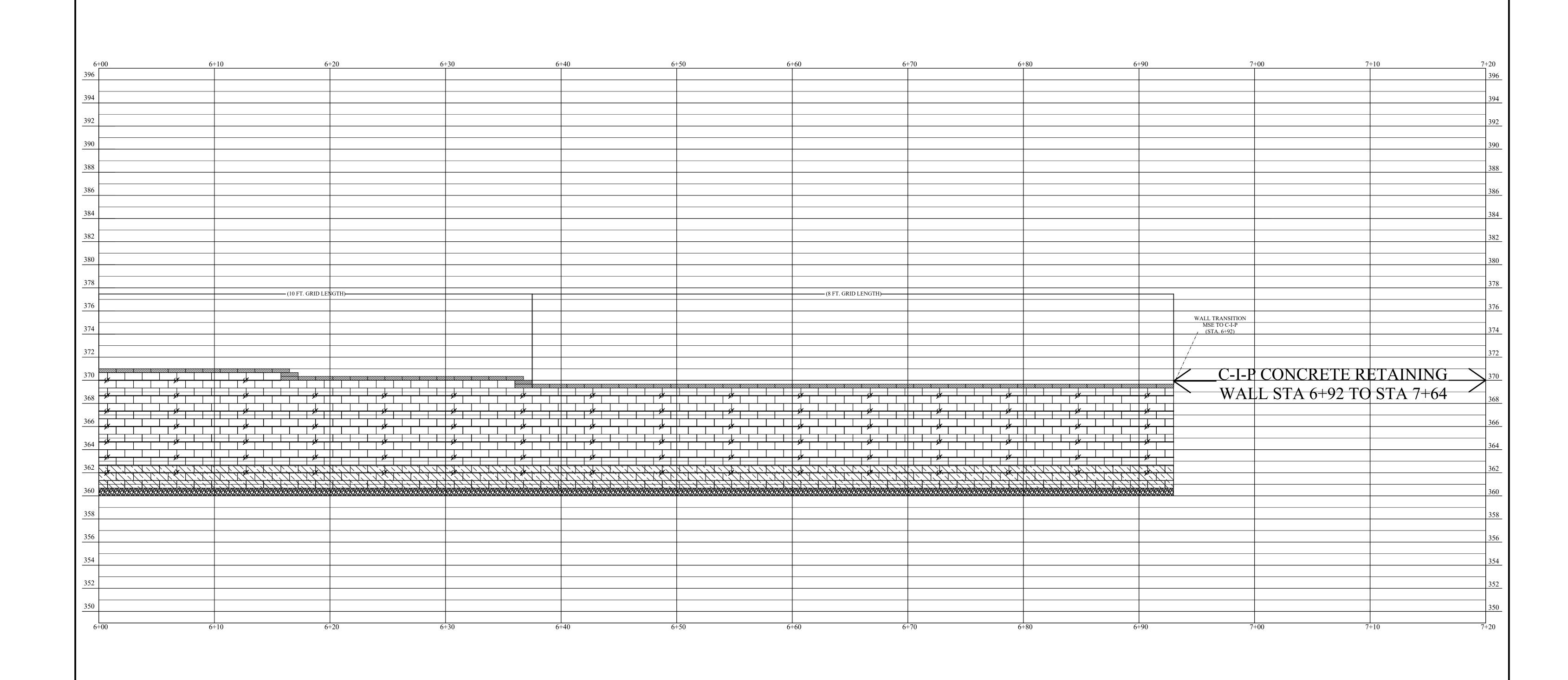
RETAINING WALL PROFILE - PART 5

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REV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Retaining Wall Profile - P	Part 5
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23 Designed by: Jalen G. Deatherage Date: 6/7/23	SHEET RW-7
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23 Segmental Retaining Wall De	sign



NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

RETAINING WALL PROFILE - PART 6

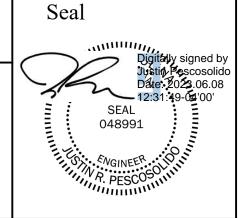
EGEND:

. MIRAFI 3XT GEOGRID DESIGNATION

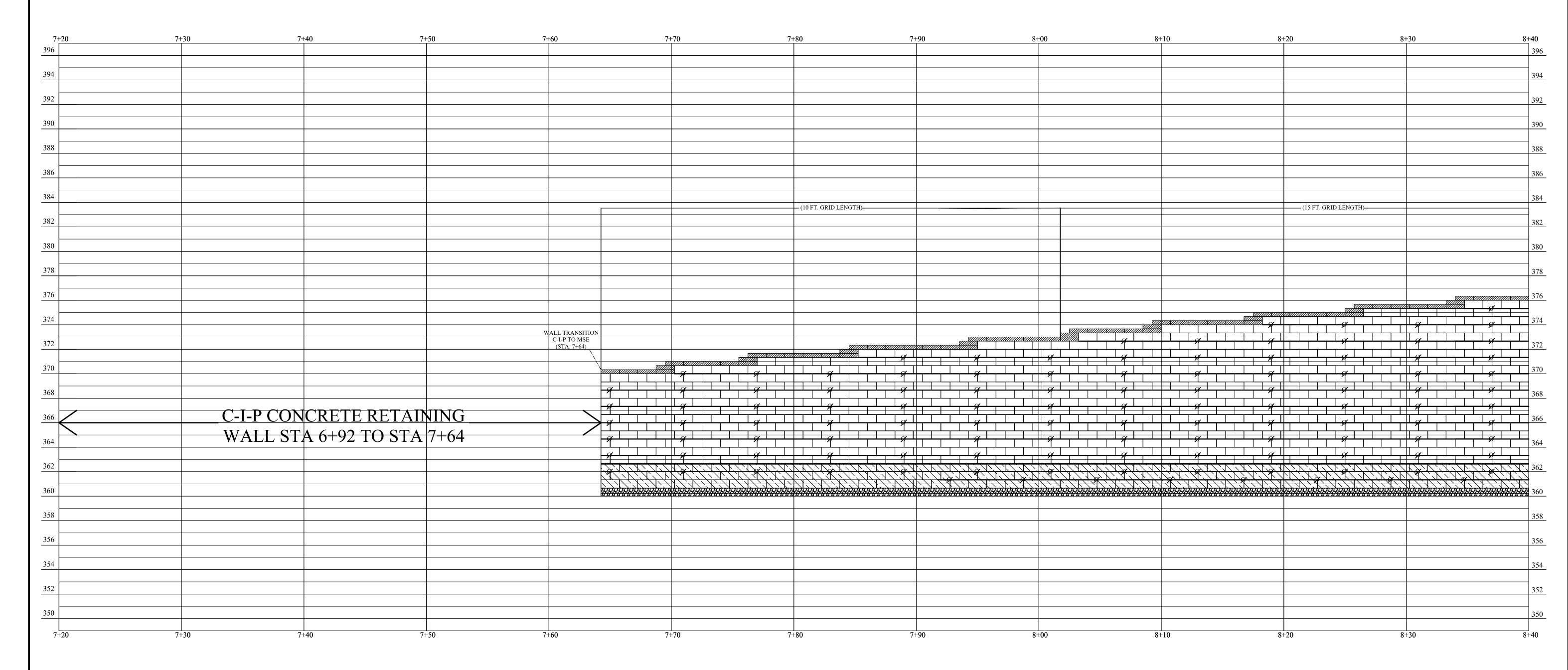
. EMBEDDED BLOCK DESIGNATION

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REV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Retaining Wall P	rofile - P	art 6
				Designed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	SHEET
				Designed by: Jalen G. Deatherage	Date: 6/7/23	RW-8
				Reviewed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	
				Segmental Retainin	g Wall De	sign



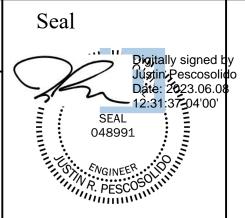
NOTE: TWO (2) GRID LAYERS ARE UTILIZED WITHIN FIRST THREE (3) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 15 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

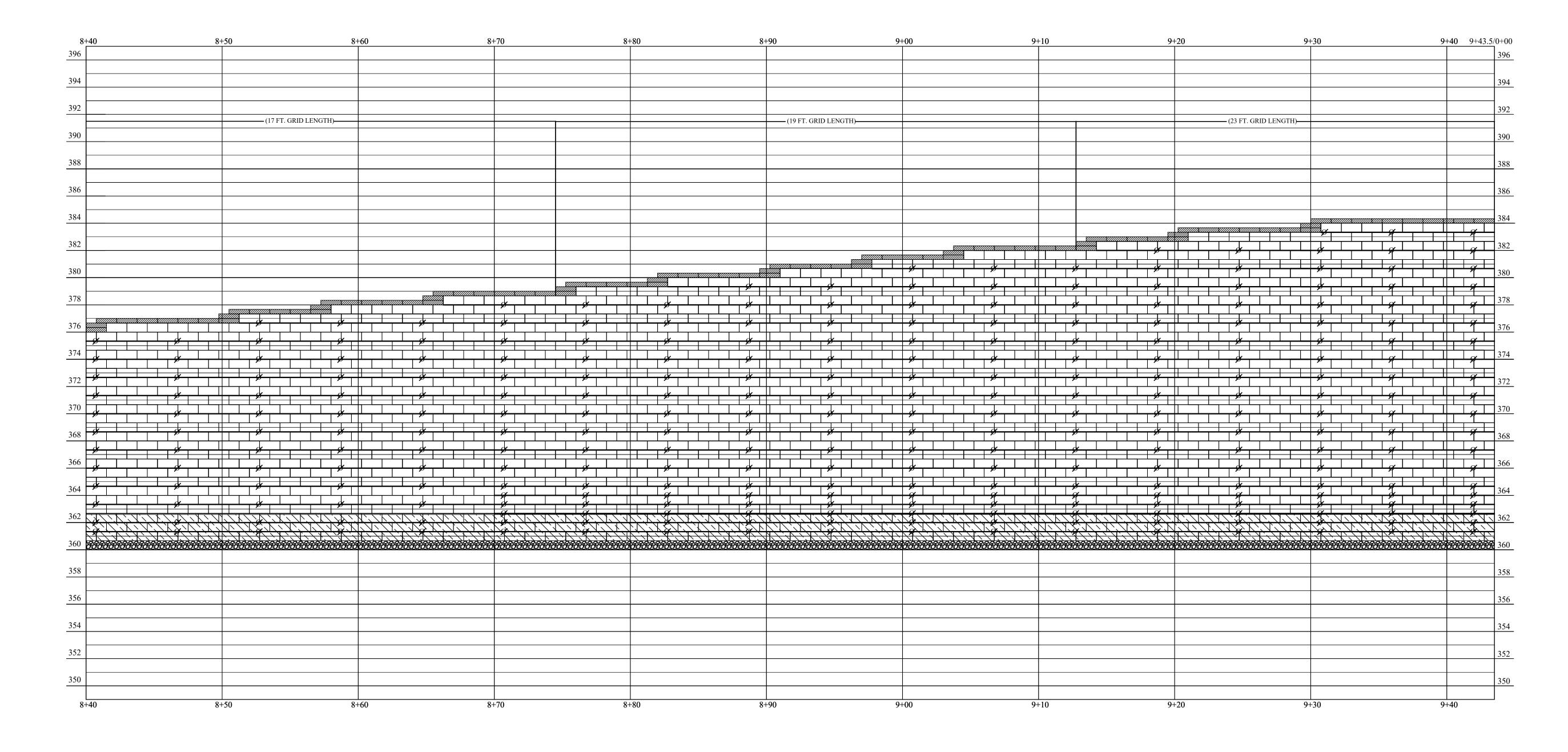
RETAINING WALL PROFILE - PART 7

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EV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Retaining Wall Profile - Part 7
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/23 Designed by: Jalen G. Deatherage Date: 6/7/23 RW-9
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/23 Segmental Retaining Wall Design



NOTE: TWO (2) GRID LAYERS ARE UTILIZED WITHIN FIRST THREE (3) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 15 FEET.

NOTE: SIX (6) GRID LAYERS ARE UTILIZED WITHIN FIRST SEVEN (7) COURSES WHERE GRID LENGTHS ARE GREATER THAN OR EQUAL TO 19 FEET.

NOTE: WASHED #57 STONE USED IN REINFORCED ZONE BELOW ELEVATION 370'. SELECT FILL USED IN REINFORCED ZONE ABOVE ELEVATION 370'. REFER TO DETAILS.

RETAINING WALL PROFILE - PART 8

1). MIRAFI 3XT GEOGRID DESIGNATION

2) EMBEDDED BLOCK DESIGNATION

3). LEVELING PAD DESIGNATION

DESCRIPTION

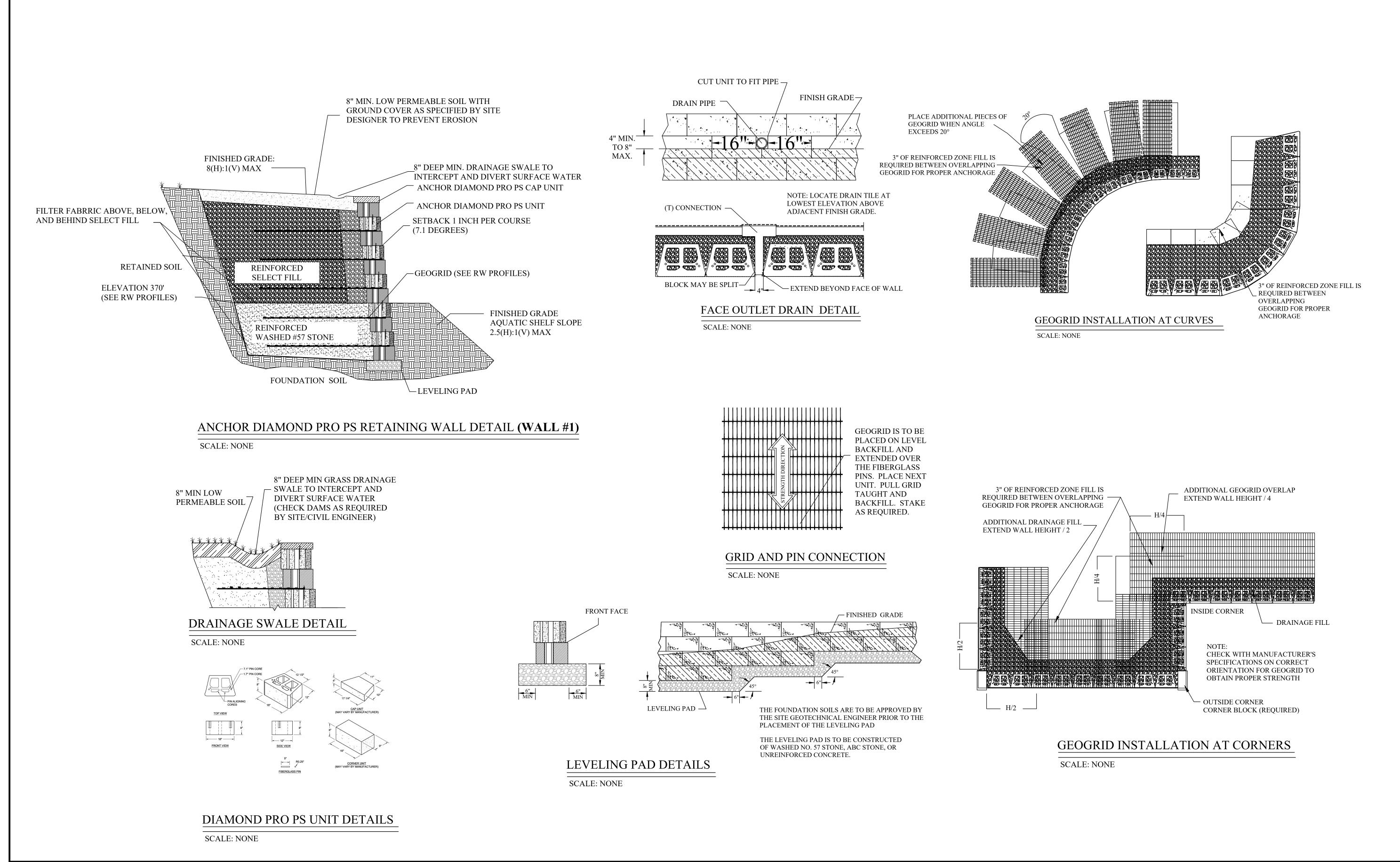
BY

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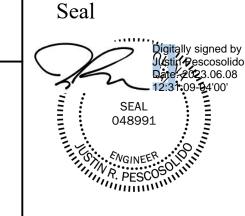


EV	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Retaining Wall Profile	- Part 8
				Designed by: Justin R. Pescosolido, P.E. Date: 6/7/ Designed by: Jalen G. Deatherage Date: 6/7/2	DXX/ 10
				Reviewed by: Justin R. Pescosolido, P.E. Date: 6/7/ Segmental Retaining Wall	23

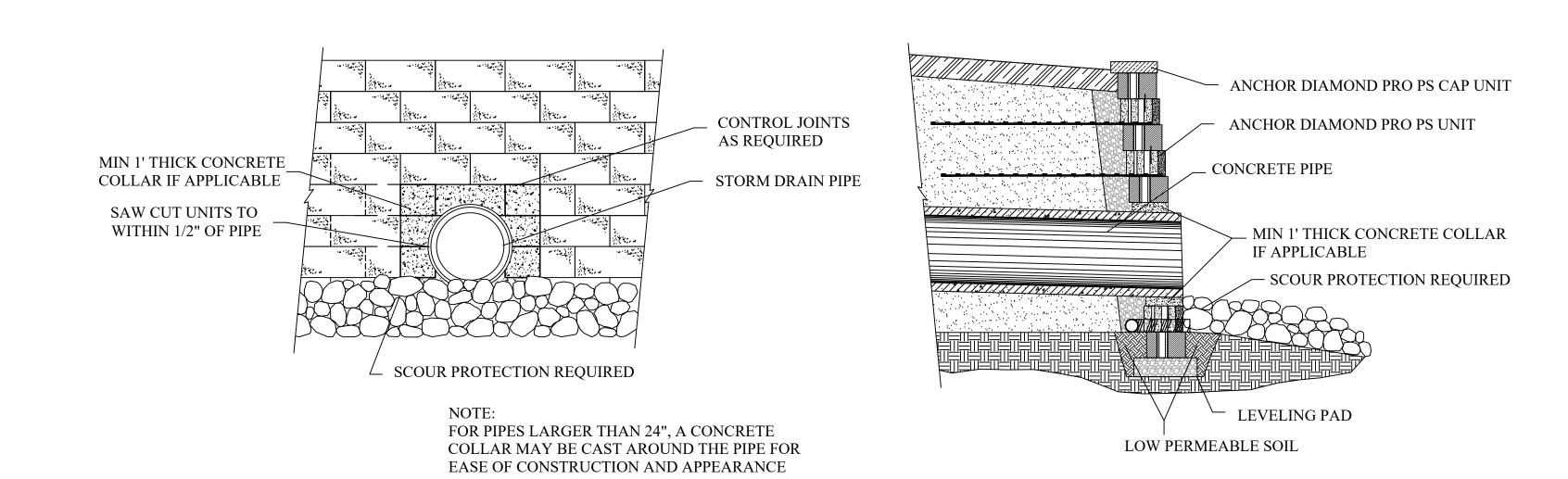


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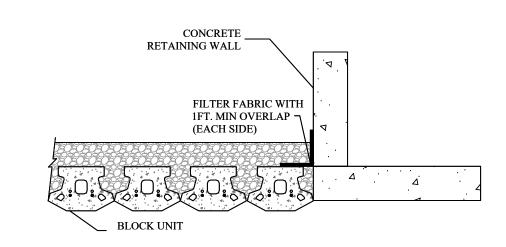


XEV 1	4-26-23	WALL TYPE/LAYOUT SCHEME 2	JRP			D
1	4 -20-25	WINDE ITTERENT OUT SCHEME 2	JIXI	Anchor Diamond Pro	PS Unit	Details
						SHEET
				Designed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	
				Designed by: Jalen G. Deatherage	Date: 6/7/23	_RW-11
				Reviewed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	
				Segmental Retainin	g Wall De	esign



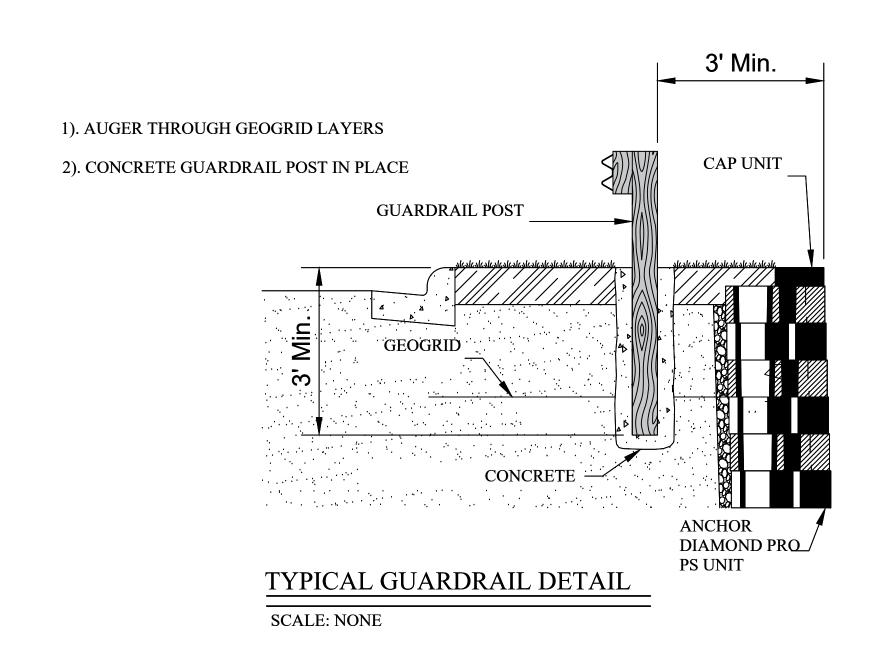
ANCHOR DIAMOND PRO PS WALL PIPE OUTLET

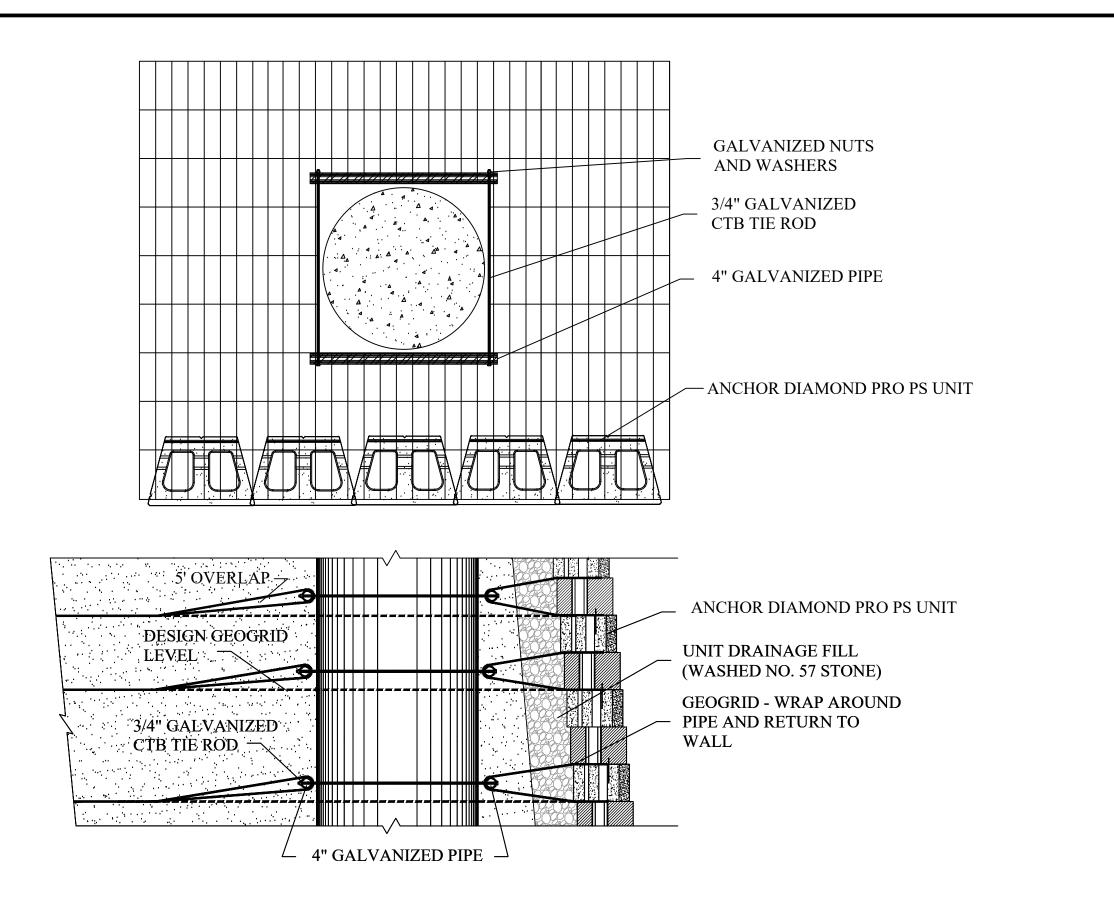
SCALE: NONE



MSE TO CONCRETE WALL CONNECTION DETAIL

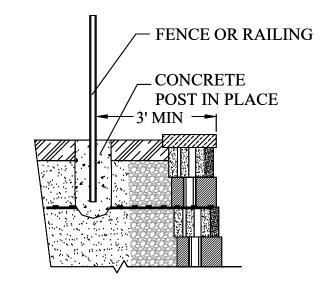
SCALE: NONE



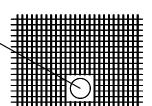


ANCHOR DIAMOND PRO PS WALL AT PIER / MANHOLE

SCALE: NONE



CUT GRID AND PLACE TUBE OR FORMS – PRIOR TO PLACING ADDITIONAL FILL



CUT SUCCESSIVE LAYERS OF GEOGRID AROUND PROPOSED FENCE POST INSTALLATION POINTS AND SET CONCRETE TUBE OR FORMS DURING WALL CONSTRUCTION. CHECK FENCE DESIGN FOR EMBEDMENT DEPTH OF FENCE POST

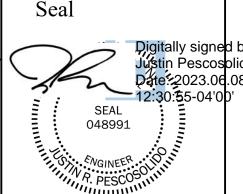
RAILING REQUIREMENTS FOR THE RETAINING WALL SHALL BE DETERMINED BY THE GENERAL CONTRACTOR. THE RAILING SHOULD BE DESIGNED IN ACCORDANCE WITH THE NORTH CAROLINA BUILDING CODE BY A REGISTERED DESIGN PROFESSIONAL. THE RAILING SHOULD BE DESIGNED SUCH THAT IT DOES NOT ADD ANY ADDITIONAL LATERAL FORCES TO THE RETAINING WALL. THE CONCRETE TUBES OR SLEEVE-IT FORMS FOR THE RAILINGS SHALL BE INSTALLED BY THE SITE CONTRACTOR AND COORDINATED WITH THE RETAINING WALL CONTRACTOR DURING CONSTRUCTION OF THE RETAINING WALL

TYPICAL HAND RAILING DETAIL

SCALE: NONE

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REV 1	4-26-23	WALL TYPE/LAYOUT SCHEME 2	JRP	Anchor Diamond Pro PS	Unit Detail	s (Cont'd)
				D ' 11 I A' D D 1'1 DE	D 4 (17/22	SHEET
				Designed by: Justin R. Pescosolido, P.E. Designed by: Jalen G. Deatherage	Date: 6/7/23 Date: 6/7/23	RW-12
				Reviewed by: Justin R. Pescosolido, P.E. Segmental Retainin	Date: 6/7/23	esign

1.0 GENERAL

SEGMENTAL RETAINING WALL SYSTEMS ARE DESIGNED AS GRAVITY RETAINING WALLS WHICH UTILIZES A HIGH DENSITY POLYESTER GEOGRID TO REINFORCE THE SOIL ZONE BEHIND THE WALL. THE GEOGRID IS POSITIVELY CONNECTED TO THE MODULAR CONCRETE BLOCK CREATING A REINFORCED SOIL MASS CAPABLE OF RESISTING LATERAL EARTH PRESSURES AND SURCHARGE LOADS. ALL REFERENCES TO THE ENGINEER REFER TO NV5 ENGINEERS AND CONSULTANTS, INC.

1.1 QUALITY ASSURANCE

WORK SHALL BE PERFORMED ONLY BY AN EXPERIENCED CONTRACTOR. CONTRACTOR SHALL SUBMIT TO THE CERTIFYING ENGINEER EVIDENCE OF QUALIFICATIONS AND REFERENCES ON PROJECTS OF SIMILAR SCOPE. THE CERTIFYING ENGINEER RESERVES THE RIGHT TO REJECT ANY AND ALL QUALIFICATIONS SUBMITTALS. THE OWNER AND/OR GENERAL CONTRACTOR SHOULD PROVIDE AN INSPECTOR AS A FULL-TIME, CONTINUOUS MONITOR OF WORK QUALITY.

1.2 BACK FILL MATERIALS

THE SOIL MATERIAL ASSOCIATED WITH THE RETAINING WALL(S) IN THE REINFORCED ZONE, THE RETAINED ZONE, OR THE FOUNDATION BEDDING SHALL HAVE, AT A MINIMUM, THE FOLLOWING PROPERTIES:

- A.) FOUNDATION SOILS
- $\Phi = 34$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 125 LBS/CU.FT $\Phi = 30$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 120 LBS/CU.FT
- B.) RETAINED SOILS
- D.) REINFORCED WASHED NO. 57 STONE $\Phi = 34$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 105 LBS/CU.FT (BELOW Elevation 370')
- E.) REINFORCED SELECT FILL
- $\Phi = 32$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 120 LBS/CU.FT (ABOVE Elevation 370')
- LABORATORY TESTING OF THE ACTUAL SOIL TO BE USED SHALL BE PERFORMED PRIOR TO CONSTRUCTION OF THE RETAINING WALL. IF THE

ASSUMED VALUES DO NOT REPRESENT THE ACTUAL SOIL CONDITIONS, THE ENGINEER SHALL BE NOTIFIED PRIOR CONSTRUCTION FOR POSSIBLE ALTERATIONS TO THIS DESIGN.

1.3 FOUNDATION LOADS

MAXIMUM DESIGN BEARING PRESSURES:

3 KIPS/SQ.FT WHERE WALL HEIGHT IS LESS THAN 18 FEET

5 KIPS/SQ.FT WHERE WALL HEIGHT IS GREATER THAN OR EQUAL TO 18 FEET

1.4 WALL BATTER

BATTER FOR THE ENTIRETY OF THE WALL SHALL BE MAINTAINED AT 7.1 DEGREES (REAR-PIN SETTING).

2.0 CONCRETE MASONRY WALL UNITS

CONCRETE WALL UNITS SHALL BE SEGMENTAL UNITS MANUFACTURED IN ACCORDANCE WITH ASTM C-1372 AND ASTM C-140, AND SHALI HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI. UNITS SHALL BE INTERLOCKED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

2.1 RETAINING WALL SHEAR CONNECTIONS

SEGMENTAL UNITS SHALL BE INTERLOCKED WITH SUITABLE SHEAR PINS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

2.2 GEOGRID REINFORCEMENT

GEOSYNTHETIC REINFORCEMENT FOR THE WALL SHALL CONSIST OF HIGH TENACITY GEOGRIDS MANUFACTURED FOR SOIL REINFORCEMENT APPLICATIONS. THE TYPE, LENGTH, AND PLACEMENT OF THE REINFORCING GEOSYNTHETIC SHALL BE AS SHOWN ON THE PLANS.

2.3 GEOTEXTILE FILTER FABRIC

GEOTEXTILE FILTER FABRIC (IF REQUIRED) SHALL CONSIST OF NEEDLE PUNCHED NON-WOVEN POLYPROPYLENE MATERIAL WHICH MEETS THE AASHTO M288-2006 CLASS 3 STRENGTH CRITERIA. IT SHALL HAVE A MAXIMUM AVERAGE ROLL VALUE OF 0.25 MM FOR ITS APPARENT OPENING SIZE AND PERMITIVITY OF AT LEAST 0.2/SEC. PRE-APPROVED NON-WOVEN GEOTEXTILES INCLUDE AMOCO 4546, CARTHAGE MILLS FX-40HS, SYNTHETIC INDUSTRIES GEOTEX 401, AND TENCATE MIRAFI 140N.

2.4 LEVELING PAD

MATERIAL SHALL CONSIST OF COMPACTED AGGREGATE BASE COURSE (ABC) STONE, WASHED NO. 57 STONE, OR UNREINFORCED CONCRETE. THE LEVELING PAD SHALL BE AT LEAST 8" IN DEPTH. IF ABC STONE IS USED, IT SHALL BE COMPACTED TO 95% OF THE MODIFIED PROCTOR (ASTM D-698) MAXIMUM DRY DENSITY. AGGREGATE MATERIAL SHALL RECEIVE A MINIMUM OF ONE PASS OF THE COMPACTION EQUIPMENT. THE LEVELING PAD TOP FOR THE WALL SECTIONS SHALL BE MAINTAINED AS SHOWN ON THE RETAINING WALL PROFILES.

2.5 UNIT FILL

THE VOID WITHIN AND BETWEEN EACH UNIT SHALL BE FILLED WITH WASHED (NO. 57) STONE HAVING 100% OF THE AGGREGATE PASSING THE 2" SIEVE. MINIMUM 3/8" WASHED STONE SIZE IS REQUIRED (NO MORE THAN 5% PASSING THE NO. 200 SIEVE). A MINIMUM OF 12 INCHES OF THIS SAME MATERIAL SHALL BE PLACED AT THE BACK OF EACH BLOCK AS INDICATED ON THE ROCKWOOD CLASSIC 8 UNIT DETAILS SHEET. EACH COURSE SHALL BE COMPLETELY FILLED AND EXCESS MATERIAL SWEPT CLEAN FROM THE TOP BLOCK BEFORE INSTALLING THE NEXT COURSE.

2.6 SOIL PROPERTIES

THE FILL MATERIALS IN THE UPPER REINFORCED ZONE (ABOVE EL 370') SHALL CONSIST OF SELECT FILL. THE FILL MATERIALS IN THE LOWER REINFORCED ZONE (BELOW EL. 370') WASHED NO. 57 STONE (BELOW ELEVATION 370') (REFER TO DETAILS). ABC STONE SHALL BE UTILIZED BELOW POND BASIN ELEVATION. "SELECT FILL" SHALL CLASSIFY AS GM, GP, SP, SP-SM, SM OR SC WITH NO MORE THAN 30% FINES (W/W). THE MINIMUM INTERNAL ANGLE OF FRICTION, COHESION, AND WET UNIT WEIGHT SHALL BE EQUAL TO OR GREATER THAN THE DESIGN VALUES PROVIDED IN SECTION 1.2. LABORATORY TESTING OF THE PROPOSED FILL MATERIALS SHALL BE PERFORMED PRIOR TO PLACEMENT TO VERIFY DESIGN CRITERIA. COPIES OF ALL LABORATORY TESTING SHALL BE PROVIDED TO THE CERTIFYING ENGINEER PRIOR TO USE IN CONSTRUCTION OF THE RETAINING WALL.

2.7 UNSUITABLE MATERIAL

SOILS CONTAINING ROOTS, BRUSH, SOD, OR OTHER ORGANIC MATERIAL SHALL NOT BE PERMITTED AS FILL. FROZEN SOILS, SNOW, ICE, HEAVY CLAYS, OR WET SOILS SHALL NOT BE PERMITTED AS FILL. MATERIAL PASSING THE NO. 40 SIEVE SHALL NOT HAVE A LIQUID LIMIT GREATER THAN 40 AND A PLASTICITY INDEX OF GREATER THAN 15, UNLESS WRITTEN CONSENT IS OBTAINED FROM THE DESIGN ENGINEER PRIOR TO PLACEMENT.

3.0 FOUNDATION REQUIREMENTS

THE FOUNDATION BEARING CAPACITY ASSUMED FOR THIS DESIGN SHALL BE VERIFIED IN THE FIELD AND COPIES OF THE TESTS PROVIDED TO THE CERTIFYING ENGINEER. THE FOUNDATION SURFACE SHALL BE CLEARED OF ALL DEBRIS AND LOOSE SOIL. FOUNDATION SOILS NOT MEETING THE MINIMUM DESIGN CRITERIA, SHALL BE REMOVED AND REPLACED UTILIZING AGGREGATE BASE COURSE (ABC) STONE, PLACED IN LIFTS NOT EXCEEDING 9", AND COMPACTED TO AT LEAST 95% OF THE MODIFIED PROCTOR (ASTM D-698) MAXIMUM DRY DENSITY.

3.1 FIRST BLOCK COURSE

THE FIRST COURSE OF BLOCK SHALL BE PLACED ON TOP OF AND IN FULL CONTACT WITH THE LEVELING PAD. THE UNITS SHALL MAINTAIN A MINIMUM DISTANCE OF 6" FROM THE FRONT AND BACK OF THE LEVELING PAD. PROPER ALIGNMENT MAY BE ACHIEVED WITH THE AID OF A STRING LINE. PROCEED TO THE NEXT COURSE OF BLOCK. EACH UNIT SHALL BE IN CONTACT WITH THE UNITS ON BOTH SIDES AS WELL AS ABOVE AND BELOW. SOME ADJUSTMENTS MAY BE REQUIRED FOR WALLS WITH CURVES AND A BATTER.

3.2 GEOGRID INSTALLATION

THE GEOGRID REINFORCEMENT SHALL BE LAID HORIZONTALLY ON COMPACTED FILL AND CONNECTED TO THE CONCRETE WALL UNITS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. GEOGRID SHALL BE PULLED TAUT REMOVING ALL SLACK FROM THE MATERIAL AND ANCHORED BEFORE ADDING FILL. GEOGRID SHALL BE INSTALLED AT THE ELEVATIONS AND LENGTHS REQUIRED AS SHOWN ON THE PLANS. THE SOIL SURFACE SHALL BE SMOOTH AND LEVEL AND HAVE BEEN COMPACTED BEFORE INSTALLING THE GEOGRID.

3.3 REINFORCED ZONE FILL PLACEMENT

SELECT FILL (OR ABC STONE) SHALL BE PLACED IN A MAXIMUM 8" LOOSE LIFT THAT IS COMPACTED TO AT LEAST 95% OF THE STANDARD (MODIFIED) PROCTOR MAXIMUM DRY DENSITY AT A MOISTURE CONTENT WITHIN 3% OF THE OPTIMUM MOISTURE CONTENT. CLEAN STONE FILL SHALL RECEIVE AT LEAST 3 PASSES OF SUITABLE COMPACTION EQUIPMENT. ONLY HAND OPERATED EQUIPMENT SHALL BE ALLOWED WITHIN 3 FEET OF THE SEGMENTAL UNITS. FILL SHALL BE PLACED FROM THE WALL REARWARD TO INSURE TAUTNESS OF THE GEOGRID. CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID.

3.4 RETAINING WALL CAPS

APPLY A CONSTRUCTION ADHESIVE TO THE UNITS TO PREVENT THEIR REMOVAL

4.0 SOIL TESTING

FOR ABC STONE FILL MATERIALS, AT LEAST ONE COMPACTION TEST SHALL BE PERFORMED FOR EVERY 100 LINEAR FEET, FOR EVERY LIFT ELEVATION REQUIRING GEOGRID OR EVERY 3RD LIFT AT A MINIMUM. TEST RESULTS SHALL BE PROVIDED TO THE CERTIFYING ENGINEER.

5.0 HYDROSTATIC PRESSURE POTENTIAL

THE ENGINEER SHALL BE NOTIFIED IF ANY OF THE FOLLOWING SHOULD BECOME EVIDENT: WATER OR WETNESS FROM OR IN A CUT BANK; LOCAL SPRINGS, LOCAL STORM DRAINS, SEWER, OR WATER LINES UNDER OR BEHIND THE WALL.

6.0 ACCEPTABLE BLOCK

SEGMENTAL BLOCK UNITS SHALL BE USED AND KEPT FREE OF DEFECTS THAT WOULD INTERFERE WITH THE PLACING OR POSITIONING OF THE UNIT OR IMPAIR ITS STRENGTH. THE CONTRACTOR SHALL PREVENT EXCESS MUD, WET CEMENT, EPOXY, AND THE LIKE MATERIALS FROM COMING IN CONTACT WITH AND AFFIXING TO THE UNITS. MINOR CRACKS INCIDENTAL TO THE USUAL METHOD MANUFACTURING OR MINOR CHIPPING RESULTING FROM SHIPMENT AND DELIVERY ARE NOT GROUNDS FOR REJECTION.

7.0 ACCEPTABLE GEOGRID

GEOGRID SHALL BE REJECTED IF 20% OR MORE OF A STRUCTURAL RIB HAS BEEN CUT OR RIPPED. THE CONTRACTOR SHALL INSPECT ALL GEOGRID DELIVERED TO THE SITE AND REJECT MATERIALS THAT MEET THIS CRITERIA. THE CONTRACTOR SHALL PREVENT EXCESS MUD, WET CEMENT, EPOXY, AND THE LIKE MATERIALS FROM COMING IN CONTACT WITH AND AFFIXING TO THE GEOGRID MATERIAL. IF THE GEOGRID IS DAMAGED ONSITE, IT SHALL BE REPLACED AT THE CONTRACTORS EXPENSE.

8.0 DRAINAGE COMPOSITE

DRAINAGE COMPOSITE IS NOT REQUIRED.

9.0 SPECIAL PROVISIONS

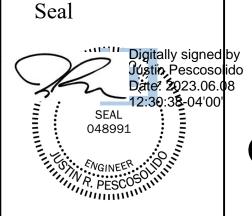
- A). MAINTAIN DRAINAGE AWAY FROM THE WALL FACE AT ALL TIMES DURING CONSTRUCTION OF THE RETAINING WALL.
- B). THE ENGINEER SHALL BE NOTIFIED BY THE INSTALLING CONTRACTOR IF THE EMBEDMENT DEPTH OF THE RETAINING WALL(S) IS LESS THAN THAT SHOWN ON THE RETAINING WALL PROFILES.
- C). AS PER THE NORTH CAROLINA STATE BUILDING CODE, A BUILDING PERMIT MUST BE OBTAINED PRIOR TO WALL CONSTRUCTION. THE CONTRACTOR SHALL CONTACT THE LOCAL MUNICIPALITY CODE ENFORCEMENT DIVISION TO OBTAIN A BUILDING PERMIT
- D). PLACEMENT OF STRUCTURAL FILL SOILS IS EXPECTED TO OCCUR AT PORTIONS OF THE LOW-SIDE OF THE RETAINING WALL TO ACCOMMODATE THE PLANNED EMERGENCY SPILLWAY. RIP-RAP SCOUR PROTECTION (DESIGNED BY OTHERS) SHALL BE PLACED AT THE BASE OF THE WALL IN THESE AREAS. ADDITIONALLY, RIP-RAP SCOUR PROTECTION (DBO) SHOULD BE INSTALLED AS PER SHEET RW-2.
- E). PLACEMENT OF LOW PERMEABLE SOILS AND CONSTRUCTION OF THE TOP-OF-WALL DRAINAGE SWALE SHALL OCCUR IMMEDIATELY AFTER WALL CONSTRUCTION. IF LOCALIZED LOW-LYING AREAS ARE PRESENT AT THE BOTTOM OF THE WALL, SCOUR PROTECTION SHALI BE COORDINATED WITH NV5.
- F). IF ANY PORTION OF THE REINFORCED ZONE OF THIS WALL IS EXPECTED TO ENCROACH INTO ANY ADJACENT PROPERTY, BUILDING AREA OR PAVEMENT AREA, A PERMANENT CONSTRUCTION EASEMENT SHALL BE FILED FOR THE AFFECTED ENTITIES.
- G). LARGE VEGETATION WITHIN FIVE FEET OF THE TOP OF THE WALL SHALL BE COORDINATED WITH NV5 TO PREVENT DAMAGE TO THE REINFORCED ZONE OF THE RETAINING WALL.
- H). PROPER FUNCTIONALITY OF THE ADJACENT STORMWATER POND IS CRITICAL TO LONG-TERM WALL PERFORMANCE. REGULAR MAINTENANCE OF THE POND IS SOLELY THE RESPONSIBILITY OF THE OWNER.

10.0 QUALIFICATION OF DESIGN

- A). STABILITY OF ANY TEMPORARY SLOPES REQUIRED BY THE INSTALLATION OF A SEGMENTAL RETAINING WALL SHALL BE ADDRESSED BY A QUALIFIED GEOTECHNICAL ENGINEER. RESPONSIBILITY OF THESE TEMPORARY SLOPES RESTS WITH THE OWNER AND/OR THE CONTRACTOR OF THE PROJECT. ALL SLOPES SHALL MEET CURRENT OSHA STANDARDS.
- B). HANDRAIL/GUARDRAIL REQUIREMENTS SHALL BE DETERMINED BY THE ARCHITECT OR GENERAL CONTRACTOR.
- C). NOTIFY THE DESIGN ENGINEER PRIOR TO MODIFYING WALL CONSTRUCTION IF EXISTING SITE CONDITIONS DEVIATE FROM CONDITIONS OUTLINED ON THE RETAINING WALL PROFILE.

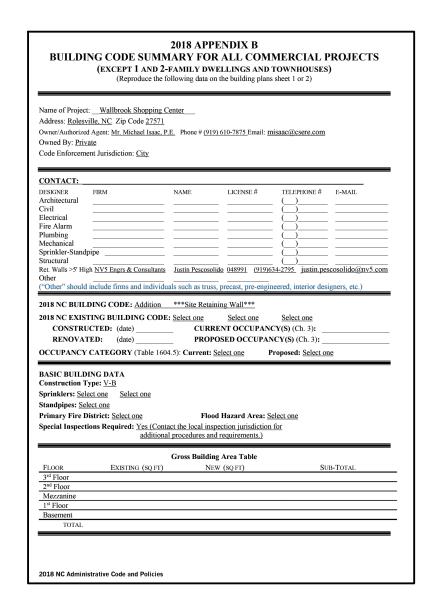
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4905 Professional Court Raleigh, North Carolina 27609 Phone: (919) 876-9799 Fax: (919) 876-8291 North Carolina Corporate License No. F-1333



REV 1	DATE 4-26-23	DESCRIPTION WALL TYPE/LAYOUT SCHEME 2	BY JRP	Specifica	tions	
				Designed by: Justin R. Pescosolido, P.E.	Date: 6/7/23 Date: 6/7/23	SHEET RW-13
				Designed by: Jalen G. Deatherage Reviewed by: Justin R. Pescosolido, P.E.	Date: 6/7/23	
				Segmental Retainin	g Wall De	esign

RETAINING WALL DESIGN FOR SHOPS AT WALLBROOK ROLESVILLE, NORTH CAROLINA



	etual Area of Occupan wable Area of Occupa		etual Area of Oc wable Area of C		= <u>≤ 1.00</u>
STORY NO.	DESCRIPTION AND USE	(A) BLDG AREA PER STORY (ACTUAL)	(B) TABLE 506.2 ⁴ AREA	(C) AREA FOR FRONTAGE INCREASE ^{1,5}	(D) ALLOWABLE AREA PER STORY OR UNLIMITED ^{2,3}
a. Perim b. Total c. Ratio d. W = 1 e. Perce Unlimited and Maximum B	Building Perimeter (F/P) = Minimum width of purity of frontage increase a applicable under cuilding Area = total numbers of open parkinum area open a	public way or open (F/P) ablic way = se $I_f = 100[F/P - 0]$ conditions of Sectic tumber of stories in g garages must co	space having 20 (P) (W) (25] x W/30 = 1 on 507. In the building x mply with Table	D (maximum 3 storie e 406.5.4.	
Tontage incl	rease is based on the	ALLOW	ABLE HEIGE	iT .	CODE DESERVAÇE
		ALLOW			CODE REFERENCE ¹
Building Hei	ight in Feet (Table 504 ight in Stories (Table 50	ALLOW AL. (33) 2 (94.4) 3	ABLE HEIGE	SHOWN ON PLANS	
Building Hei Building Hei Provide code 'he maximu	ight in Feet (Table 504 ight in Stories (Table 50	ALLOW AL 3) 2 14.4) 3 own on Plans" quai control towers m	ABLE HEIGH LOWABLE attity is not basee ast comply with	SHOWN ON PLANS d on Table 504.3 or 5 Table 412.3.1.	

ALLOWABLE AREA

Primary Occupancy Classification(s): Select one Select one Select one Select one Select one

Incidental Uses (Table 509):

Special Uses (Chapter 4 - List Code Sections):

BUILDING ELEMENT	FIRE SEPARATION DISTANCE (FEET)	REQ'D	PROVIDED (W/* REDUCTION)	DETAIL # AND SHEET #	DESIGN # FOR RATED ASSEMBLY	SHEET # FOR RATED PENETRATION	SHEET # FOR RATED JOINTS
Structural Frame, ncluding columns, girders, russes	(FEE1)				ASSEMBLI		JOEVIS
Bearing Walls							
Exterior							
North							
East							
West							
South							
Interior							
Nonbearing Walls and Partitions Exterior walls							
North							
East							
West							
South							
Interior walls and partitions							
Floor Construction Including supporting beams and joists							
Floor Ceiling Assembly		-					
Columns Supporting Floors Roof Construction, including supporting beams and joists							
Roof Ceiling Assembly							
Columns Supporting Roof							
Shaft Enclosures - Exit							
Shaft Enclosures - Other							
Corridor Separation							
Occupancy/Fire Barrier Separat	ion	 					
Party/Fire Wall Separation Smoke Barrier Separation							
Smoke Partition							
Tenant/Dwelling Unit/							
Sleeping Unit Separation							
ncidental Use Separation	tuto o o to ot						
dicate section number perm	ntting reduction						

FIRE SEPARATION DISTANCE	DEGREE OF OPENINGS	LL OPENING CALCUI ALLOWABLE AREA	ACTUAL SHOWN ON PLAN
(FEET) FROM PROPERTY LINES	PROTECTION (TABLE 705.8)	(%)	(%)
	(TABLE 705.0)		
	LIFE SAFETY SYSTE	M REQUIREMENTS	
Emergency Lighting:	Select one		
Exit Signs:	Select one		
Fire Alarm: Smoke Detection Systems:	Select one Select one		
Carbon Monoxide Detection:			
	LIFE SAFETY PLAN R	EQUIREMENTS	
fe Safety Plan Sheet #:	<u> </u>		
Fire and/or smoke rated	wall locations (Chapter 7)		
	ty line locations (if not on the	* /	
= ' '	ea with respect to distance to a		*
Occupancy Use for each Occupant loads for each	area as it relates to occupant le	oad calculation (Table 1004	4.1.2)
Exit access travel distance			
		006 2 2(1))	
Common path of travel d	listances (Tables 1006.2.1 & 1	006.3.2(1))	
Common path of travel d Dead end lengths (1020.		006.3.2(1))	
Dead end lengths (1020.4	4) h exit door		
Dead end lengths (1020. Clear exit widths for each Maximum calculated occ	4) h exit door cupant load capacity each exit		sed on egress width (1005.3
Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for	4) h exit door cupant load capacity each exit each exit door	door can accommodate bas	
Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for	4) h exit door cupant load capacity each exit each exit door n indicating where fire rated fl	door can accommodate bas	
Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy su Location of doors with p	4) h exit door upant load capacity each exit each exit door i indicating where fire rated fi eparation anic hardware (1010.1.10)	door can accommodate bas	acture is provided for
Dead end lengths (1020. Clear exit widths for eacl Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy si Location of doors with p Location of doors with d	4) h exit door uppant load capacity each exit each exit door n indicating where fire rated fi eparation anic hardware (1010.1.10) elayed egress locks and the am	door can accommodate bas oor/ceiling and/or roof stru- tount of delay (1010.1.9.7)	acture is provided for
Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy se Location of doors with pe Location of doors with de Location of doors with de	4) h exit door upant load capacity each exit each exit door n indicating where fire rated fi eparation anic hardware (1010.1.10) elayed egress locks and the an lectromagnetic egress locks (1	door can accommodate bas oor/ceiling and/or roof stru- tount of delay (1010.1.9.7)	acture is provided for
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Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy se Location of doors with pe Location of doors with de Location of doors with de	4) h exit door upant load capacity each exit each exit door n indicating where fire rated fleparation anic hardware (1010.1.10) elayed egress locks and the an lectromagnetic egress locks (1) ed with hold-open devices scape windows (1030)	door can accommodate bas oor/ceiling and/or roof stru- tount of delay (1010.1.9.7)	acture is provided for
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Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy se Location of doors with pe Location of doors with de Location of doors equipp Location of doors equipp Location of doors equipp Location of emergency e The square footage of ea The square footage of ea	4) h exit door upant load capacity each exit each exit door n indicating where fire rated fi eparation anic hardware (1010.1.10) elayed egress locks and the an lectromagnetic egress locks (1 ed with hold-open devices scape windows (1030) ch fire area (202) ch smoke compartment for Oc	door can accommodate bas oor/ceiling and/or roof stru ount of delay (1010.1.9.7) 010.1.9.9)	ucture is provided for
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Dead end lengths (1020. Clear exit widths for each Maximum calculated occ Actual occupant load for A separate schematic pla purposes of occupancy se Location of doors with pe Location of doors with de Location of doors equipp Location of doors equipp Location of doors equipp Location of emergency e The square footage of ea The square footage of ea	4) h exit door upant load capacity each exit each exit door n indicating where fire rated fi eparation anic hardware (1010.1.10) elayed egress locks and the an lectromagnetic egress locks (1 ed with hold-open devices scape windows (1030) ch fire area (202) ch smoke compartment for Oc	door can accommodate bas oor/ceiling and/or roof stru ount of delay (1010.1.9.7) 010.1.9.9)	ucture is provided for

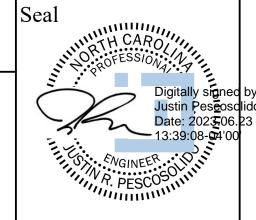
Item				Detailed Instructions and Frequencies
Verify the properties of the in-place and proposed soil materials	Continuous	7	Periodic	Materials shall meet the assumed design values in the "Back Fi Materials" and "Soil Properties" sections of the Specifications
Verify the suitability of in-place and proposed soil materials	Continuous	7	Periodic	Materials shall meet the requirements of the "Soil Properties "Unsuitable Material", 'Foundation Requirements', and "Structur Fill Placement" sections of the Specifications
Verify subgrade is adequate to achieve design bearing capacity	Continuous	V	Periodic	Bearing capacity of foundation and reinforced zone shall be accordance with the "Foundation Load" and "Foundation Requirements" sections of the Specifications
Verify leveling pad material and proper depth	Continuous	7	Periodic	Materials shall meet requirements of the "Leveling Pad" section of the Specifications
Verify placement, setback, and connection method of units	✓ Continuous		Periodic	Retaining Wall Plans Detail and "Wall Batter", "Retaining Wa Shear Connections", "First Block Course" and "Retaining Wall Ca Units" sections of the Specifications
Verify installation of drainage tile and drainage system	✓ Continuous		Periodic	Retaining Wall Plans Detail and "Unit Fill" and "Hydrostat Pressure Potential" sections of the Specifications
Verify geogrid material, length, and elevation placement	✓ Continuous		Periodic	Materials shall be as indicated on the RW Profile and meet the requirements of the "Geogrid Reinforcement", "Geogrid Installation", & "Acceptable Geogrid" sections of the Specification.
Verify density, moisture content, and lift thickness for wall fill	✓ Continuous	7	Periodic	Placement of fill shall meet requirements of the "Structural Fi Placement" and "Soil Testing" sections of the Specifications
A summary of testing services, sign Professional prior to issuate 4905 Professional Court Raleigh, North Carolina 27609 Phone (919) 876-9799 Fax (919) www.nv5.com	nce of the fina			ered professional engineer shall be provided to the Registered report. Registered Design Professional Justin R. Pescosolido, P.E.

Plan Sheet Index:

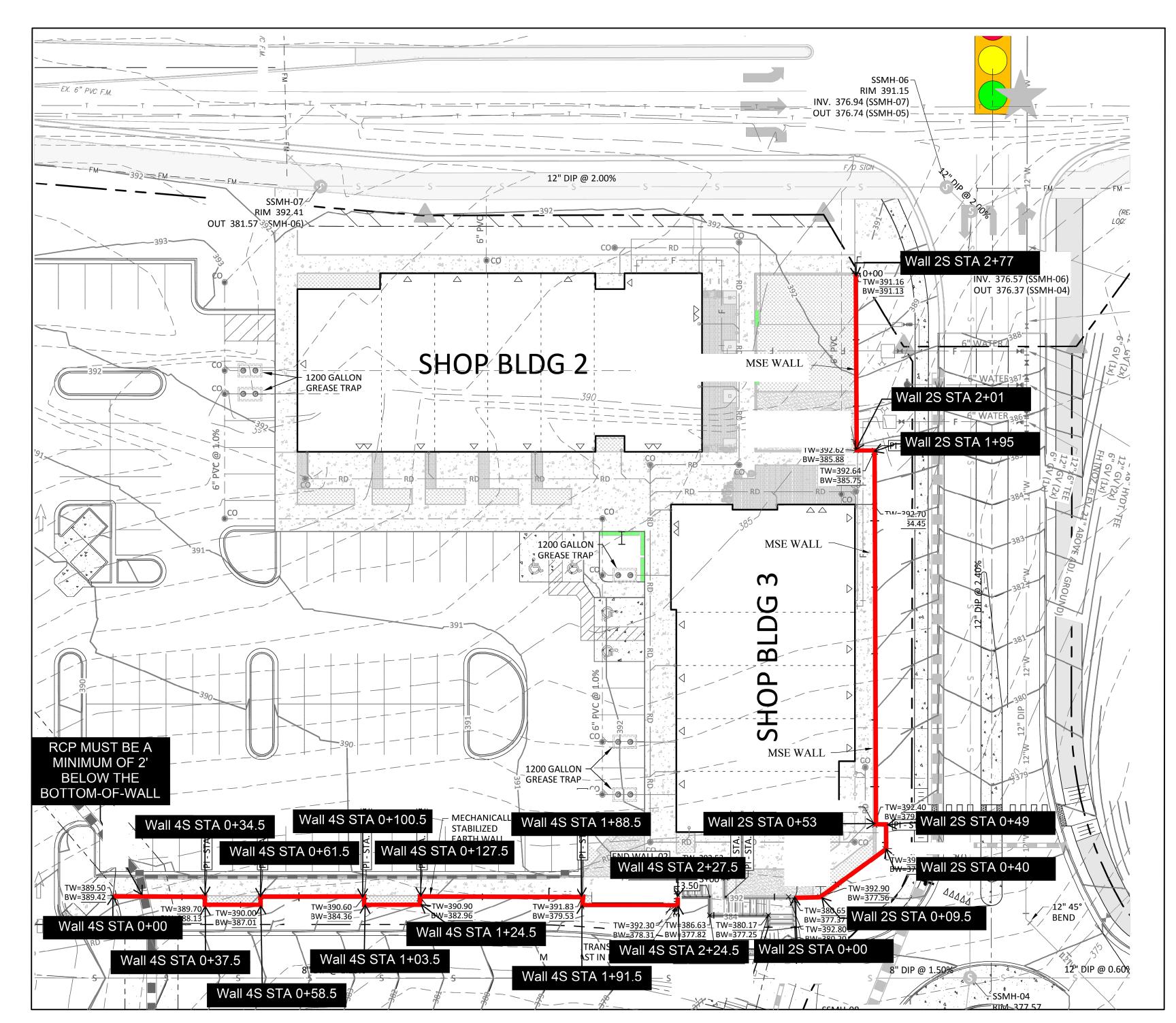
<i>RW-1</i>	Title Sheet
<i>RW-2</i>	Retaining Wall Layout
<i>RW-3</i>	Wall 2S Profile Part 1
<i>RW-4</i>	Wall 2S Profile Part 2
<i>RW-5</i>	Wall 4S Profile
<i>RW-6</i>	Anchor Diamond Pro PS Unit Details
<i>RW-7</i>	Additional Details
<i>RW-8</i>	Specifications

NV5 Engineers and Consultants, Inc.

4905 Professional Court Raleigh, North Carolina 27609 Phone: (919) 876-9799 Fax: (919) 876-8291 North Carolina Corporate License No. F-1333



}	1 1	DATE 6/14/23	Wall 2S, Site Layout, Details	JGD	Title Sheet	
					Designed by: Justin R. Pescosolido, P.E. Date: 1-17-23	
-					Drawn By: Jaxon Dean Date: 1-17-23	$\mathbb{Z}[RW-1]$
ŀ					Reviewed By: Justin R. Pescosolido, P.E. Date: 1-17-23	
					Retaining Wall Design	



WE NOTE THAT THIS SITE LAYOUT IS APPROXIMATE. STAKING OF THE ACTUAL WALL LOCATIONS IN THE FIELD IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR AND SHOULD BE PERFORMED BASED ON THE APPROVED SITE/CIVIL DRAWINGS. OUR REPRESENTATIVE SHOULD BE ALLOWED TO REVIEW STAKING PRIOR TO WALL CONSTRUCTION.

General Notes

The retaining wall(s) contained herein have been designed using active earth pressure theory. Therefore, outward movement at the top of the wall(s) should be expected. Outward movement of the retaining wall(s) may be limited by using high quality fill soils with a low fines content within the reinforced zone of the wall(s). Some cracks could develop at the ground surface due to lateral movement of the wall(s). These cracks should be filled in as soon as they are observed to help protect the soils below the ground surface from softening related to water infiltration that could affect the support characteristics for adjacent construction.

Preliminary analyses for global stability and total and differential settlement were performed as part of the design services for the segmental retaining wall(s). Our analyses were based, in part, on assumed in-situ soil properties derived from our previous Report of Subsurface Investigation and Geotechnical Engineering Evaluation and our previous experience with similar conditions in close geographic proximity to this site. If soil conditions encountered during construction are significantly different than those assumed herein, NV5 Engineers and Consultants, Inc. shall be contacted immediately for review of and possible alterations to this design.

The Engineer requests that representatives of the owner and/or general contractor arrange a pre-construction meeting with all pertinent parties involved for the construction of the retaining wall(s) shown on these plans. The Engineer's responsibility is limited to providing only the design services of the project's retaining wall(s) contained herein. Retaining wall construction monitoring and retaining wall certifying are beyond the scope of these design services. The Engineer shall not be required to sign any document, no matter by whom requested, in which the Engineer is required to certify, guarantee, or warrant conditions of which the Engineer has not or cannot ascertain.

Retaining Wall 4S was designed considering an additional 250 pounds per square foot (psf) live load at a setback of approximately 10 feet from top-of-wall to account for vehicle traffic in the planned parking lot and drive aisles. Retaining Wall 2S was designed considering foundation surcharge loads as provided to us by the structural engineer of record. No other dead load surcharge conditions were considered in the wall design. Structures such as light poles, handrail, guardrail, or drainage structures to be installed in the vicinity of the retaining wall(s) shall be designed and constructed to resist imposing additional lateral loads on the retaining wall(s). If future construction alters the assumed loading conditions of the retaining wall(s), NV5 Engineers and Consultants, Inc. shall be notified to review the design criteria for the imposed loads.

Construction Notes

REV DATE

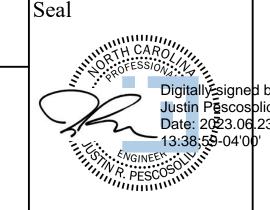
- 1. Prior to construction, confirmation of the distances to property lines, Tree Buffers, roadways, sidewalks, and/or curb and gutter to the face(s) of the proposed wall(s) shall be performed.
- 2. Prior to construction, confirmation of existing utility line locations (Stormwater, Sewer, Water, Electrical, and Gas) and the locations of future utility lines shall be performed.
- 3. Prior to construction, confirmation of the in-situ and proposed grades shall be performed by a qualified surveyor. NV5 Engineers and Consultants, Inc. shall be notified if the site grades are different than those shown on these drawings.
- 4. During construction, care must be exercised to prevent the undermining of any existing structures.

DESCRIPTION

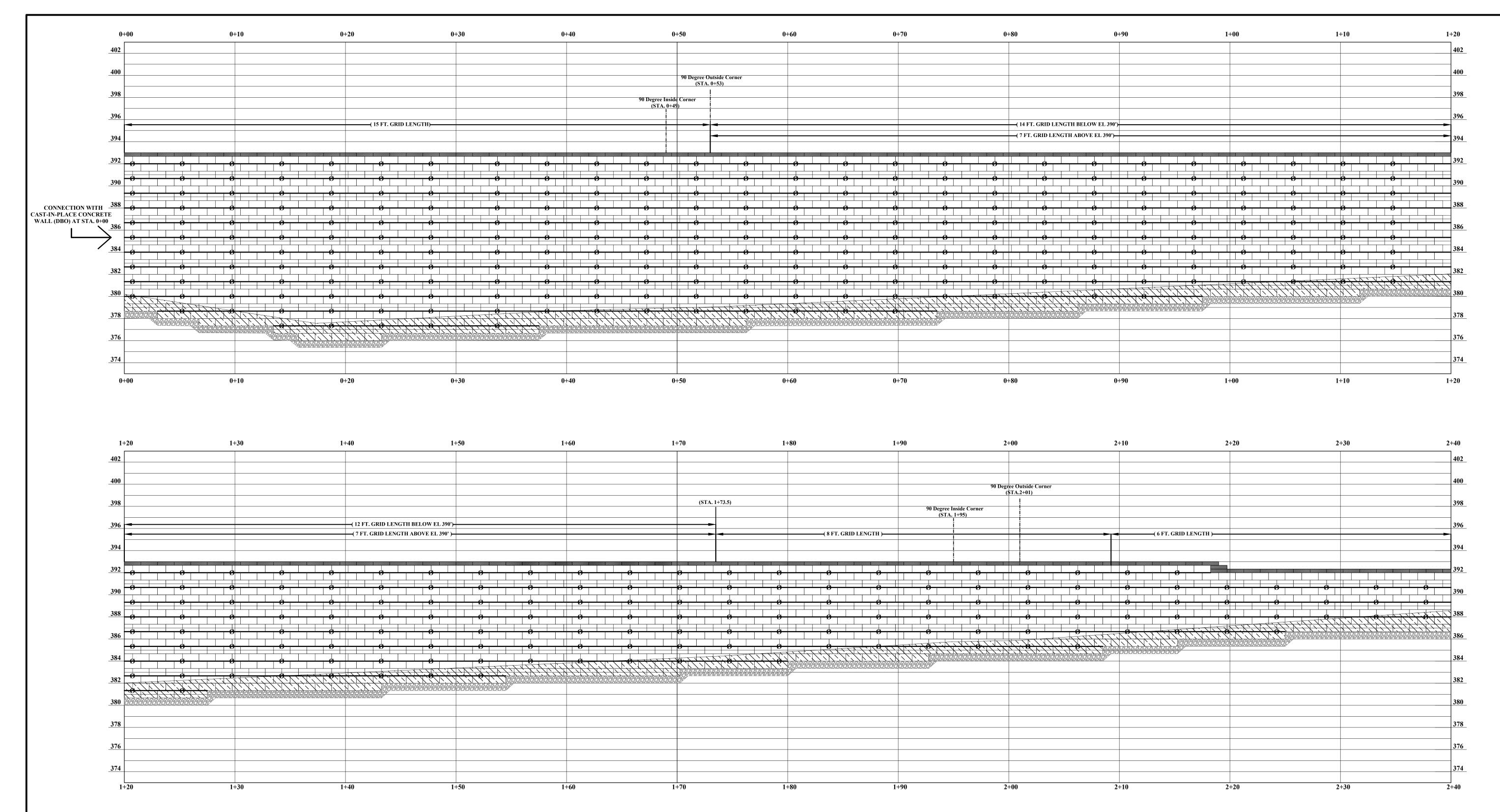
- 5. Utility structures and underground lines located within the reinforced zone of the retaining wall(s) shall be installed prior to or during construction of the retaining wall(s) to prevent damage to the reinforcement layers. If the presence of utility structures interferes with the integrity of the reinforcement, NV5 Engineers and Consultants, Inc. shall be notified during construction to recommend suitable remedial measures that will ensure proper operation of the retaining wall(s).
- 6. After construction, heavy equipment should not operate within 3 feet of the top portion of the wall(s) to prevent adverse impacts to the structural integrity of the retaining wall(s).
- 7. After construction, care must be exercised to prevent damage to the upper layers of reinforcement and degrading of the retained soils of the retaining wall(s). Installation of light poles, signs, handrails, guardrails, shrubs, or trees (etc.) in the reinforced zone of this retaining wall(s) shall not damage the upper layers of reinforcement. Any damaged reinforcement shall be repaired.
- 8. Surface water drainage shall be designed by others to discharge surface water away from the wall face(s) and away from the foundations of adjacent construction at all times during and after construction of the retaining wall(s). All downspouts from the nearby structures should be directed away from the wall(s) and slope(s) above the wall(s).

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F	1	6/14/23	Wall 2S, Site Layout, Details	JGD	Retaining Wal	l Layou	ıt
F					Designed by: Justin R. Pescosolido, P.E.	Date: 1-17-23	SHEET
F						Date: 1-17-23	RW-2
					Reviewed By: Justin R. Pescosolido, P.E. Retaining Wall	Date: 1-17-23 Design	



NOTE: CONNECTION TO CAST-IN-PLACE CONCRETE
WALL (CONCRETE WALL DESIGNED BY OTHERS)
NOTE: TOP 2 GRID LAYERS (ABOVE EL 390') 7 FEET
REMAINING GRID LAYERS (BELOW EL 390') 14 FEET
(FROM STA 0+53 TO STA 1+20)
NOTE: TOP 2 GRID LAYERS (ABOVE EL 390') 7 FEET
REMAINING GRID LAYERS (BELOW EL 390') 12 FEET

(FROM STA 1+20 TO STA 1+73.5)

RETAINING WALL 2S PROFILE Part 1

<u>LEGEND:</u>
1). MIRAFI 3XT GEOGRID DESIGNATION

2). EMBEDDED BLOCK DESIGNATION

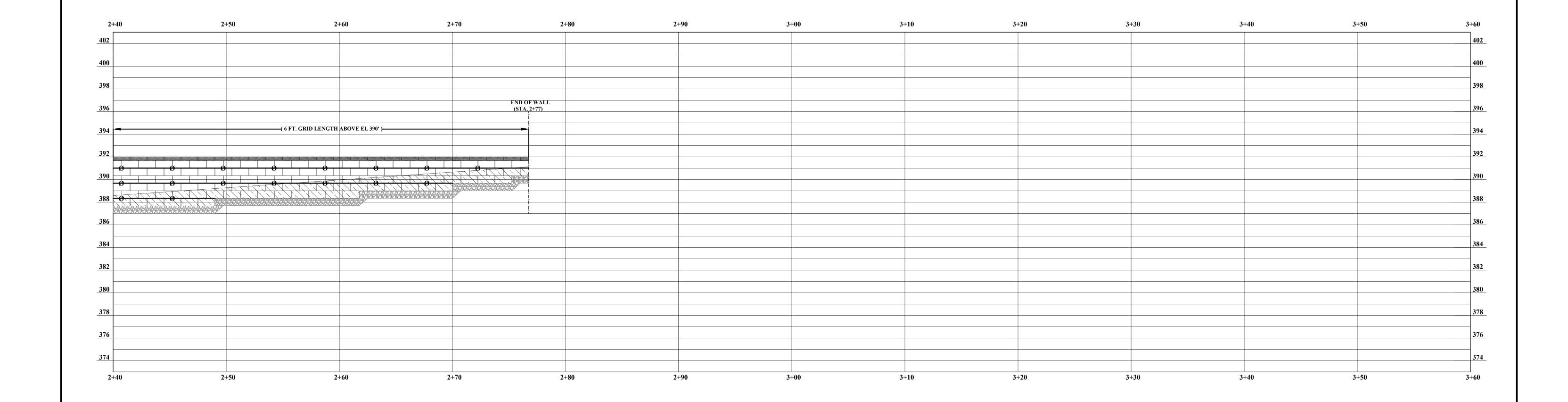
3). LEVELING PAD DESIGNATIO

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1		6/14/23	Wall 2S, Site Layout, Details	JGD	D -4	D C1.	D4 1
-	L	0/14/23	Wan 25, She Layout, Details	JOD	Retaining Wall 2S	Prome	Part I
					1		
							SHEET
					Designed by: Justin R. Pescosolido, P.E.	Date: 1-17-23	
						D + 1.17.00	RW-3
_					Drawn By: Jaxon Dean	Date: 1-17-23	IN VV -3
					Reviewed By: Justin R. Pescosolido, P.E.	Date: 1-17-23	
					Retaining Wal	11 Decign	
					Retaining was	n Design	

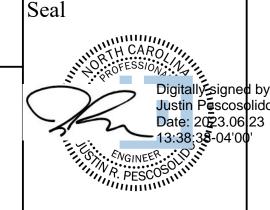


LEGEND:1). MIRAFI 3XT GEOGRID DESIGNATION

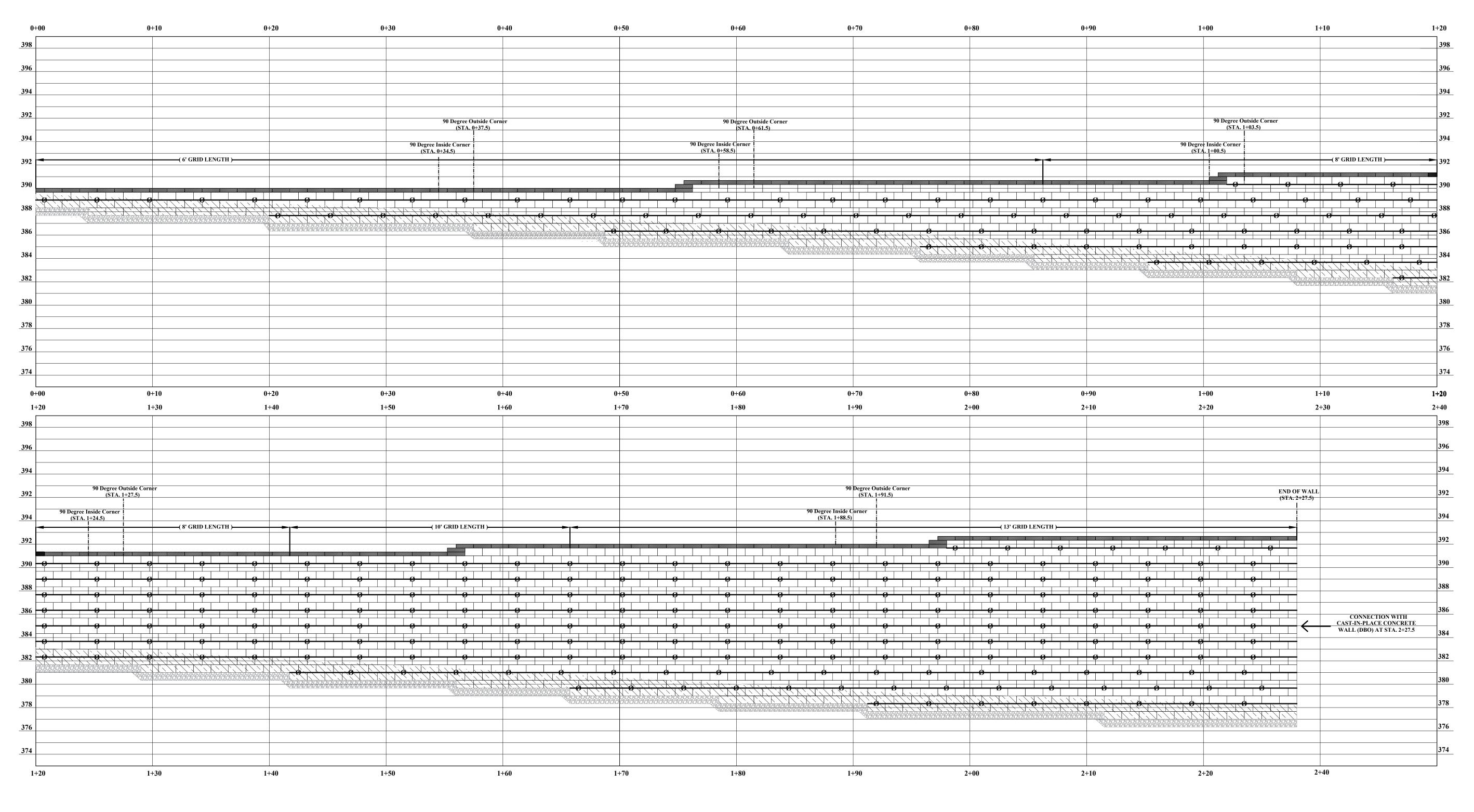
RETAINING WALL 2S PROFILE Part 2

NV5 Engineers and Consultants, Inc.

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KE V	DAIL	DESCRIPTION	DY			
1	6/14/23	Wall 2S, Site Layout, Details	JGD	Retaining Wall 2S	Profile	Part 7
				1 Actaining Wan 25	1 1 01110	
				1		
						SHEET
				Designed by: Justin R. Pescosolido, P.E.	Date: 1-17-23	
						DIII 1
				Drawn By: Jaxon Dean	Date: 1-17-23	RW-4
				Reviewed By: Justin R. Pescosolido, P.E.	Date: 1-17-23	
				Retaining Wal	11 Decign	
				1 Retaining was	n Design	



NOTE: CONNECTION TO CAST-IN-PLACE CONCRETE WALL (CONCRETE WALL DESIGNED BY OTHERS)

RETAINING WALL #4S

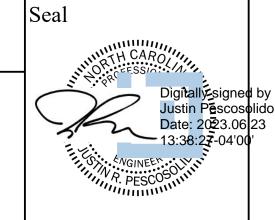
<u>LEGEND:</u>
1). MIRAFI 3XT GEOGRID DESIGNATION

2). EMBEDDED BLOCK DESIGNATION

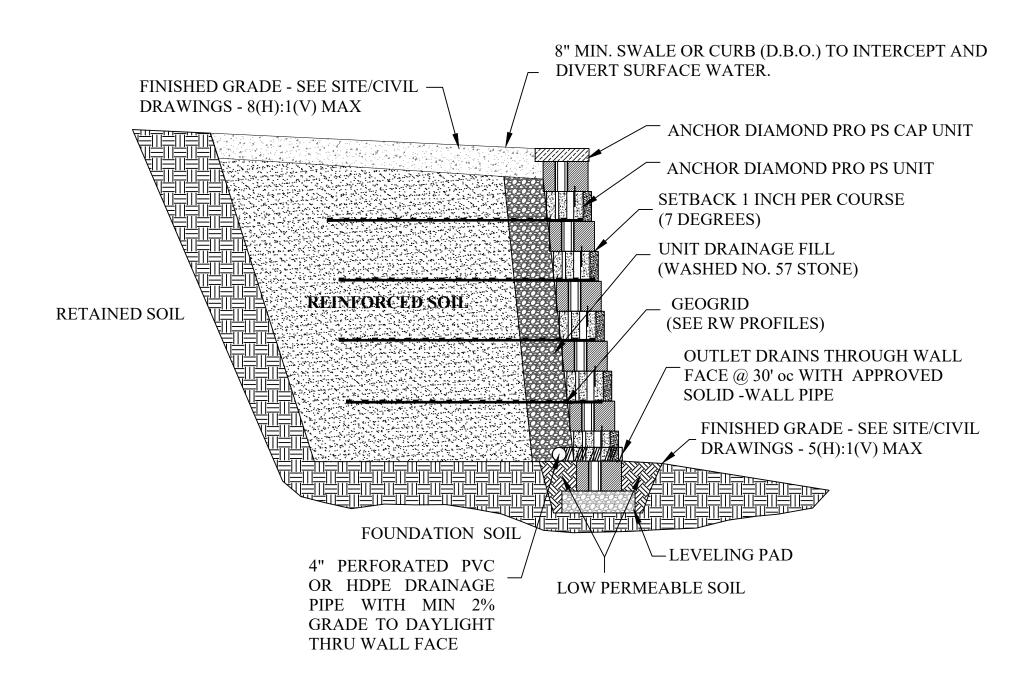
3). LEVELING PAD DESIGNATION

NV5 Engineers and Consultants, Inc.

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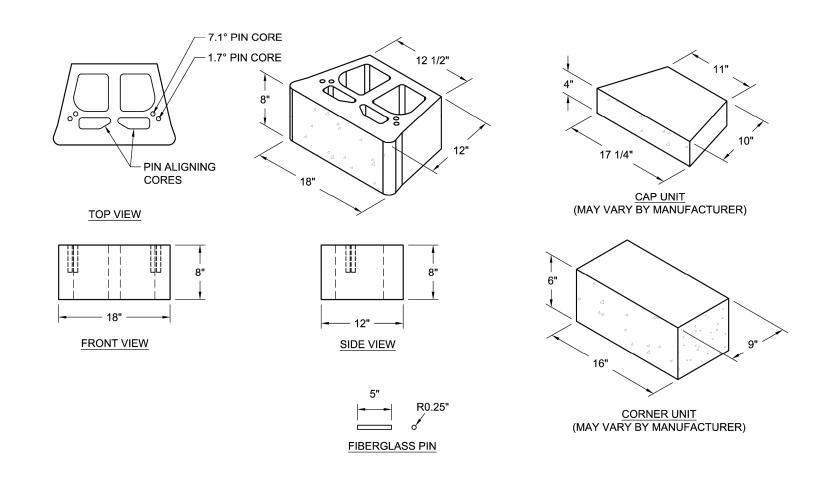


KE V	DATE	DESCRIPTION	BY		100	O4 T
1	6/14/23	Wall 2S, Site Layout, Details	JGD	Retaining Wall	I 48 Proi	file
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				Designed by: Justin R. Pescosolido, P.E.	Date: 1-17-23	
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				Drawn By: Jaxon Dean	Date: 1-17-23	RW-5
				Reviewed By: Justin R. Pescosolido, P.E.	Date: 1-17-23	
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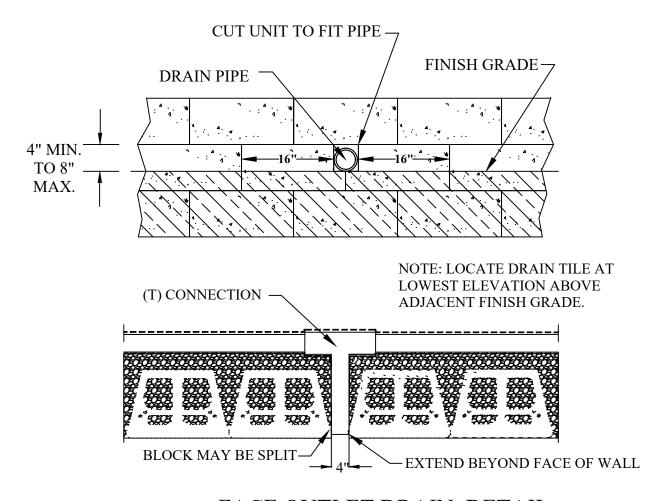
ANCHOR DIAMOND PRO PS RETAINING WALL DETAIL

SCALE: NONE



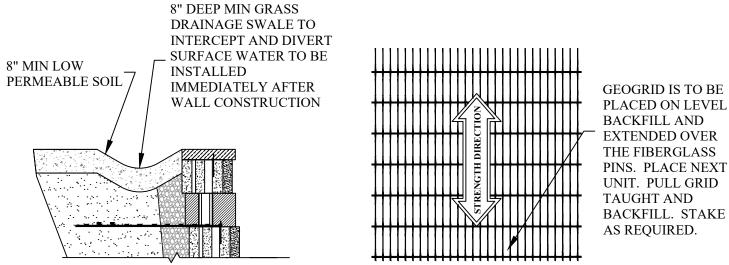
DIAMOND PRO UNIT DETAILS

SCALE: NONE

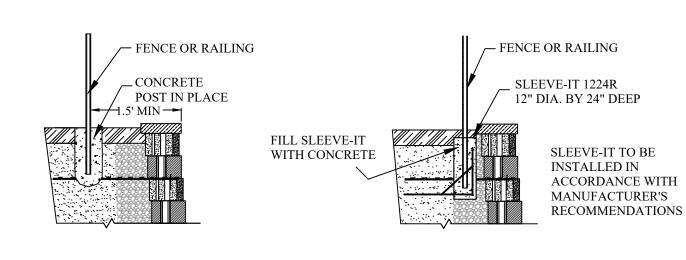


FACE OUTLET DRAIN DETAIL

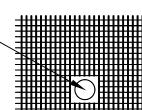
SCALE: NONE



DRAINAGE SWALE DETAILGRID AND PIN CONNECTIONSCALE: NONESCALE: NONE



CUT GRID AND PLACE
TUBE OR FORMS
PRIOR TO PLACING
ADDITIONAL FILL

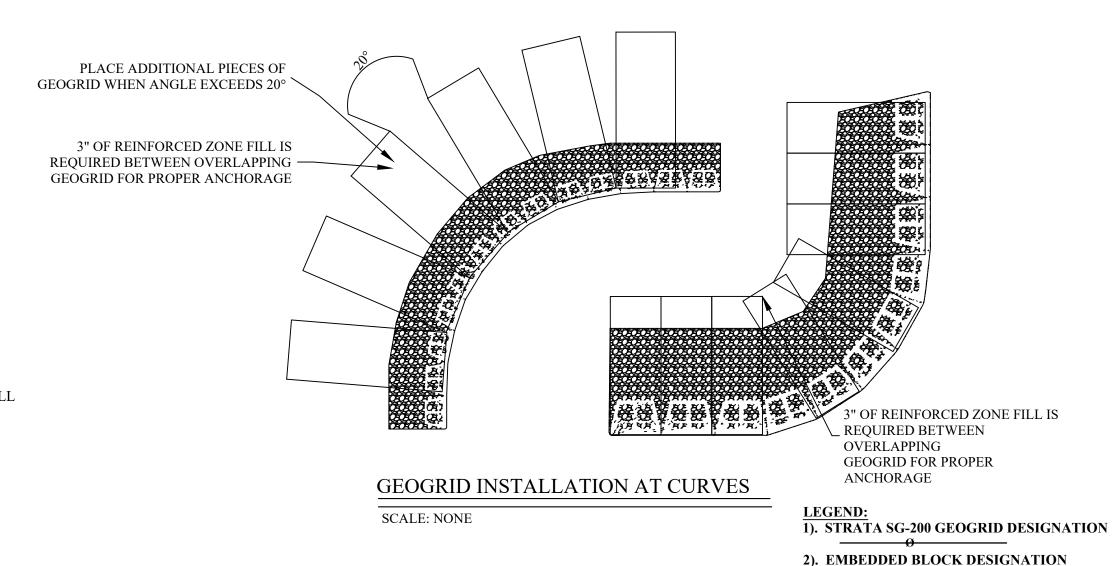


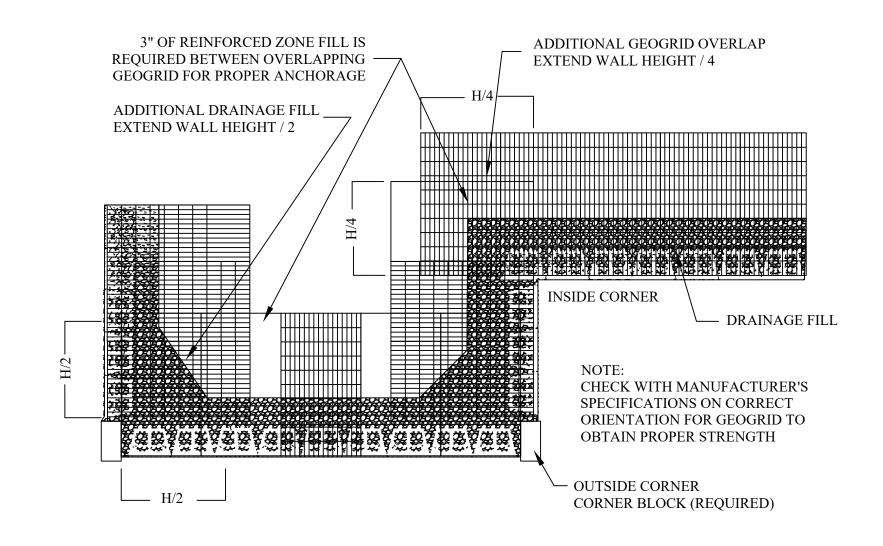
CUT SUCCESSIVE LAYERS OF GEOGRID AROUND PROPOSED FENCE POST INSTALLATION POINTS AND SET CONCRETE TUBE OR FORMS DURING WALL CONSTRUCTION. CHECK FENCE DESIGN FOR EMBEDMENT DEPTH OF FENCE POST

RAILING REQUIREMENTS FOR THE RETAINING WALL SHALL BE DETERMINED BY THE SITE DESIGNER. THE RAILING SHOULD BE DESIGNED IN ACCORDANCE WITH THE NORTH CAROLINA BUILDING CODE BY A REGISTERED DESIGN PROFESSIONAL. THE RAILING SHOULD BE DESIGNED SUCH THAT IT DOES NOT ADD ANY ADDITIONAL LATERAL FORCES TO THE RETAINING WALL. THE CONCRETE TUBES OR SLEEVE-IT FORMS FOR THE RAILINGS SHALL BE INSTALLED BY THE SITE CONTRACTOR AND COORDINATED WITH THE RETAINING WALL CONTRACTOR DURING CONSTRUCTION OF THE RETAINING WALL.

TYPICAL HAND RAILING DETAIL

SCALE: NONE





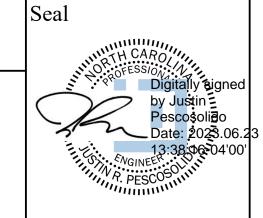
3). LEVELING PAD DESIGNATION

GEOGRID INSTALLATION AT CORNERS

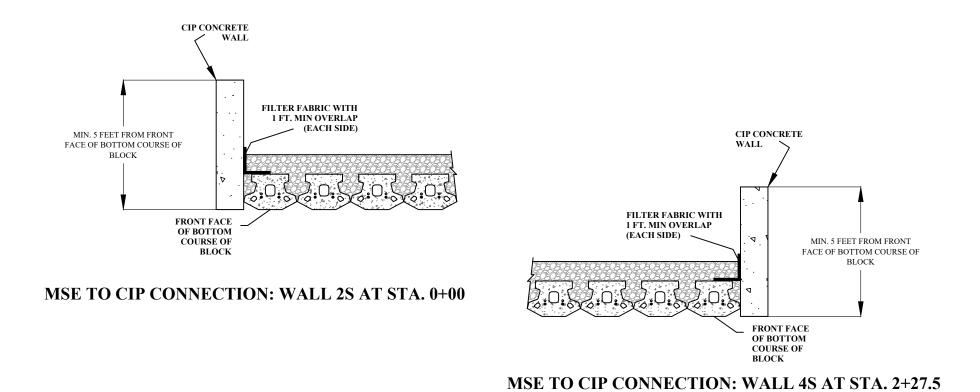
SCALE: NONE

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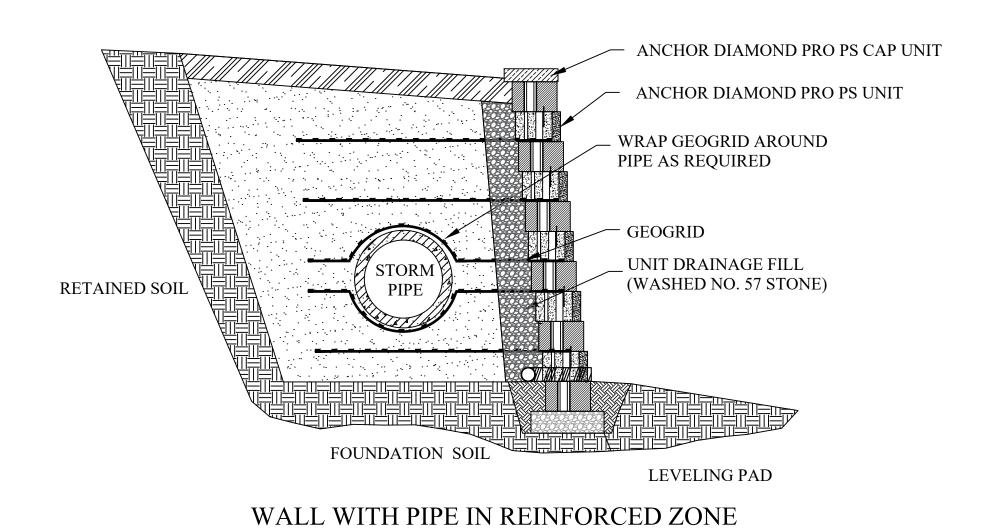


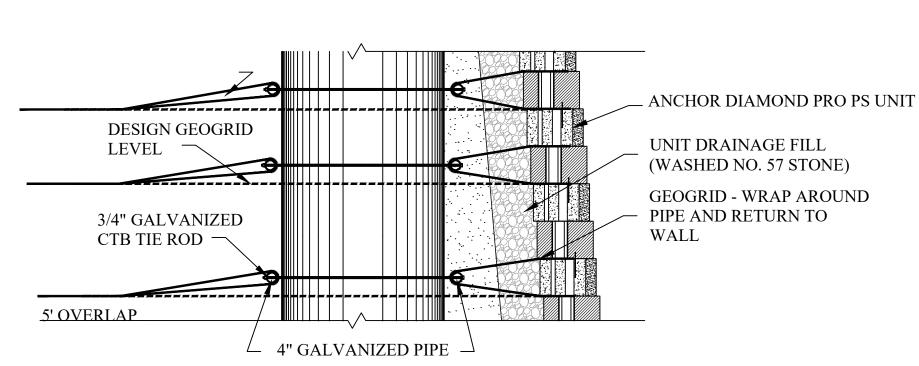
REV 1	DATE 6/14/23	DESCRIPTION Wall 2S, Site Layout, Details	JGD	Anchor Diamond Pro PS Unit Details
				Designed by: Justin R. Pescosolido, P.E. Date: 1-17-23 RW-6
				Reviewed By: Justin R. Pescosolido, P.E. Date: 1-17-23 Retaining Wall Design

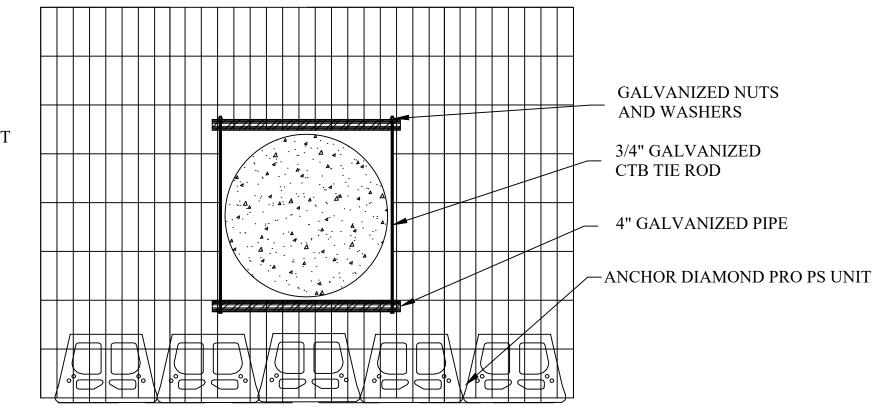


MSE WALL & CIP WALL CONNECTION DETAIL

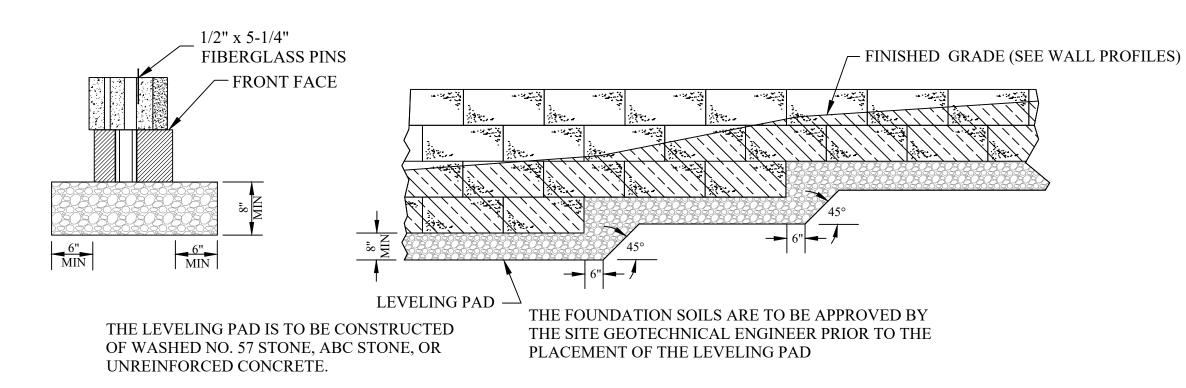
SCALE: NONE







WALL AT PIER / MANHOLE

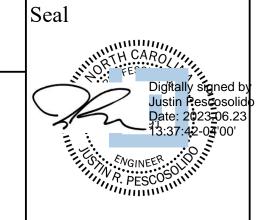


LEVELING PAD DETAILS

SCALE: NONE

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REV 1	DATE 6/14/23	Wall 2S, Site Layout, Details	JGD	Additional Details	
				Designed by: Justin R. Pescosolido, P.E. Date: 1-17-23 Drawn By: Jaxon Dean Date: 1-17-23	SHEET RW-6
				Reviewed By: Justin R. Pescosolido, P.E. Date: 1-17-23 Retaining Wall Design	

1.0 GENERAL

SEGMENTAL RETAINING WALL SYSTEMS ARE DESIGNED AS GRAVITY RETAINING WALLS UTILIZING A HIGH DENSITY POLYESTER GEOGRID TO REINFORCE THE SOIL ZONE BEHIND THE WALL. THE GEOGRID IS POSITIVELY CONNECTED TO THE MODULAR CONCRETE BLOCK CREATING A REINFORCED SOIL MASS CAPABLE OF RESISTING CERTAIN LATERAL EARTH PRESSURES AND SURCHARGED LOADS. ALL REFERENCES TO THE ENGINEER REFER TO NV5 ENGINEERS AND CONSULTANTS, INC.

1.1 QUALITY ASSURANCE

WORK SHALL BE PERFORMED ONLY BY AN EXPERIENCED CONTRACTOR. CONTRACTOR SHALL SUBMIT TO THE CERTIFYING ENGINEER EVIDENCE OF QUALIFICATIONS AND REFERENCES ON PROJECTS OF SIMILAR SCOPE. THE CERTIFYING ENGINEER RESERVES THE RIGHT TO REJECT ANY AND ALL QUALIFICATIONS SUBMITTALS. THE OWNER AND/OR GENERAL CONTRACTOR SHOULD PROVIDE AN INSPECTOR AS A FULL-TIME CONTINUOUS MONITOR OF WORK QUALITY.

1.2 BACK FILL MATERIALS

THE SOIL MATERIAL ASSOCIATED WITH THE RETAINING WALL(S) IN THE REINFORCED ZONE, THE RETAINED ZONE, OR THE FOUNDATION BEDDING SHALL HAVE, AT A MINIMUM, THE FOLLOWING PROPERTIES:

- A.) FOUNDATION SOILS $\Phi = 26$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 115 LBS/CU.FT
- B.) RETAINED SOILS $\Phi = 26$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 115 LBS/CU.FT B.) REINFORCED SOILS $\Phi = 30$ DEGREES, COHESION = 0 PSF, WET UNIT WEIGHT = 125 LBS/CU.FT
- SEE PROFILE SHEETS FOR BOTTOM OF WALL ELEVATIONS. IF THE ASSUMED VALUES DO NOT REPRESENT THE ACTUAL SOIL CONDITIONS, THE ENGINEER SHALL BE NOTIFIED AND THE WALL(S) SHALL BE REDESIGNED.

1.3 FOUNDATION LOADS

THE MAXIMUM APPLIED FOUNDATION LOAD FOR THE WALL(S) IS 2.5 KIPS/SQ.FT.

1.4 WALL BATTER

BATTER FOR THE ENTIRE WALL SHALL BE MAINTAINED AT 1-INCH PER COURSE.

2.1 CONCRETE MASONRY WALL UNITS

CONCRETE WALL UNITS SHALL BE SEGMENTAL UNITS MANUFACTURED IN ACCORDANCE WITH ASTM C-1372 AND ASTM C-140, AND SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI.

2.2 RETAINING WALL SHEAR CONNECTIONS

SEGMENTAL WALL UNITS SHALL BE INTERLOCKED UTILIZING MANUFACTURER SPECIFIED FIBERGLASS SHEAR PINS AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

2.3 GEOGRID REINFORCEMENT

GEOSYNTHETIC REINFORCEMENT SHALL CONSIST OF HIGH TENACITY GEOGRIDS MANUFACTURED FOR SOIL REINFORCEMENT APPLICATIONS. THE TYPE, LENGTH, AND PLACEMENT OF THE REINFORCING GEOSYNTHETIC SHALL BE AS SHOWN ON THE PLANS.

2.4 LEVELING PAD

MATERIAL SHALL CONSIST OF COMPACTED AGGREGATE BASE COURSE (ABC) STONE, WASHED NO. 57 STONE, OR UNREINFORCED CONCRETE AND BE A MINIMUM OF 8" THICK (UNO). ABC STONE SHALL BE COMPACTED TO 95% OF THE STANDARD PROCTOR (ASTM D-698) MAXIMUM DRY DENSITY. AGGREGATE MATERIAL SHALL RECEIVE A MINIMUM OF ONE PASS OF THE COMPACTION EQUIPMENT. THE TOP OF THE LEVELING PAD FOR THE WALL SECTIONS SHALL BE MAINTAINED AS INDICATED ON THE RETAINING WALL PROFILE.

2.5 UNIT FILL

THE VOID WITHIN EACH UNIT SHALL BE FILLED WITH WASHED NO. 57 STONE HAVING 100% OF THE AGGREGATE PASSING THE 2" SIEVE WITH A MINIMUM OF 3/8" WASHED STONE SIZE AND NO MORE THAN 5% PASSING THE NO. 200 SIEVE. THE UNIT FILL SHALL EXTEND AT LEAST 12" BEHIND THE UNITS. GRAVITY FILL ZONE SHALL CONSIST OF WASHED #57 STONE.

2.6 SOIL PROPERTIES

REINFORCED ZONE SELECT FILL MATERIALS SHALL CLASSIFY AS GM, GP, SP, SP-SM, SM OR SC WITH NO MORE THAN 20% FINES. THE MINIMUM INTERNAL ANGLE OF FRICTION, COHESION, AND WET UNIT WEIGHT SHALL BE EQUAL TO OR GREATER THAN THE DESIGN VALUES PROVIDED IN SECTION 1.2.

2.7 UNSUITABLE MATERIAL

SOILS CONTAINING ROOTS, BRUSH, SOD, OR OTHER ORGANIC MATERIAL SHALL NOT BE PERMITTED AS FILL. FROZEN SOILS, SNOW, ICE, HEAVY CLAYS, OR WET SOILS SHALL NOT BE PERMITTED AS FILL. MATERIAL PASSING THE NO. 40 SIEVE SHALL NOT HAVE A LIQUID LIMIT OF GREATER THAN 40 AND A PLASTICITY INDEX OF GREATER THAN 15, UNLESS WRITTEN CONSENT IS OBTAINED FROM THE DESIGN ENGINEER PRIOR TO PLACEMENT.

3.1 FOUNDATION REQUIREMENTS

THE FOUNDATION BEARING CAPACITY THAT WAS ASSUMED FOR THE DESIGN SHALL BE VERIFIED IN THE FIELD WITH A MINIMUM FACTOR OF SAFETY OF 2.0, AND COPIES OF THE TESTS PROVIDED TO THE CERTIFYING ENGINEER. THE FOUNDATION ZONE SHALL BE CLEARED OF ALL DEBRIS AND LOOSE SOIL. FOUNDATION SOILS NOT MEETING THE MINIMUM DESIGN CRITERIA, SHALL BE REMOVED AND REPLACED UTILIZING AGGREGATE BASE COURSE (ABC) STONE, PLACED IN LIFTS NOT EXCEEDING 8", AND COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR (ASTM D-698) MAXIMUM DRY DENSITY.

3.2 FIRST BLOCK COURSE

THE FIRST COURSE OF BLOCK SHALL BE PLACED ON TOP OF AND IN FULL CONTACT WITH THE LEVELING PAD. THE UNITS SHALL MAINTAIN A MINIMUM DISTANCE OF 6" FROM THE FRONT AND BACK OF THE LEVELING PAD. PROPER ALIGNMENT MAY BE ACHIEVED WITH THE AID OF A STRING LINE. PROCEED TO THE NEXT COURSE OF BLOCK. EACH UNIT SHALL BE IN CONTACT WITH THE UNITS ON BOTH SIDES AS WELL AS ABOVE AND BELOW. SOME ADJUSTMENTS MAY BE REQUIRED FOR WALLS WITH CURVES AND A BATTER.

3.3 GEOGRID INSTALLATION

THE GEOGRID REINFORCEMENT SHALL BE LAID HORIZONTALLY ON COMPACTED REINFORCED ZONE FILL AND POSTITIVELY CONNECTED TO THE CONCRETE WALL UNITS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. GEOGRID SHALL BE PULLED TAUT, REMOVING ALL SLACK, AND ANCHORED BEFORE ADDING FILL. GEOGRID SHALL BE INSTALLED AT THE ELEVATIONS AND LENGTHS INDICATED ON THE RETAINING WALL PROFILE. THE SOIL SURFACE SHALL BE SMOOTH AND LEVEL AND HAVE BEEN COMPACTED BEFORE INSTALLING THE GEOGRID.

3.4 REINFORCED ZONE FILL PLACEMENT

REINFORCED ZONE SELECT FILL OR ABC STONE SHALL BE PLACED IN A MAXIMUM 6" LIFT THAT IS COMPACTED TO 95% OF THE STANDARD OR NCDOT MODIFIED PROCTOR MAXIMUM DRY DENSITY AT A MOISTURE CONTENT WITHIN 3% OF THE OPTIMUM MOISTURE CONTENT. ONLY HAND OPERATED EQUIPMENT SHALL BE ALLOWED WITHIN 3 FEET OF THE SEGMENTAL UNITS. FILL SHALL BE PLACED FROM THE WALL REARWARD TO INSURE TAUTNESS OF THE GEOGRID. CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID.

3.5 RETAINING WALL CAP UNITS

RETAINING WALL CAPS TO BE SECURED TO TOP COURSE OF BLOCK WITH APPROPRIATE ADHESIVE.

4.0 SOIL TESTING

WHERE REINFORCED PROCESSED FILL OR ABC STONE MATERIALS ARE PLACED, COMPACTION TESTING SHALL BE PERFORMED FOR EVERY LIFT ELEVATION REQUIRING GEOGRID OR EVERY THIRD LIFT AS A MINIMUM. TEST RESULTS SHALL BE PROVIDED TO THE CERTIFYING ENGINEER. VERIFICATION OF SUFFICIENT COMPACTIVE EFFORT SHOULD BE PERFORMED FOR CLEAN AGGREGATE PLACEMENT.

5.0 HYDROSTATIC PRESSURE POTENTIAL

ANY SURFACE RUNOFF OR STORMWATER SHALL BE DISCHARGED AWAY FROM THE WALL FOUNDATION AND ANY ADJACENT FOUNDATION SYSTEMS. FOUNDATION DRAINS FOR ADJACENT STRUCTURES SHOULD BE ADEQUATELY PIPED AWAY FROM THE RETAINING WALL. ROOF DRAINAGE SYSTEMS SHOULD BE DESIGNED TO PREVENT ROOF RUNOFF FROM BEING DISCHARGED BEHIND THE RETAINING WALL. SURFACE WATER COLLECTION AND PIPING SYSTEMS SHALL BE DESIGNED BY OTHERS FOR YARD AREAS BEHIND THE RETAINING WALL. DRAINAGE SYSTEMS SHALL BE DESIGNED AND MAINTAINED TO AVOID EROSION OF THE SOILS AT THE AREA BELOW THE WALL.

6.0 ACCEPTABLE BLOCK

SEGMENTAL BLOCK UNITS SHALL BE USED AND KEPT FREE OF DEFECTS THAT WOULD INTERFERE WITH THE PLACING OR POSITIONING OF THE UNIT OR IMPAIR ITS STRENGTH. THE CONTRACTOR SHALL PREVENT EXCESS MUD, WET CEMENT, EPOXY, AND THE LIKE MATERIALS FROM COMING IN CONTACT WITH AND AFFIXING TO THE UNITS. MINOR CRACKS INCIDENTAL TO THE USUAL METHOD OF MANUFACTURING OR MINOR CHIPPING RESULTING FROM SHIPMENT AND DELIVERY ARE NOT GROUNDS FOR REJECTION.

7.0 ACCEPTABLE GEOGRID

GEOGRID SHALL BE REJECTED IF 20% OR MORE OF A STRUCTURAL RIB HAS BEEN CUT OR RIPPED. THE CONTRACTOR SHALL INSPECT ALL GEOGRID DELIVERED TO THE SITE AND REJECT MATERIALS THAT MEET THIS CRITERIA. THE CONTRACTOR SHALL PREVENT EXCESS MUD, WET CEMENT, EPOXY, AND THE LIKE MATERIALS FROM COMING IN CONTACT WITH AND AFFIXING TO THE GEOGRID MATERIAL. IF THE GEOGRID IS DAMAGED ONSITE, IT SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.

8.0 DRAINAGE COMPOSITE

DRAINAGE COMPOSITE REQUIREMENTS TO BE DETERMINED BY PROJECT GEOTECHNICAL ENGINEER AT TIME OF CONSTRUCTION.

9.0 SPECIAL PROVISIONS

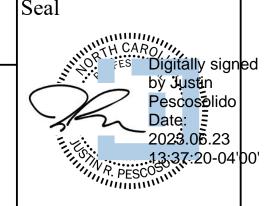
- A). MAINTAIN THE DIRECTION OF DRAINAGE AWAY FROM THE WALL FACE AT ALL TIMES DURING CONSTRUCTION OF THE RETAINING WALL(S) AND FINISHED GRADING AS SHOWN ON THE DRAINAGE PLANS TO BE PREPARED BY OTHERS.
- B). NV5 ENGINEERS SHALL BE CONTACTED FOR REVIEW OF THE ADJOINING REINFORCED CONCRETE WALL DESIGNS AS THEY BECOME AVAILABLE.
- C). THE ENGINEER SHALL BE NOTIFIED BY THE INSTALLING CONTRACTOR SHOULD THE EMBEDMENT DEPTH OF THE RETAINING WALL BE LESS THAN THAT WHICH IS SHOWN ON THE RETAINING WALL PROFILE.
- D). AS PER THE NORTH CAROLINA STATE BUILDING CODE, A BUILDING PERMIT MUST BE OBTAINED PRIOR TO WALL CONSTRUCTION. THE CONTRACTOR SHALL CONTACT THE LOCAL MUNICIPALITY CODE ENFORCEMENT DIVISION TO OBTAIN A BUILDING PERMIT.

10.0 QUALIFICATION OF DESIGN

- A). STABILITY OF ANY TEMPORARY SLOPES REQUIRED BY THE INSTALLATION OF A SEGMENTAL RETAINING WALL SHALL BE ADDRESSED BY A QUALIFIED GEOTECHNICAL ENGINEER. RESPONSIBILITY OF THESE TEMPORARY SLOPES RESTS WITH THE OWNER AND/OR THE CONTRACTOR OF THE PROJECT. ALL SLOPES SHALL MEET CURRENT OSHA STANDARDS.
- B). HANDRAIL/GUARDRAIL REQUIREMENTS SHALL BE DETERMINED BY THE ARCHITECT OR GENERAL CONTRACTOR.
- C). NOTIFY THE DESIGN ENGINEER PRIOR TO MODIFYING WALL CONSTRUCTION IF EXISTING SITE CONDITIONS DEVIATE FROM CONDITIONS OUTLINED ON THE RETAINING WALL PROFILE.

NV5 Engineers and Consultants, Inc.

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REV 1	DATE 6/14/23	Wall 2S, Site Layout, Details	JGD	Specifications	
				Designed by: Justin R. Pescosolido, P.E. Date: 1-17-23 Prawn By: Jaxon Dean Date: 1-17-23	
				Reviewed By: Justin R. Pescosolido, P.E. Date: 1-17-23 Retaining Wall Design	



3625 Brookside Parkway Suite 395

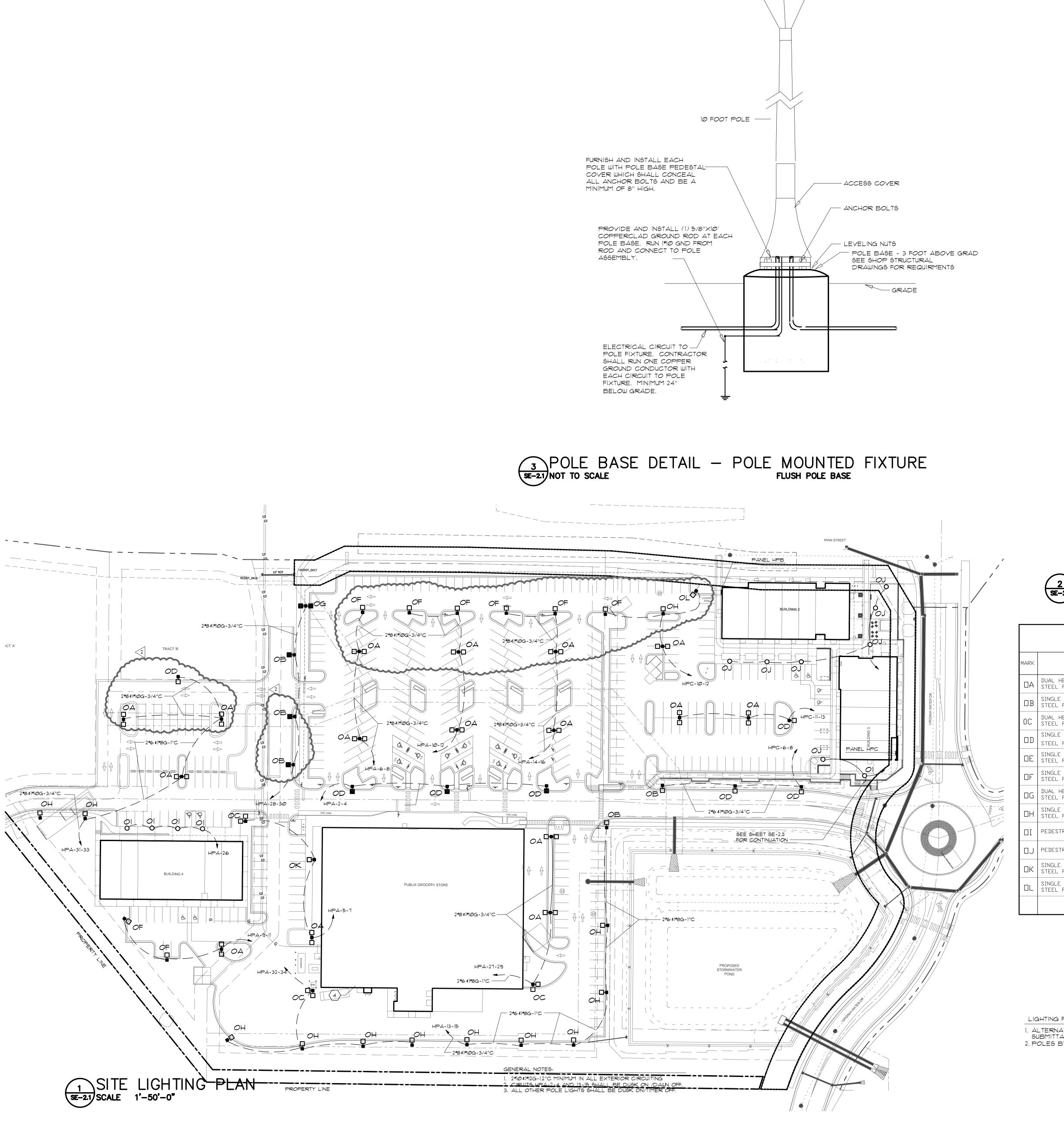
Alpharetta, Georgia 30022 Phone: 770.753.8787 Web: www.hiscutt-aia.com

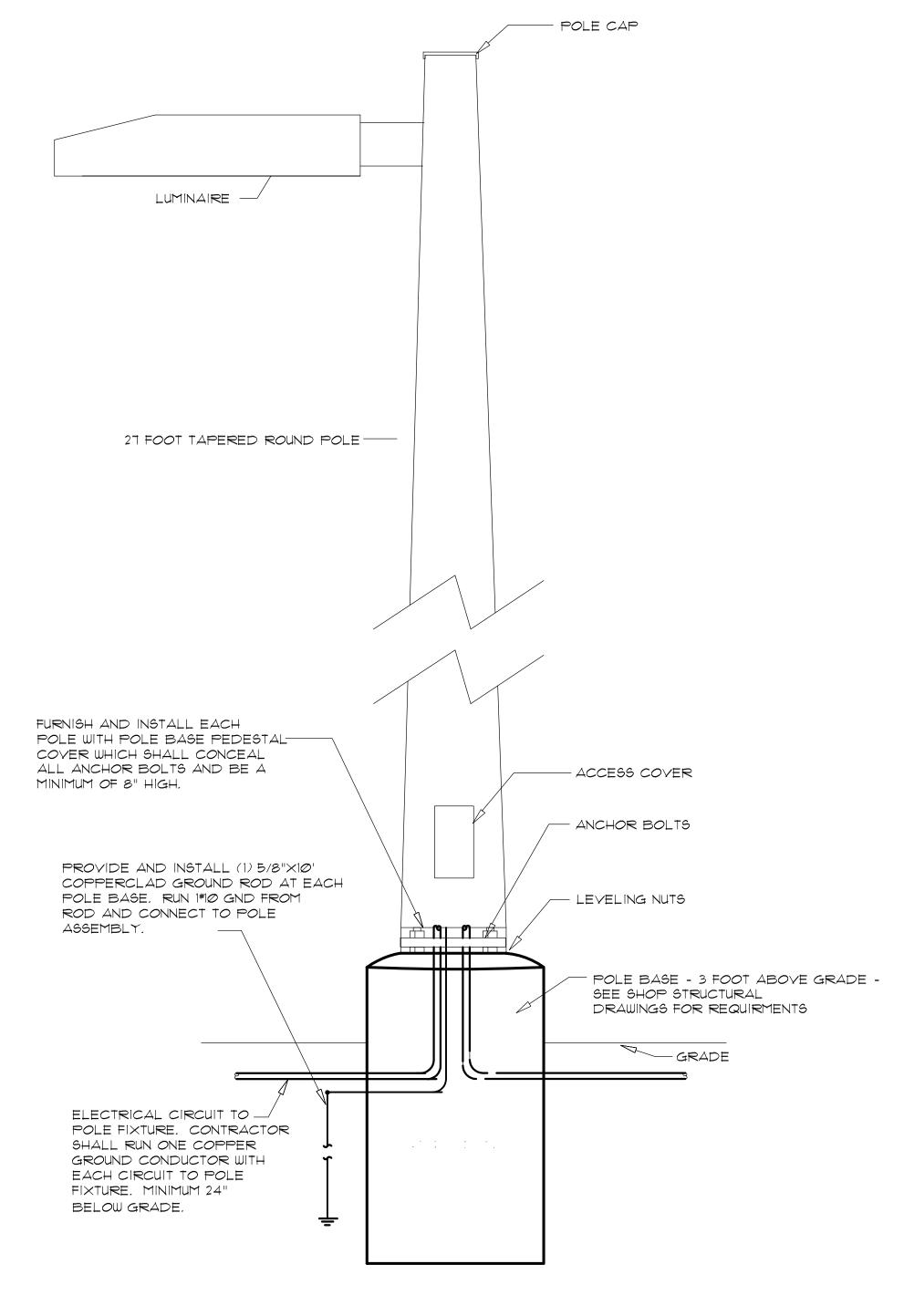


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PROJECT NO: 22922SE11

Savant Engineering, PLLC 5064 Roswell Road, Suite D-301 Sandy Springs GA 30342 770.319.7400 ©2022 Firm #P-1854 Project 22922





POLE BASE DETAIL — POLE MOUNTED FIXTURE SE-2.1 NOT TO SCALE ELEVATED POLE BASE

				LAMPS			
MARK	DESCRIPTION	MOUNTING	VOLT	TYPE	N□.	WATTS	MANUFACTURER
ПΑ	DUAL HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE, BACK TO BACK, BRONZE	DETAIL 2/SE-1.1	208	LED, 4000K	2	558	McGRAW EDISON (2)GLEON-SA5C-740-U-5WQ-B2B-BZ
□В	SINGLE HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE, BRONZE.	DETAIL 2/SE-1.1	208	LED, 4000K	1	279	McGRAW EDISON GLEON-SA5C-740-U-5WQ-BZ
0C	DUAL HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE, 2090, BRONZE	DETAIL 2/SE-1.1	208	LED, 4000K	2	558	McGRAW EDISON (2)GLEON-SA5C-740-U-5WQ-2@90-BZ
	SINGLE HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE. BRONZE.	DETAIL 2/SE-1.1	208	LED, 4000K	1	279	McGRAW EDISON GLEON-SA5C-740-U-T4W-BZ
ΠE	SINGLE HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE. BRONZE.	DETAIL 2/SE-2.1	208	LED, 4000K	1	279	McGRAW EDISON GLEON-SA5C-740-U-SL2-BZ
ΠF	SINGLE HEAD LED POLE LIGHT ON 12.0 FOOT TAPERED STEEL POLE, BRONZE.	DETAIL 2/SE-2.1	208	LED, 4000K	1	113	McGRAW EDISON GLEON-SA2C-740-U-T4FT-BZ
ΠG	DUAL HEAD LED POLE LIGHT ON 12.0 FOOT TAPERED STEEL POLE, BACK TO BACK, BRONZE	DETAIL 2/SE-2.1	208	LED, 4000K	2	226	McGRAW EDISON (2)GLEON-SA2C-740-U-5WQ-B2B-BZ
ΠН	SINGLE HEAD LED POLE LIGHT ON 12.0 FOOT TAPERED STEEL POLE, BACK TO BACK, BRONZE	DETAIL 2/SE-2.1	208	LED, 4000K	1	113	McGRAW EDISON GLEON-SA2C-740-U-T4W-BZ
	PEDESTRIAN FIXTURE ON 10 FOOT POLE, BRONZE FINISH.	DETAIL 3/SE-2.1	208	LED, 4000K	1	48	INVUE LXS-VA-2-740-U-AST-S-BZ WITH ARP-5-L-6-10-A-BZ-5 POLE
ПJ	PEDESTRIAN FIXTURE ON 10 FOOT POLE, BRONZE FINISH.	DETAIL 3/SE-2.1	208	LED, 4000K	1	41	INVUE LXS-VA-2-740-U-SYM-S-BZ WITH ARP-5-L-6-10-A-BZ-5 POLE
ΠK	SINGLE HEAD LED POLE LIGHT ON 27,0 FOOT TAPERED STEEL POLE. BRONZE.	DETAIL 2/SE-2.1	208	LED, 4000K	1	279	McGRAW EDISON GLEON-SA5C-740-U-T4FT-BZ
	SINGLE HEAD LED POLE LIGHT ON 27.0 FOOT TAPERED STEEL POLE. BRONZE.	DETAIL 2/SE-2.1	208	LED, 4000K	1	279	M⊂GRAW EDISON GLEON-SA5C-740-U-SL3-BZ

LIGHTING FIXTURE SCHEDULE NOTES:

1. ALTERNATE SUPPLIERS OF SPECIFIED EQUIPMENT WILL BE ACCEPTABLE ONLY BY FORMAL SUBMITTAL 10 DAYS PRIOR TO BID, WITH PHOTOMETRIC PLAN.
2. POLES BY KW INDUSTRIES.

Savant
Engineering, PLLC

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Sandy Springs GA 30342 770.319.7400
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ROLINA

WALLBROOK
US 401 at Virginia Water Drive
WAKE COUNTY, NORTH CAROLIN

HTING PLAN

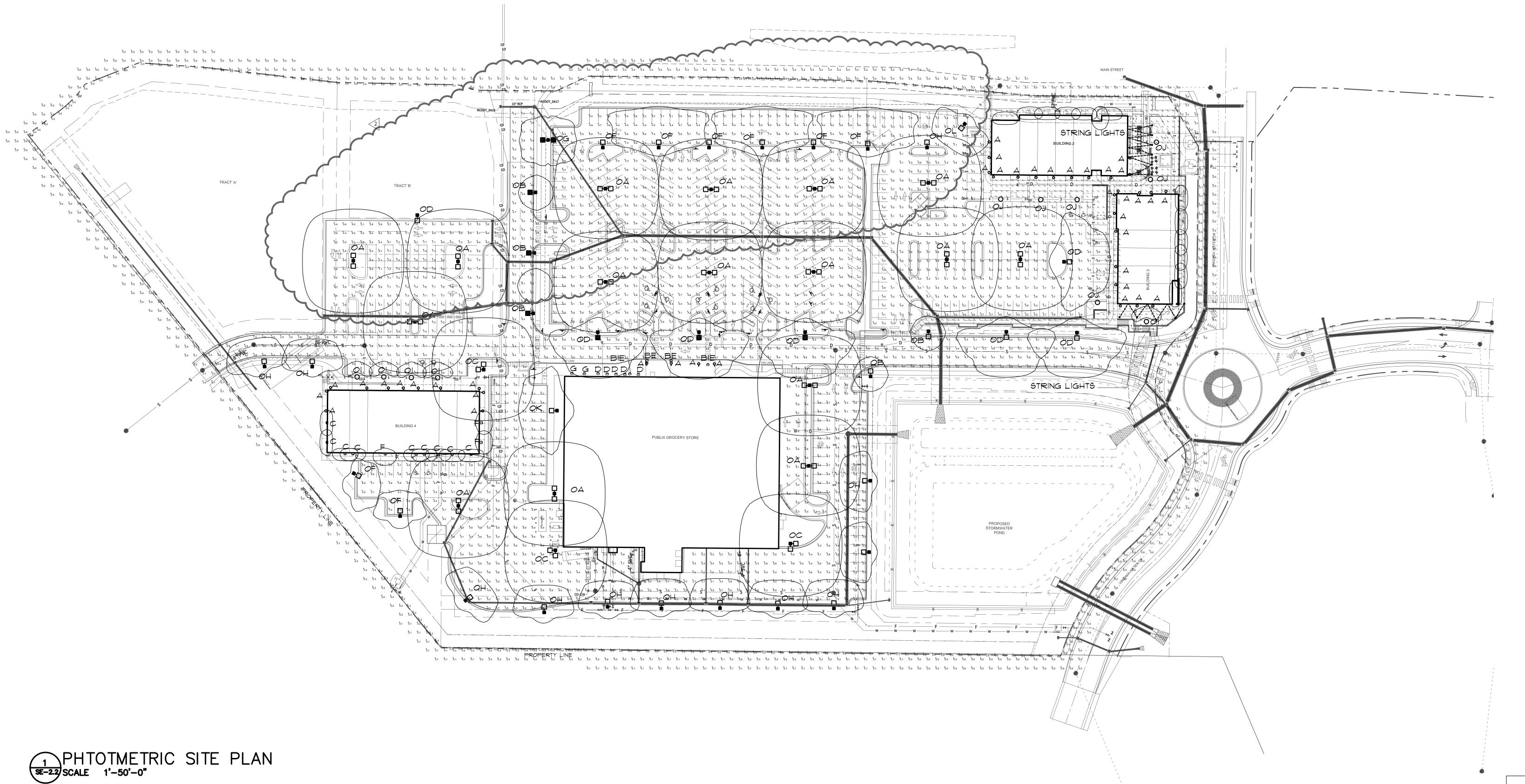
SHEET TITLE:
SITE LIGH

/10/22 PERMIT & PRICING
09/12/22 TOWN CIVIL/ELEC RESUBMITTAL
09/19/23 TOWN CIVIL/ELEC RESUBMITTAL

PROJECT NO: 20–117
FILE: 22922SE21

HEET OF

Calculation Summary									
Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min	PtSpcLr	PtSpcTb
Bldg 4 Parking Lot	Illuminance	Fc	6.47	8.3	2.7	2.40	3.07	10	10
Property Line	Illuminance	Fc	0.16	0.5	0.0	N.A.	N.A.	10	N.A.
20' Beyond Property Line	Illuminance	Fc	0.10	0.5	0.0	N.A.	N.A.	10	10
Entrance Drive & Parking	Illuminance	Fc	5.12	9.0	0.2	25.60	45.00	10	10
Publix Drive-Thru	Illuminance	Fc	6.03	8.9	5.1	1.18	1.75	10	10
Publix Front Parking & Drives	Illuminance	Fc	5.35	8.8	0.7	7.64	12.57	10	10
Publix & Retail 4 Rear & Sides	Illuminance	Fc	5.14	9.7	0.1	51.40	97.00	10	10
Retail Walkway Area	Illuminance	Fc	1.88	5.5	0.0	N.A.	N.A.	6	6
Retail Parking	Illuminance	Fc	5.16	9.6	2.5	2.06	3.84	10	10
SITE	Illuminance	Fc	5.34	9.7	0.1	53.40	97.00		



Hiscutt Q Associates ARCHITECTURE

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WALLBROOK
US 401 at Virginia Water Drive
WAKE COUNTY, NORTH CAROLINA
FOR.

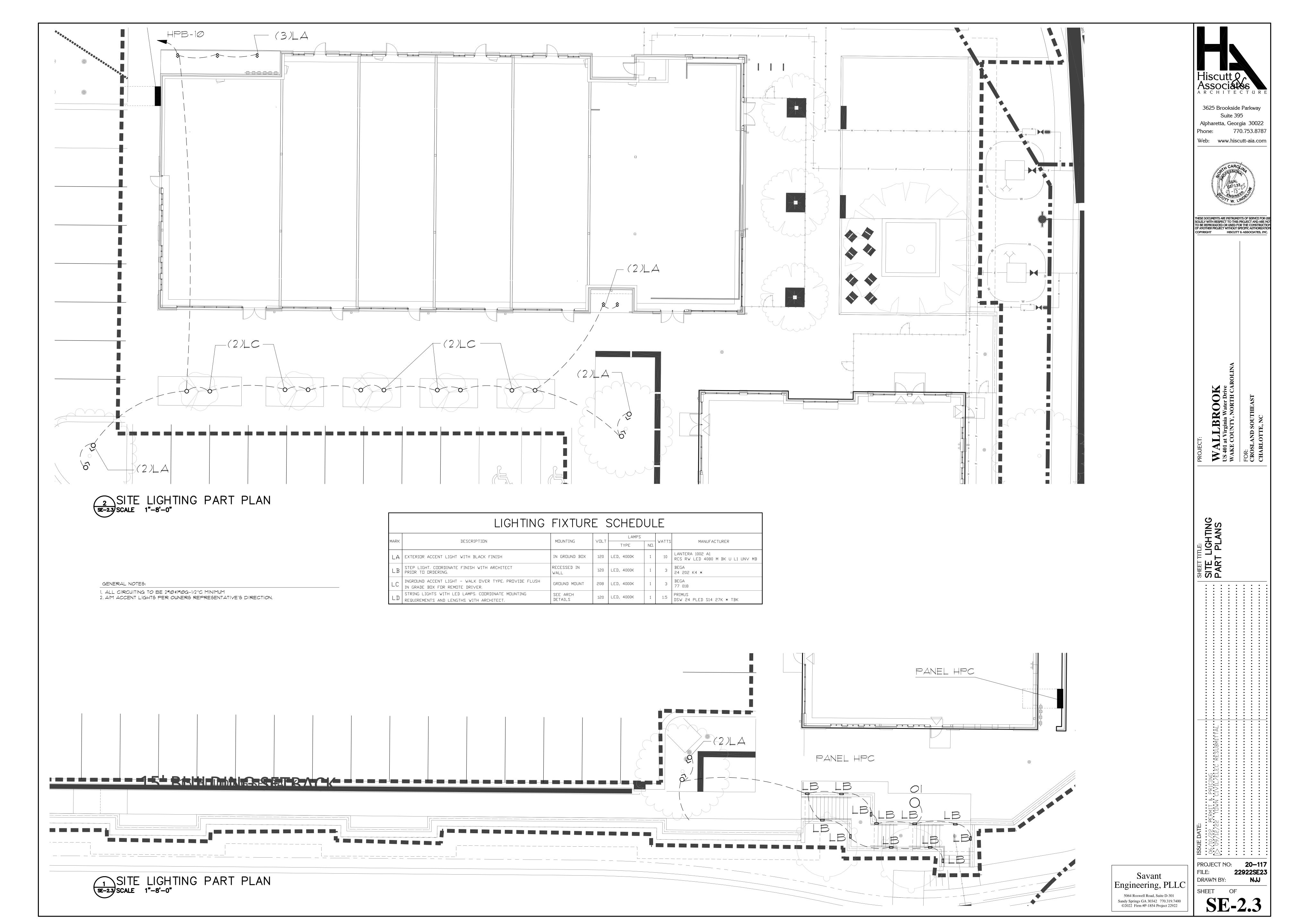
HOTOMETRIC SITE LAN

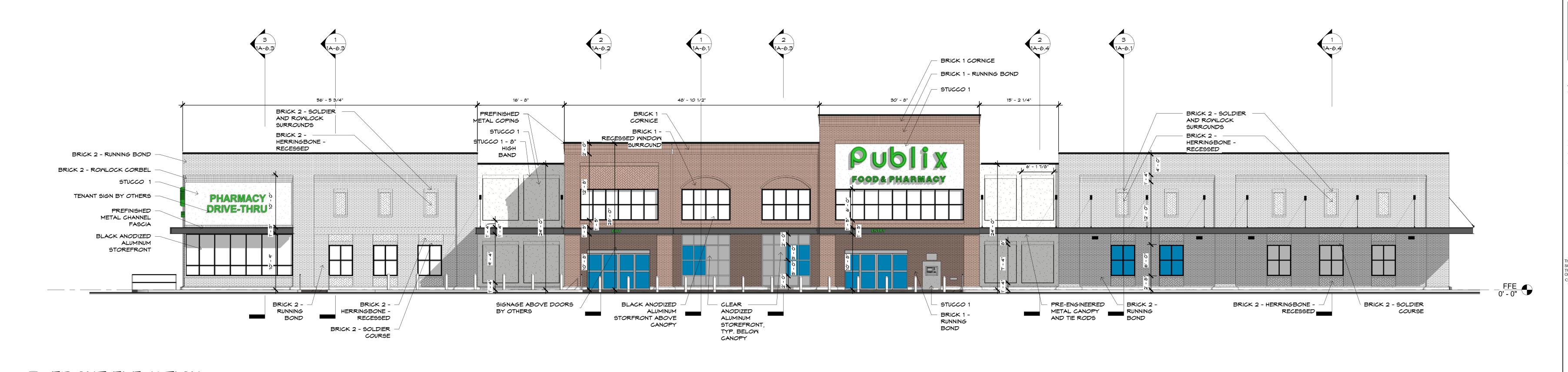
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FILE: 22909SE22
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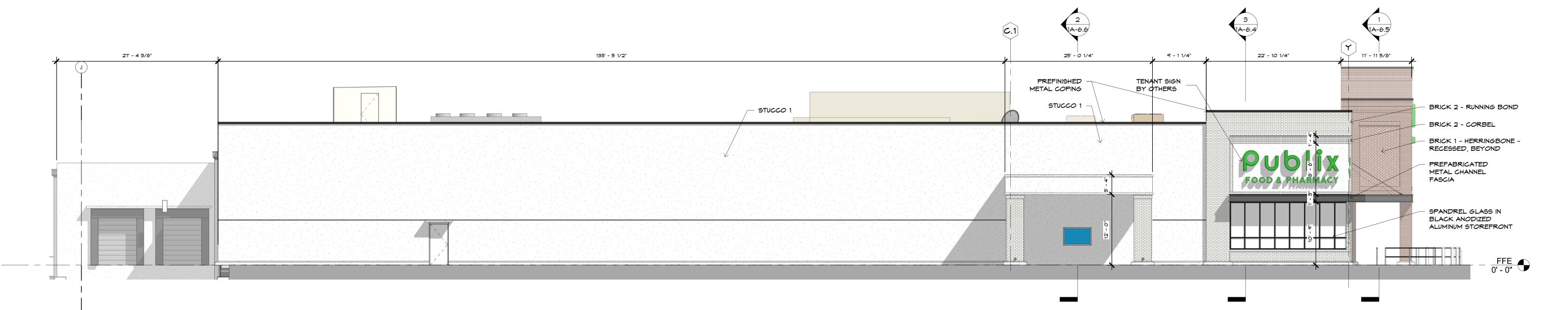
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FRONT ELEVATION 1A-4.1 SCALE 1/8" = 1'-0"



2 LEFT ELEVATION

1A-4.1 SCALE 1/8" = 1'-0"

EXTERIOR FINISH SCHEDULE				
MATERIAL DESCRIPTION	MANUFACTURER	COLOR	REMARKS:	
BR-1 - BRICK 1	CHEROKEE BRICK	"MELROSE"	MODULAR	
BR-2 - BRICK 2	CHEROKEE BRICK	"BROOKHAVEN"	MODULAR	
BR-2 - BRICK 3	CHEROKEE BRICK	"MELROSE"	MODULAR THIN BRICK	
BRICK MORTAR	CEMEX	"IVORY"		
STC 1 - FIELD STUCCO	FINESTONE STUCCO	MATCH DRYVIT 110 "VAN DYKE"	SAND FINE	
SF-1 - ALUMINUM STOREFRONT FRAME 1		CLEAR ANODIZED FRAME		
SF-2 ALUMINUM STOREFRONT FRAME 2		BLACK ANODIZED FRAME		
PREFINISHED ALUMINUM SCUPPERS & DOWNSPOUTS	PAC-CLAD	"MATTE BLACK"		
MTL-1 - PREFINISHED METAL PANEL	PAC-CLAD	"WEATHERED STEEL"	FLUSH PANELS	
MTL-2 - PREFINISHED METAL CANOPY & COPING	PAC-CLAD	"MATTE BLACK"	STANDING SEAM JOINTS	
MTL-3 - PREFINISHED METAL COPING	PAC-CLAD	"BONE MHITE"	STANDING SEAM JOINTS	

- FINISH MATERIAL NOTES:
- 1. SUBMIT AND OBTAIN APPROVED SAMPLES BEFORE PROCEEDING WITH WORK.
- 2. ALL MASONRY VENEER SHALL BE OBTAINED FROM SAME MANUFACTURER'S RUN FOR COLOR CONSISTENCY. MASON TO BREAK APART AND MIX PALETTES ON JOB SITE FOR CONSISTENT COLOR MIXING. NO EXCEPTIONS.

3. A MOCK UP PANEL IS REQUIRED TO BE APPROVED BY ARCHITECT AND/OR OWNER'S REPRESENTATIVE BEFORE FINISH MORK MAY BEGIN.

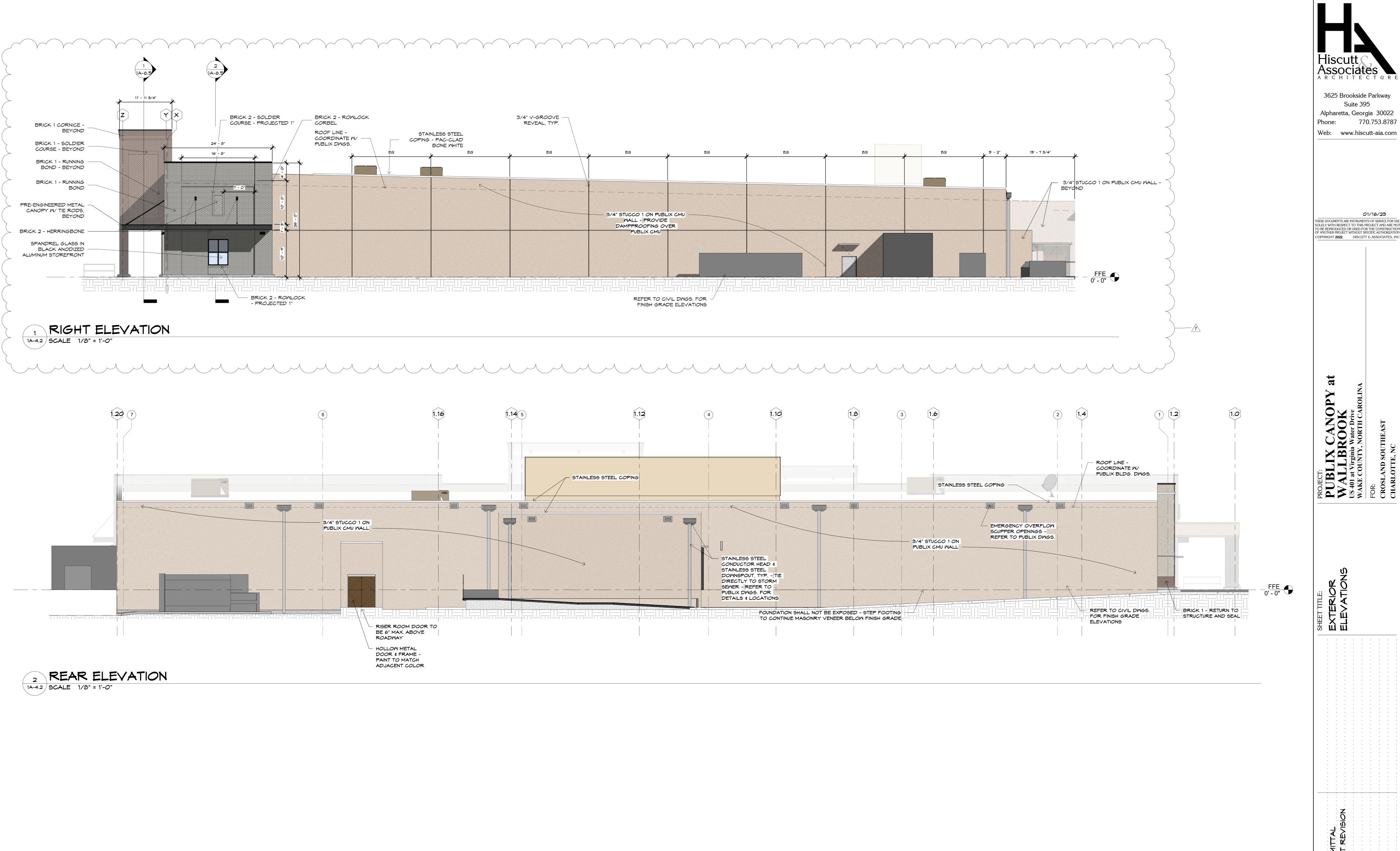
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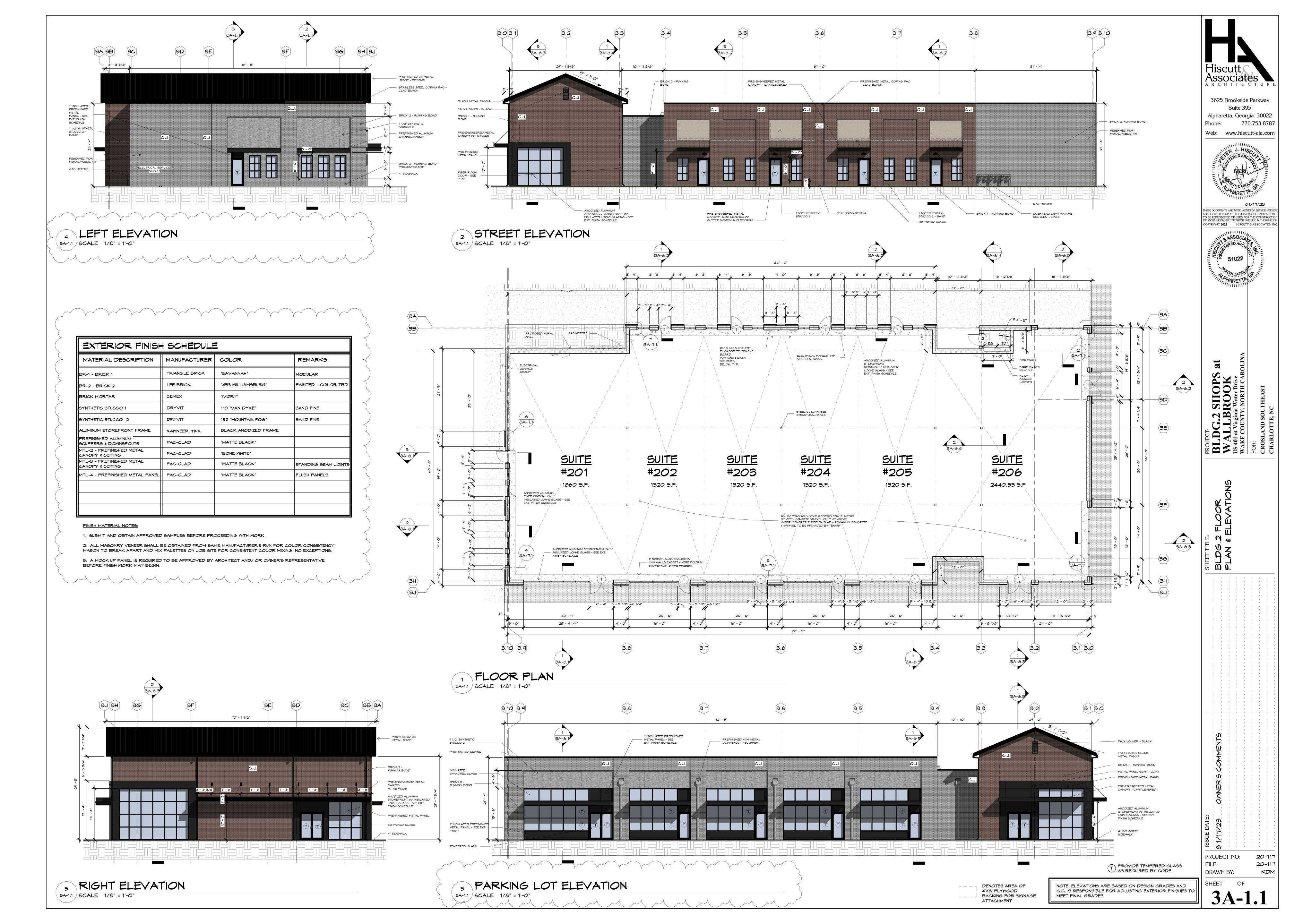


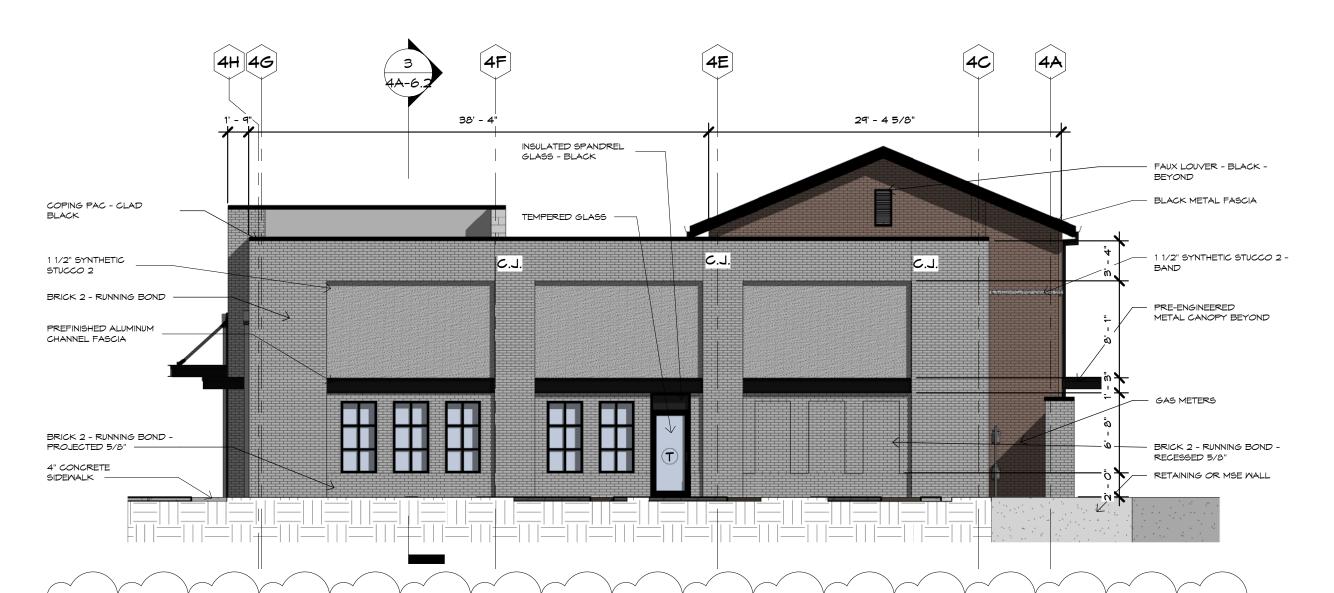
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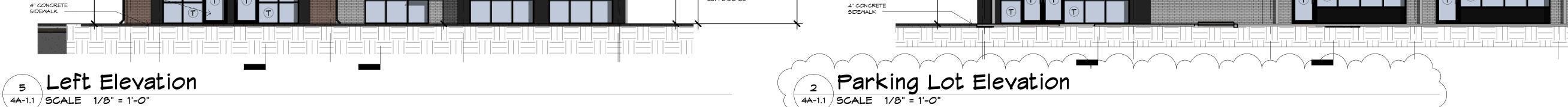


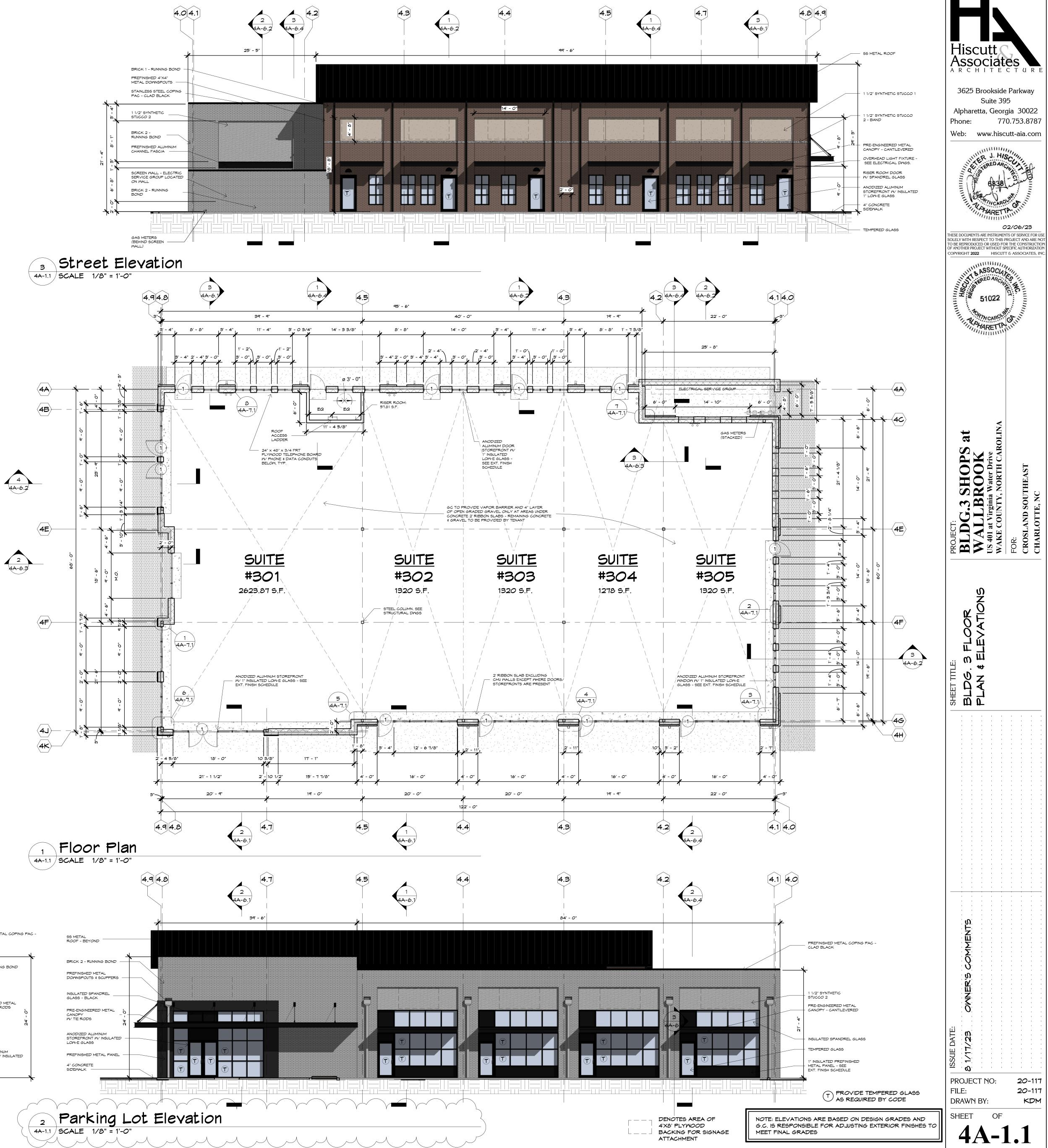
4 Right Elevation
4A-1.1 SCALE 1/8" = 1'-0"

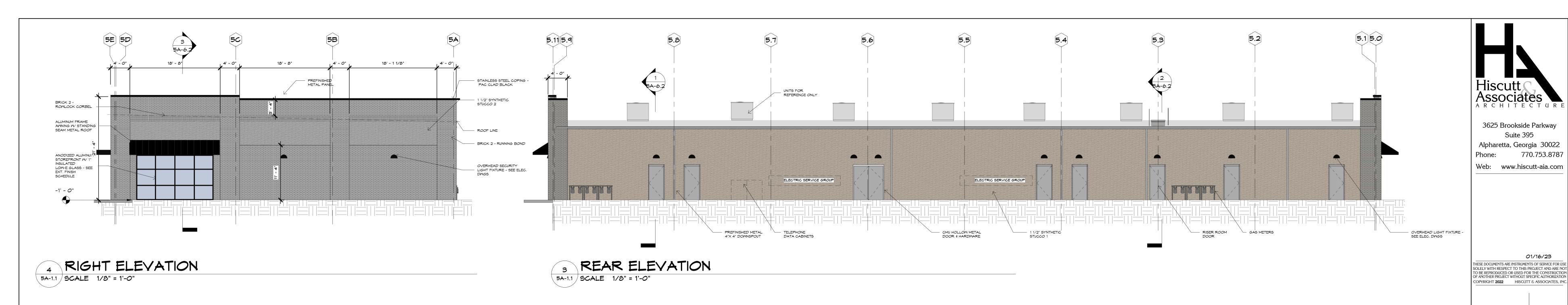
EXTERIOR FINISH SCHEDULE					
MATERIAL DESCRIPTION	MANUFACTURER	COLOR	REMARKS:		
BR-1 - BRICK 1	TRIANGLE BRICK	"SAVANNAH"	MODULAR		
BR-2 - BRICK 2	LEE BRICK	"453 MILLIAMSBURG"	PAINTED - COLOR TBD		
BRICK MORTAR	CEMEX	"IVORY"			
SYNTHETIC STUCCO 1	DRYVIT	110 "VAN DYKE"	SAND FINE		
SYNTHETIC STUCCO 2	DRYVIT	132 "MOUNTAIN FOG"	SAND FINE		
ALUMINUM STOREFRONT FRAME	KAMNEER, YKK	BLACK ANODIZED FRAME			
PREFINISHED ALUMINUM SCUPPERS & DOWNSPOUTS	PAC-CLAD	"MATTE BLACK"			
MTL-2 - PREFINISHED METAL CANOPY & COPING	PAC-CLAD	"BONE WHITE"			
MTL-3 - PREFINISHED METAL CANOPY & COPING	PAC-CLAD	"MATTE BLACK"	STANDING SEAM JOINTS		
MTL-4 - PREFINISHED METAL PANEL	PAC-CLAD	"MATTE BLACK"	FLUSH PANELS		

- FINISH MATERIAL NOTES:
- 1. SUBMIT AND OBTAIN APPROVED SAMPLES BEFORE PROCEEDING WITH WORK.
- 2. ALL MASONRY VENEER SHALL BE OBTAINED FROM SAME MANUFACTURER'S RUN FOR COLOR CONSISTENCY. MASON TO BREAK APART AND MIX PALETTES ON JOB SITE FOR CONSISTENT COLOR MIXING. NO EXCEPTIONS.
- 3. A MOCK UP PANEL IS REQUIRED TO BE APPROVED BY ARCHITECT AND/ OR OWNER'S REPRESENTATIVE BEFORE FINISH WORK MAY BEGIN.









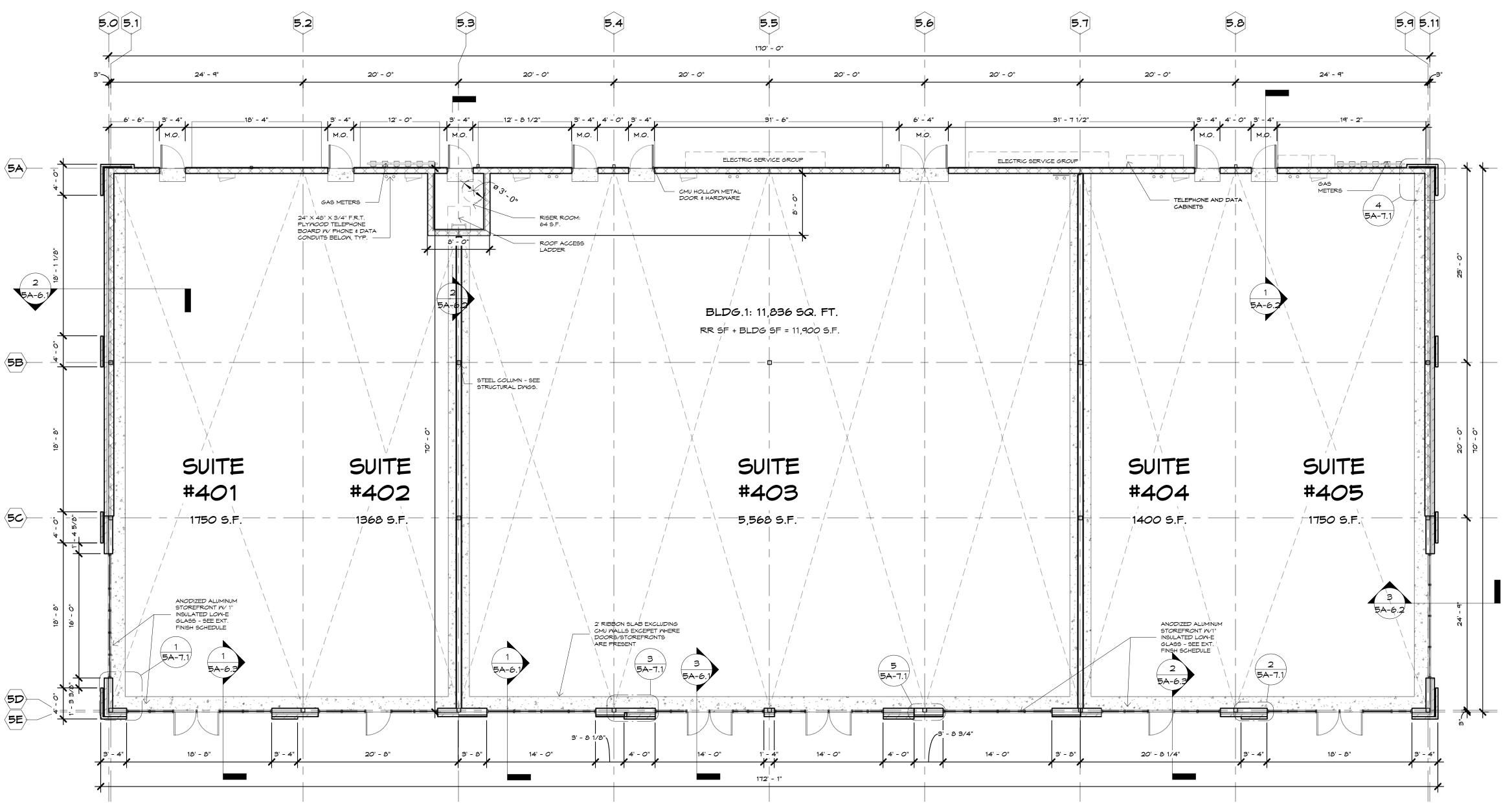
MANUFACTURER	COLOR	REMARKS:	
CHEROKEE BRICK	"MELROSE"	MODULAR	
LEE BRICK	"453 WILLIAMSBURG"	PAINTED - COLOR TBD	
CEMEX	"IVORY"		
DRYVIT	110 "VAN DYKE"	SAND FINE	
DRYVIT	132 "MOUNTAIN FOG"	SAND FINE	
KAMNEER, YKK	BLACK ANODIZED FRAME		
PAC-CLAD	"MATTE BLACK"		
PAC-CLAD	"BONE WHITE"		
PAC-CLAD	"MATTE BLACK"	STANDING SEAM JOINTS	
PAC-CLAD	"MATTE BLACK"	FLUSH PANELS	
	LEE BRICK CEMEX DRYVIT DRYVIT KAWNEER, YKK PAC-CLAD PAC-CLAD PAC-CLAD	LEE BRICK "453 WILLIAMSBURG" CEMEX "IVORY" DRYVIT 110 "VAN DYKE" DRYVIT 132 "MOUNTAIN FOG" KAWNEER, YKK BLACK ANODIZED FRAME PAC-CLAD "MATTE BLACK" PAC-CLAD "BONE WHITE" PAC-CLAD "MATTE BLACK"	

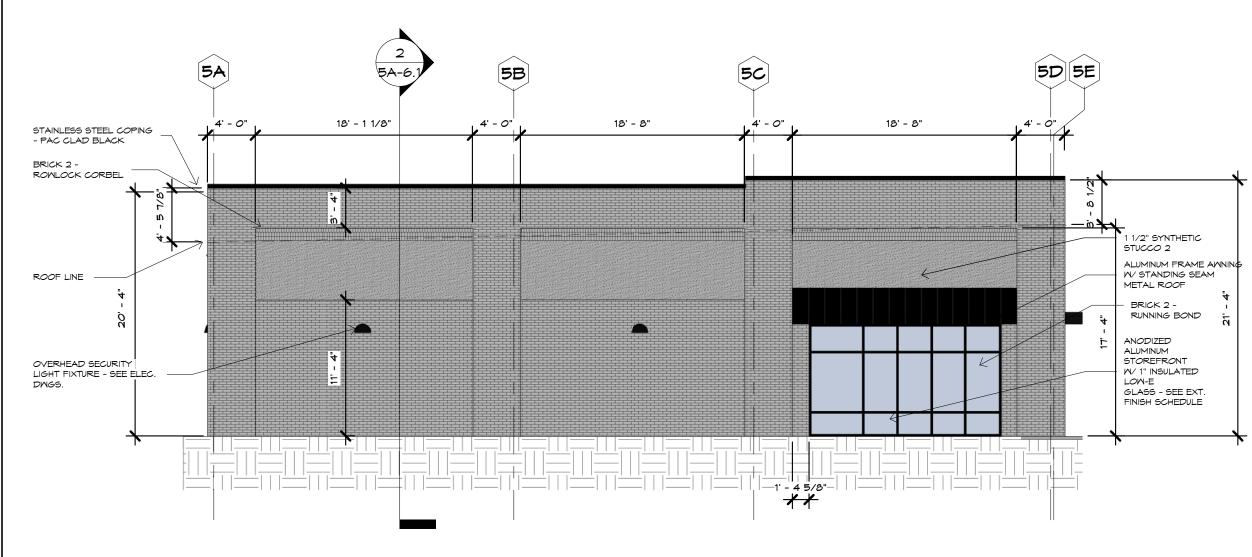
FINISH MATERIAL NOTES:

LEFT ELEVATION

5A-1.1 SCALE 1/8" = 1'-0"

- 1. SUBMIT AND OBTAIN APPROVED SAMPLES BEFORE PROCEEDING WITH WORK.
- 2. ALL MASONRY VENEER SHALL BE OBTAINED FROM SAME MANUFACTURER'S RUN FOR COLOR CONSISTENCY. MASON TO BREAK APART AND MIX PALETTES ON JOB SITE FOR CONSISTENT COLOR MIXING. NO EXCEPTIONS.
- 3. A MOCK UP PANEL IS REQUIRED TO BE APPROVED BY ARCHITECT AND/ OR OWNER'S REPRESENTATIVE BEFORE FINISH WORK MAY BEGIN.





25' - 4" 20' - 8" __ TENANT SIGNAGE BY OTHERS PREFINISHED METAL PANELS STAINLESS STEEL COPING -PAC CLAD BLACK - BRICK 1 - HERRINGBONE - RECESSED 5/8" ALUMINUM FRAME AWNING
 W/ SS METAL ROOF TENANT TENANT TENANT STUCCO 2 TENANT SIGNAGE N BY OTHERS (DESIGNED BY OTHERS) TEMPERED GLASS DENOTES AREA OF

2 FRONT ELEVATION

5A-1.1 SCALE 1/8" = 1'-0"

FLOOR PLAN

5A-1.1 SCALE 1/8" = 1'-0"

T PROVIDE TEMPERED GLASS AS REQUIRED BY CODE 4'X8' PLYMOOD BACKING FOR SIGNAGE ATTACHMENT FACADE PERCENTAGES: 57.6% BRICK NOTE: ELEVATIONS ARE BASED ON DESIGN GRADES AND G.C. IS RESPONSIBLE FOR ADJUSTING EXTERIOR FINISHES TO 31% GLASS MEET FINAL GRADES

TENANT

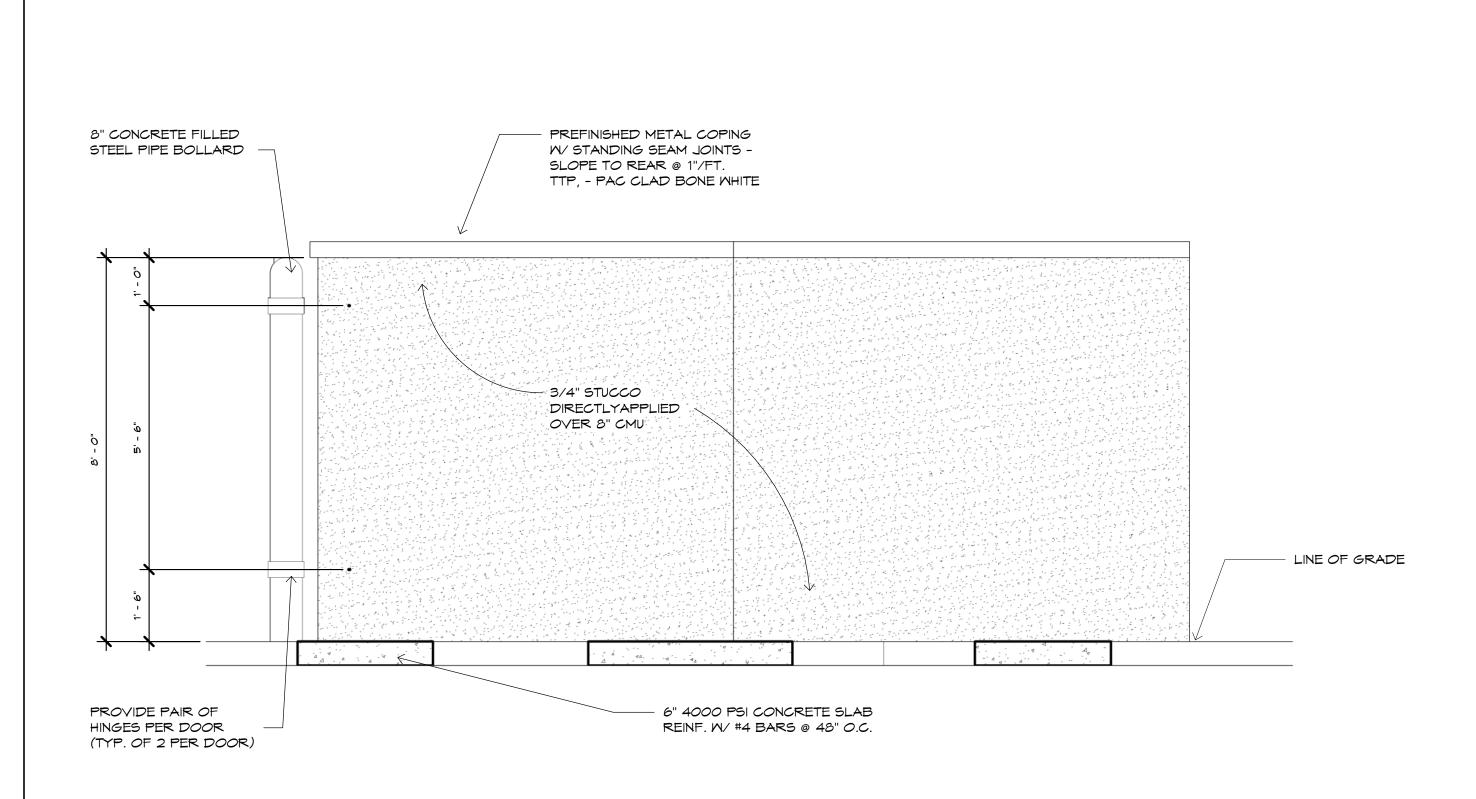
PROJECT NO: FILE: 20-117 DRAWN BY: KDM

5.95.11

TENANT

01/16/23

SHEET



7 Dumpster Right Elevation 3A-8.0 SCALE 1/2" = 1'-0"

Bollard Detail

3A-8.0 SCALE 3/4" = 1'-0"

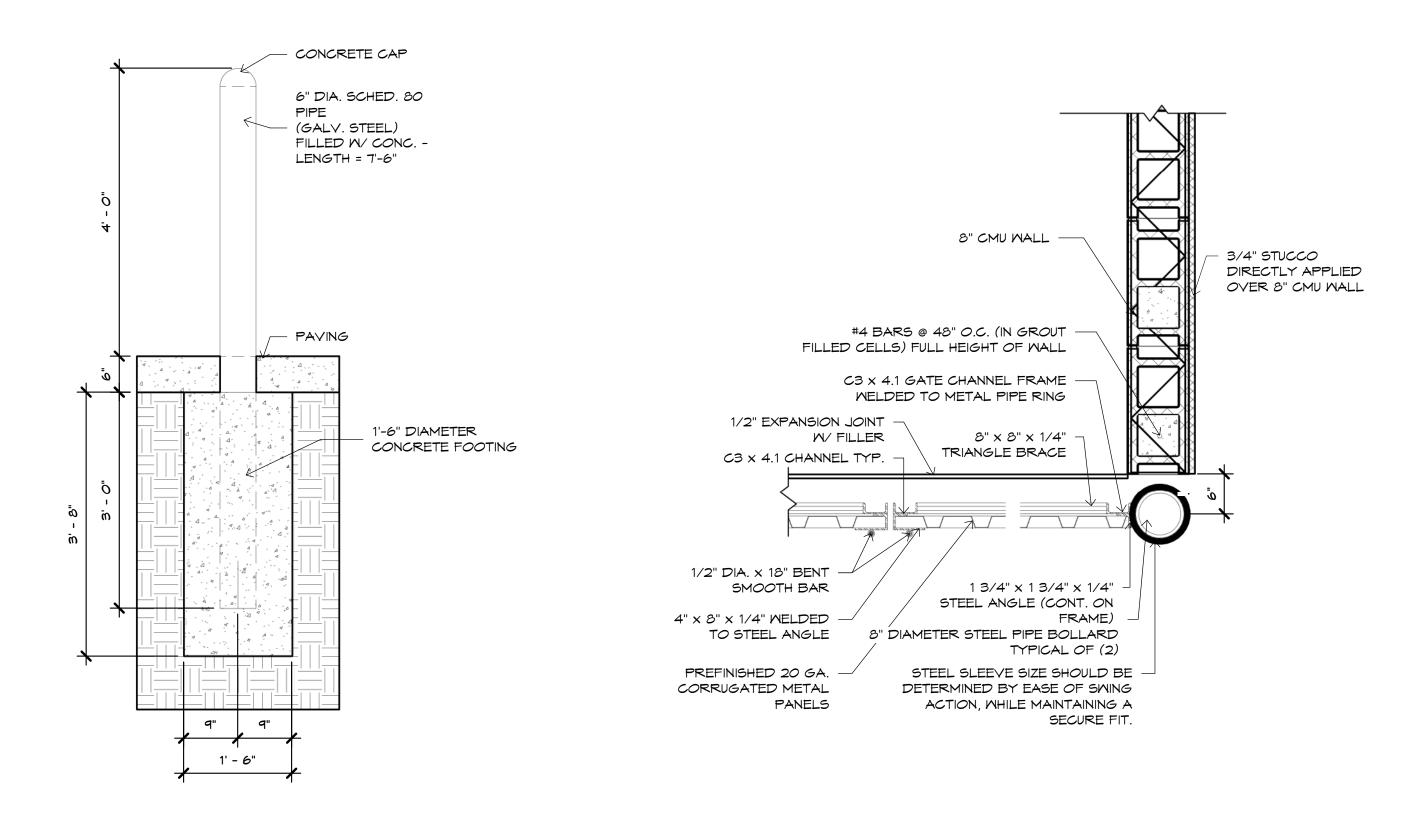
— 3/4" F.R.T. EXT. GRADE 24 GA. PREFINISHED METAL COPING W/ STANDING SEAM PLYMOOD JOINTS OVER MEMBRANE - STAINLESS STEEL SCREMS W/ NEOPRENE WASHERS @ 6" O.C. FLASHING ON 3/4" F.R.T. EXTERIOR GRADE PLYWOOD INTO HORIZONTAL SLOTTED SUBSTRATE OPENINGS IN COPING 2X F.R.T. WOOD BLOCKING - SEAL JOINT 6" BOLLARDS FOR DUMPSTER PAD, TYP. BOND BEAM 3/4" STUCCO DIRECTLY APPLIED 8" CMU MALL 6" 4000 PSI CONCRETE SLAB REINF. W/ #4 BARS @ 48" O.C. LINE OF GRADE GROUT CAVITY BELOW GRADE - 2'-0" DIA. MINIMUM CONCRETE LL ALL CELLS BELOW GRADE FOUNDATION - 1/2" DIA. X 4" WELDED STUD ANCHORS AT QUARTER POINTS

5 Gate Detail

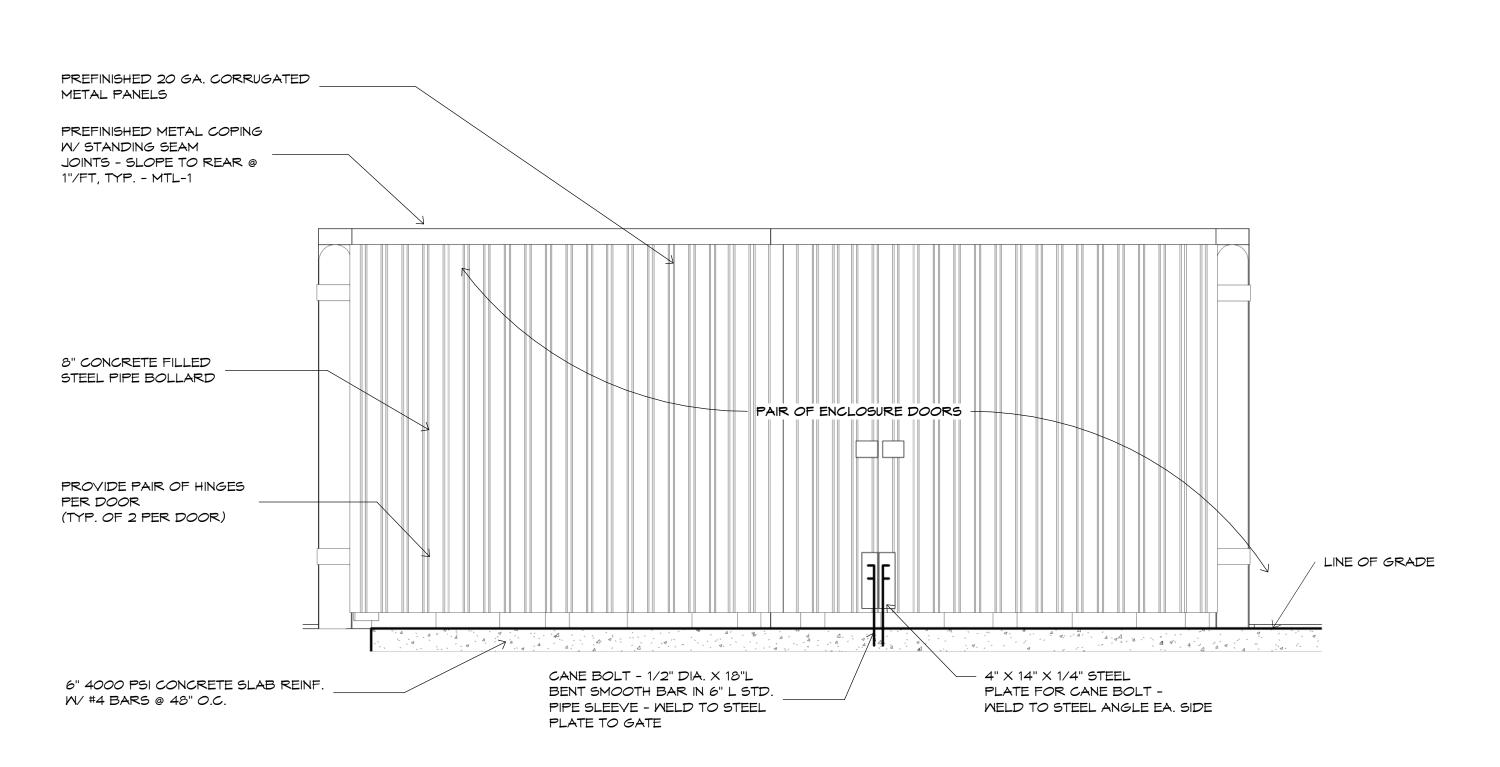
3A-8.0 SCALE 1/2" = 1'-0"

4 Dumpster Wall Section

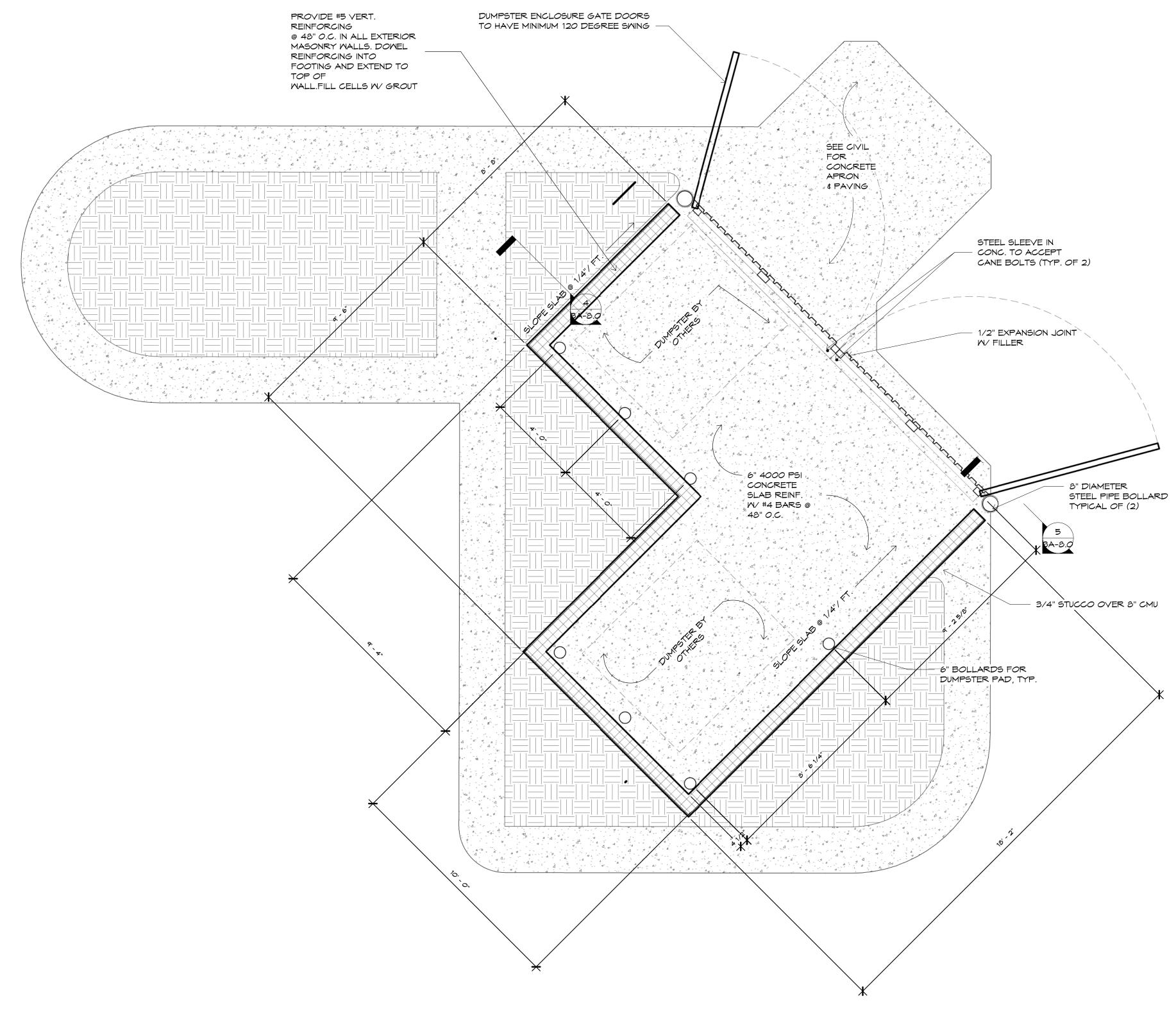
3A-8.0 SCALE 1/2" = 1'-0"



3 Dumpster Hinge Detail
3A-8.0 SCALE 1" = 1'-0"



6 Dumpster Front Elevation
3A-8.0 SCALE 1/2" = 1'-0"



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FILE:

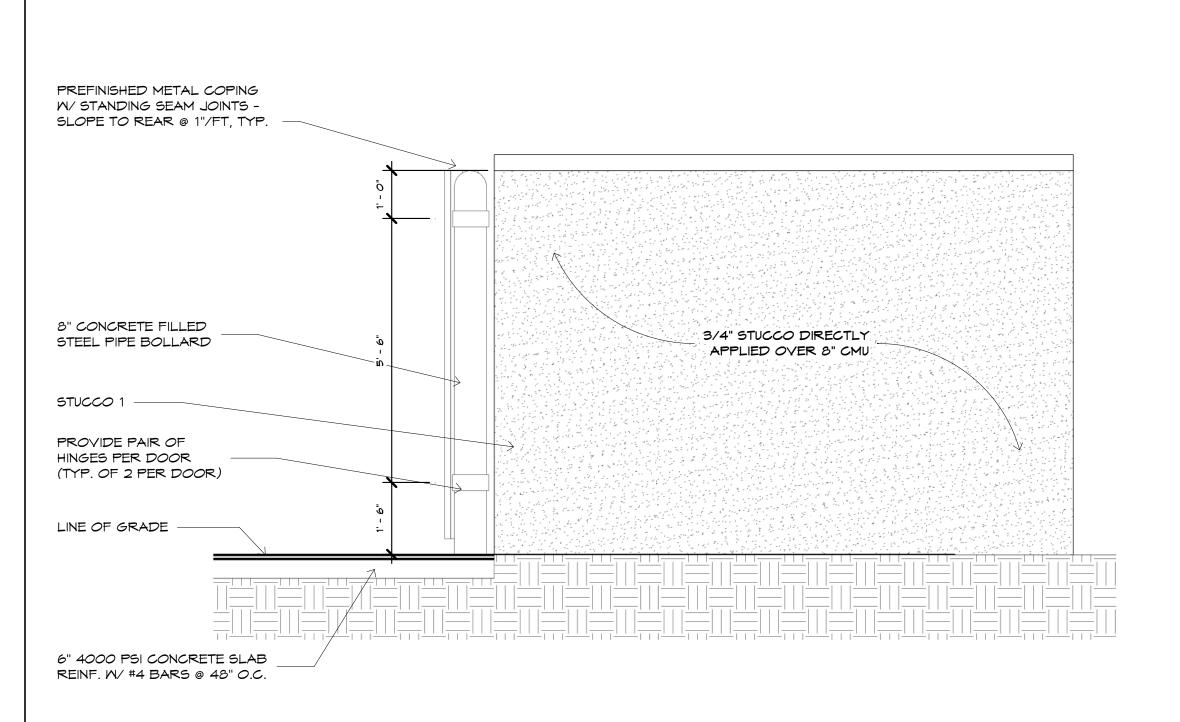
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3A-8.0

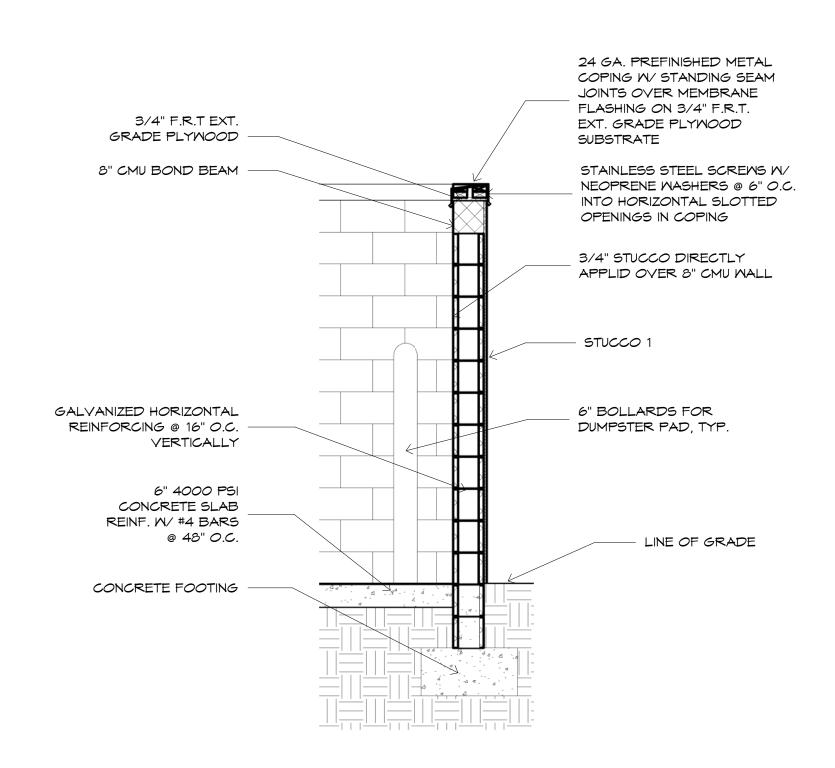
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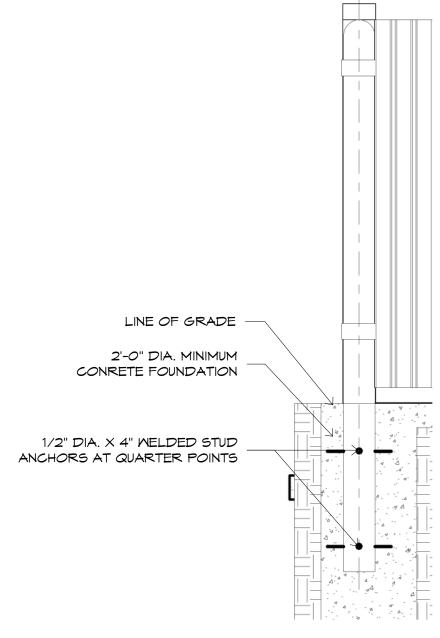
20-117

1 Dumpster Plan
3A-8.0 SCALE 3/8" = 1'-0"

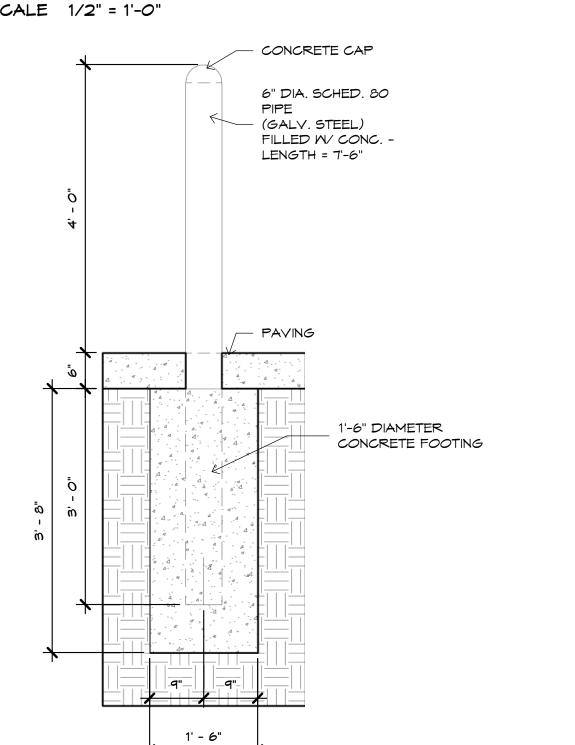


2 Dumpster Right Elevation 5A-8.0 SCALE 1/2" = 1'-0"

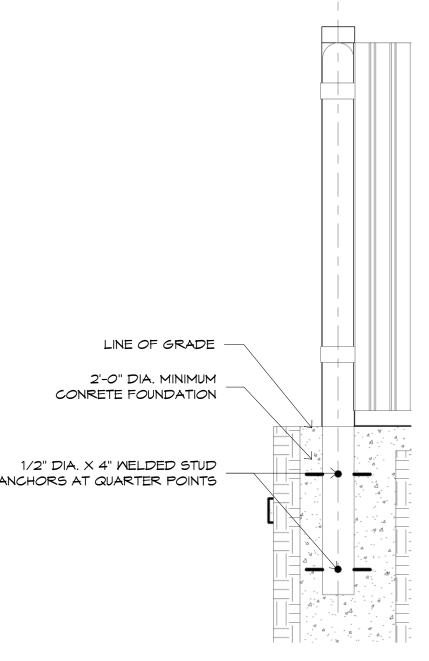




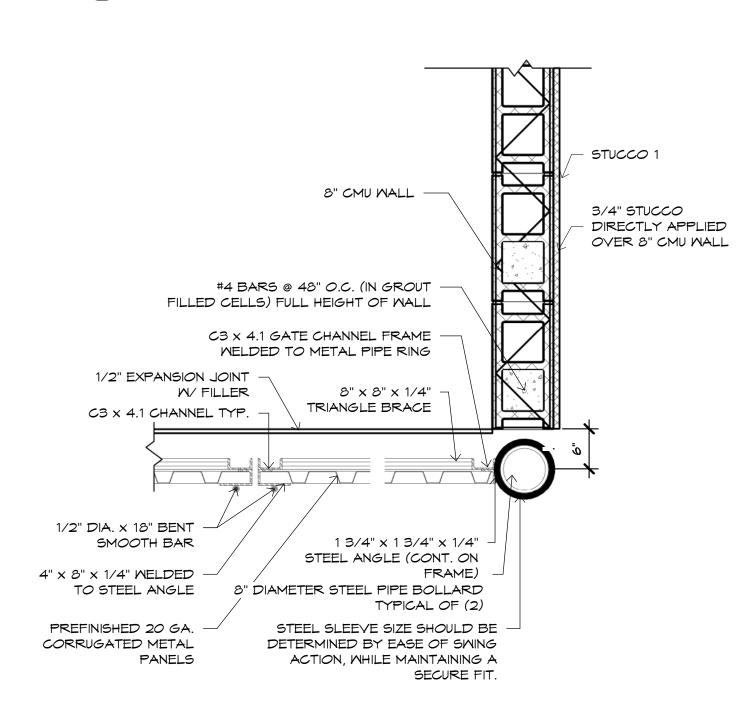
3 Dumpster Wall Section 5A-8.0 SCALE 1/2" = 1'-0"



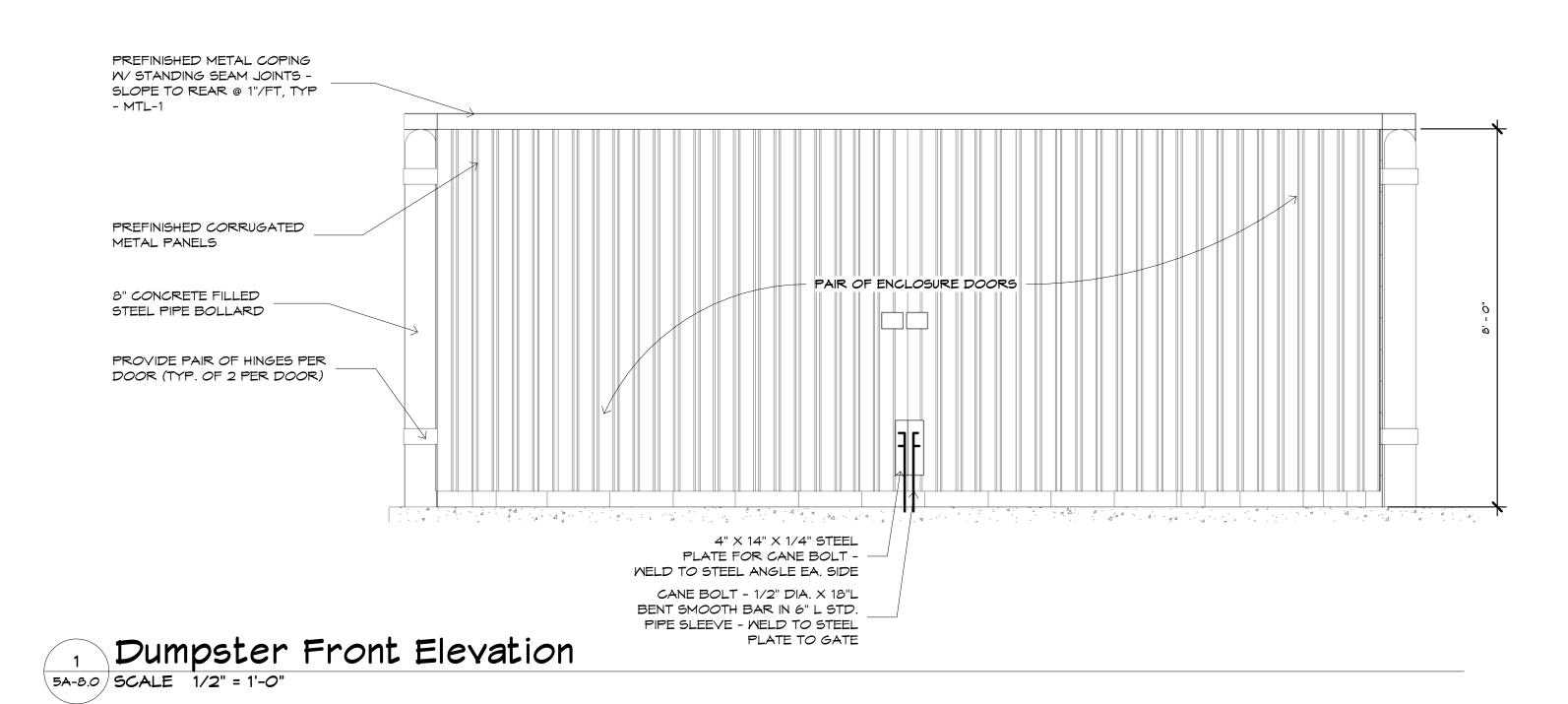
Bollard Detail 6 Bollara De 5A-8.0 SCALE 3/4" = 1'-0"

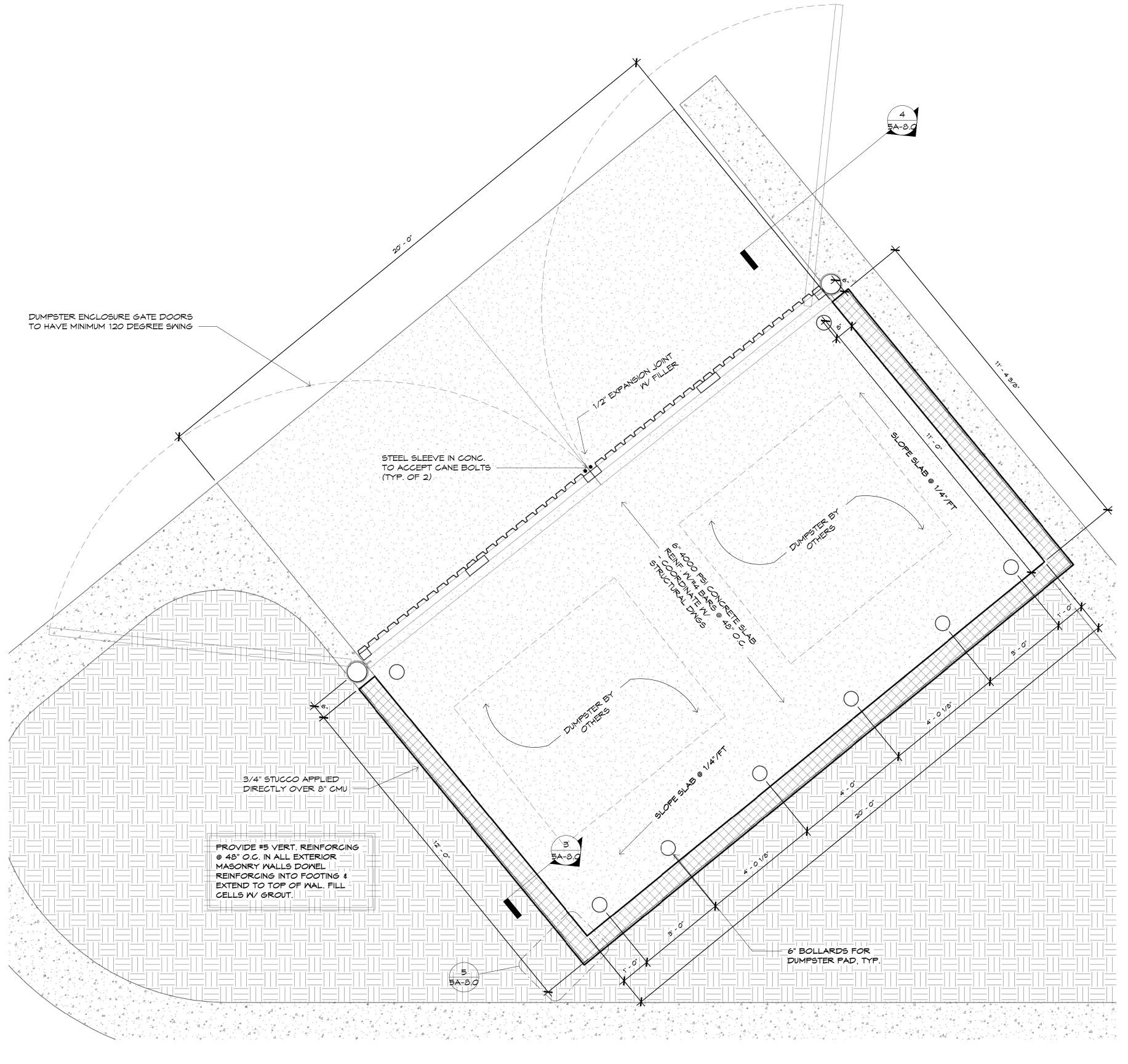


4 Gate Detail 5A-8.0 SCALE 1/2" = 1'-0"



5 Dumpster Hinge Detail
5A-8.0 SCALE 1" = 1'-0"





7 Dumpster Plan 5A-8.0 SCALE 1/2" = 1'-0"

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