Agenda Item F.4



Memo

To:	Mayor Currin and Board of Commissioners
From:	Michael Elabarger, Senior Planner
Date:	September 5, 2023
Re:	Hills at Harris Creek
	Map Amendment Rezoning REZ-23-03
	Voluntary Annexation Petition ANX-22-04

Background

Rezoning (REZ-23-03)

The Town of Rolesville Planning Department received an initial Map Amendment (Rezoning) application in January of 2022 [MA 22-01] for approximatley 116 acres consisting of four (4) tracts of land located on the north side of Mitchell Mill Road, between Jonesville and Rolesville Roads. This application requested rezoning from Wake County's R-30 Zoning District to the Town's Land Development Ordinance (LDO) zoning district of Neighborhood Center.(NC) as a Conditional Zoning (CZ) District. This was reviewed several times over the course of 2022. In early 2023, the Applicant chose to significantly revise the scope of the application, and REZ-23-03 was submitted to usurp MA 22-01, and this requested a small NC District with the majority of the land area proposed for the Residential High Density (RH) District, both as Conditional Zoning (CZ) Districts. Project details include maximums of 270 total residential dwellings units (combined single-family detached and attached), with no more than 115 of them being detached units, and a non-residential component in the NC-CZ District meeting the minimum requirements for that District.



Hills at Harris Creek Concept/Sketch Plan

ANNEXATION PETITION (ANX-22-04

A Voluntary Annexation Petition has also been submitted and reviewed and is processing simultaneously with this rezoning request. As provided by G.S. 160A-31, the petition was investigated by the Town Clerk as to its sufficiency of meeting G. S. 160A-31 after the October 4, 2022 Town Board Meeting; see Attachment 14. On August 1, 2023, the Town Board of Commissioners scheduled a legislative hearing for the annexation petition (to be held September 5, 2023).

The Rezoning applicant has proposed a Concept/Sketch Plan (see Attachment 5, Exhibit 1) and a set of Conditions of Approval (see Attachment 9, Exhibit 5).

Applicant Justification

The applicant provided this brief statement regarding the submittal (see Attachment 4 also).

The proposed rezoning of the parcels described above is made in careful consideration of the Rolesville Comprehensive Plan 2017. The Future Land Use Map designates these parcels as Medium Density Residential, and we believe that a combination of the Mixed Use Neighborhood Center Conditional District and Residential High Density Conditional District would allow for the best variety of housing types to meet this objective while integrating seamlessly with the surrounding community. The proposed community includes a mix of housing types (single-family detached homes and townhomes) and is within the density levels recommended by the Comprehensive Plan.

Mixed-use neighborhoods such as the one proposed herein are routinely applauded for their connectivity and walkability and the multiple housing types appeal to various income levels while still maintaining the appeal and quality of the neighborhood. The proposed rezoning is in accordance with the Comprehensive Plan and reasonable and in the public interest. We request your support for the proposed zoning.

Neighborhood Meetings

Under the header of MA 22-01, the Applicant conducted Neighborhood meetings on March 16, 2022 and January 10, 2023. With the new application REZ-23-03, the applicant held a meeting on May 23, 2023 – see Attachment 11. Per the Applicant, four (4) persons attended/signed-in at the most recent meeting.

Comprehensive Plan

Land Use

The 2017 Comprehensive Plan's Future Land Use Map designates the subject property, and the entire area south of Harris Creek to Mitchell Mill Road, as appropriate for Medium-Density Residential development. Per the Plan, this is defined as:

Predominantly single family residential uses with portion of duplex, townhouse, or multifamily residential. These are lots or tracts at a density range of three to five (3-5) dwelling units per gross acre including preserved open space areas along with limited non-residential uses under planned unit development or form base code provisions.

Community Transportation Plan

The Town of Rolesville's Community Transportation Plan (CTP, adopted 2021) includes recommendations for Thoroughfares, Collectors, and intersections.

Thoroughfare Recommendations

• Mitchell Mills Road is planned to be a 4-lane, Raised Median-divided section with curb & gutter, bike lanes, and sidewalks.

Collector Recommendations

 At the far northern tip of the subject property, an east/west Collector roadway is identified. As demonstrated in the Concept/Sketch Plan (Attachment 5), the Applicant is proposing a more southern route for this roadway, moving it away from the environmental features (Harris Creek) further to the north, and bringing it closer to Mitchell Mill, where it can serve as a more near-by parallel roadway to the Arterial that Mitchell Mill will eventually function as. This more southern route aligns more so with a similar alignment approved with MA 22-06, 5109 Mitchell Mill Road, which when built will connect to Jonesville Road west of this subject property/project.

Intersection Recommendations

- There are no intersection recommendations associated with the subject property.
- The closest intersection recommendations are located at Mitchell Mill and Rolesville Roads, for an intersection realignment.

Greenway and Bike Plans

As per the 2022 Greenway and Bike Plans, proposed pedestrian routes are shown in the following locations:

- A ten foot (10') greenway is shown on the northern end of the property, on the south side of the Harris Creek.
- A ten foot (10') side path is illustrated on the north side of Mitchell Mill Road.
- Bicycle lane within Mitchell Mill Road.

Consistency

The applicant's rezoning request is consistent with the Town of Rolesville's Comprehensive Plan for the following reasons:

- The proposed mix of residential product types, with a commercial element, fits the Medium Density Residential land use description.
- The proposed vehicular circulation network will enhance or establish Thoroughfare and Collector connections, respectively, as recommended by the Town's Community Transportation Plan.
- The proposed Greenways will establish pedestrian connections as recommended by Rolesville's Greenway Plan.

Traffic

Traffic Impact Analysis

The consulting firm, Ramey Kemp Associates, performed the Traffic Impact Analysis (TIA) for this project on behalf of the Town; the study analyzed a development of 155 Single Family Detached and 115 Single family Attached (townhome) housing units, and 12,000 square feet of Day Care center non-residential use. The Final Report dated June 2023 (sealed June 23, 2023) is included as Attachment 12 to this memo. Traffic conditions during weekday AM and PM peak hours were looked at in three scenarios: 2022 Existing conditions, 2027 No-build conditions, and 2027 Build conditions. See excerpted Table E-1 from the TIA report:

Land Use (ITE Code)	Intensity	Daily Traffic	M AM Pea	/eekda <mark>ak Hou</mark> (vph)	y r Trips	W PM Pea	/eekda ak Hou (vph)	ir Trips
		(vpu)	Enter	Exit	Total	Enter	Exit	Total
Single-Family Home (210)	155 DU	1,510	28	83	111	95	55	150
Single-Family Attached Housing (220)	115 DU	812	14	45	59	44	26	70
Day Care Center (565)	12 KSF	572	70	62	132	63	70	133
Total Trips	Total Trips		112	190	302	202	151	353
Pass-By Trips: Day (44% Pl	Pass-By Trips: Day Care Center (44% PM)		2	-	221	-30	-30	-60
Total Primar	Total Primary Trips		112	190	302	172	121	293

Table E-1: Site Trip Generation

Three (3) intersections were studied for capacity analysis and Level Of Service (LOS) impact of this development – US 401 Bypass and Jonesville Road; US 401 Bypass and Eastern U-turn location; Mitchell Mill Road and Jonesville Road / Peebles Road.

TIA Summary – Intersection Improvements	
Recommendations	

Required Frontage Improvements per Rolesville Community Transportation Plan (CTP)	• Widen one-half section of Mitchell Mill Road along the site frontage to this roadway's ultimate section (4-lane Median [raised] divided.
US 401 Bypass and Jonesville Road	 Conduct a full signal warrant analysis prior to Full Build- out of the proposed development and install a traffic signal if warranted and approved by NCDOT and Town.
Mitchell Mill Road and Jonesville Road / Peebles Road	 Construct a south-bound (Jonesville Rd) left-turn lane with at least 100 feet of storage and appropriate decel and taper. It should be noted that this improvement was also identified by the 5109 Mitchell Mill Rd TIA.
	• Construct a westbound (Mitchell Mill Road) right-turn lane with at least 100 feet of storage and appropriate decel and taper.
	 Conduct a full signal warrant analysis prior to Full Build- out of the proposed development and install a traffic signal if warranted and approved by NCDOT and Town.
Mitchell Mill Road and Site Access 1	 Construct the southbound approach (Site Access 1) as a right-in/right-out with one ingress lane and one egress lane.
	Provide stop control for the southbound approach (Site Access 1).
	 Construct a concrete median on Mitchell Mill Road that restricts access to right-in/right-out.
Mitchell Mill Road and Site Access 2	 Construct the southbound approach (Site Access 2) with one ingress lane and one egress lane.
	 Provide stop control for the southbound approach (Site Access 2).
	 Construct the westbound (Mitchell Mill Road) right-lane with at least 100 feet of storage and appropriate decel and taper.

Development Review

The Technical Review Committee (TRC) reviewed two (2) versions of the Rezoning application (and three (3) reviews of MA 22-01 prior to that), with all comments pertinent to the consideration of the proposed districts and the general development plan being resolved. Note that this does not mean that all LDO subdivision and/or site development regulations have been demonstrated, as Attachment 5 is only a conceptual plan, and not an engineered and dimensioned layout.

LDO Appendix A / 2.3.G. Review Standards with Staff Responses

The Land Development Ordinance (LDO) Handbook, Appendix A, includes the following standards that may be considered when reviewing a rezoning request; Staff responses follow each standard below.

1. Is the application consistent with the Comprehensive Plan and other applicable adopted Town plans;

Staff finds that the general project intent to develop a mix of single-family detached and attached dwelling unit, with a commercial use associated with the Attached units, is consistent with the Medium Density Residential land use, and the pedestrian and transportation improvements will fulfill the development of the Community Transportation Plan, Greenway Plan, and Bicycle Plan.

2. Is the application in conflict with any provision of the LDO or the Town Code of Ordinances;

Based on the level of detail provided, the application materials appear consistent with the LDO; the project will demonstrate full compliance with subdivision/land development requirements at later stage of development.

3. Does the application correct any errors in the existing zoning present at the time it was adopted;

The subject property is currently under land use control of Wake County and therefore is not consistent with the 2017 Rolesville Comprehensive Plan; the property is not within the Town of Rolesville Extraterritorial Jurisdiction (ETJ); per the circa 2014 Annexation Agreement with the City of Raleigh, this property, if sought to Voluntarily Annex, would annex into Rolesville.

4. Does the application allow uses that are compatible with existing and allowed uses on surrounding land;

Presently, the surrounding land uses are larger lot single family detached residential pursuant to Wake County's R-30 low density zoning. These existing land uses are inconsistent with the Rolesville Comprehensive Plan, which covers this region. The application therefore complies with the Rolesville Comprehensive Plan, but is significantly different from the existing or currently allowed (under Wake County R-30 zoning) development pattern.

5. Would the application ensure efficient development within the Town, including the capacity and safety of the street network, public facilities, and other similar considerations;

The application, with an envisioned density in the range of 2.3 dwelling units per acre (270 units/116 acres) is actually lower than the prescribed residential density range of 3 to 5 dwelling units per acre. The application is demonstrating compliance with planned Community Transportation Plan roadways, as well as pedestrian features. The identified amenities are demonstrating general compliance with LDO amenity requirements.

- 6. Would the application result in a logical and orderly development pattern; and Based on the concept/sketch plan, the proposed development indicates intentions to meet or exceed all subdivision requirements, including roadway and pedestrian improvements. All LDO subdivision requirements will be demonstrated on the subsequent Major Preliminary Subdivision Plat.
- 7. Would the application result in adverse impacts on water, air, noise, storm water management, wildlife, vegetation, wetlands, and the natural functioning of the environment.

Based on the concept/sketch plan and the requirement to comply with all LDO minimum requirements to develop the project, beyond the inherent and permitted impacts of land development, the impacts as described should not result.

8. If a conditional rezoning, the BOC may also consider if the conditional rezoning addresses the impacts reasonably expected to be generated by the development or use of the site, can reasonably be implemented and enforced for the subject

property, and if it will mitigate specific issues that would likely result if the subject property were zoned to accommodate all the uses and the minimum standards of the corresponding general zoning district.

While submitted as requesting Conditional Zoning (CZ) of the general NC and RH districts, the proposal includes nothing that could not be developed via a General District Rezoning to the NC and RH Districts.

Overall Analysis

The project entails developing approximately 150 single family detached dwelling units with the RH Zoning District utilizing the LDO Section 3.1.3. standards of 7,500 SF minimum lots sizes with 75' of minimum lot width, and then developing up to 115 Single family Attached dwelling units (townhomes) across a portion of the RH District and then also within the NC District. The Applicant has shown the intention in the Concept/Sketch Plan (Attachment 5) and Exhibit 4 (Attachment 8) to subdivide Single family Attached (Townhome) lots both fronting on new/proposed public streets, but also utilizing private access easements for some units/buildings. The latter is the impetus for TA-23-05 and providing LDO regulations for the option to create Single family Attached lots that do not have lot frontage on public streets (right-of-ways), but rather private access easements. Staff generally supports the creation of such an option in the LDO, as it promotes more creative community development, it lessens the propensity of driveway entrances onto public streets, and it lessens the long-term maintenance responsibilities of new public streets by eliminating the need for public streets while still facilitating the development of fee simple (Townhome) lots. The NC District is rather small (~13 acres), making the minimum 15% of gross area for non-residential uses relatively small, but the inclusion of a neighborhood serving nonresidential use will be beneficial to the local community, and as demonstrated in the TIA trip calculations, can serve to reduce vehicle trips by containing some (of the non-residential caused trips) to stay within the project.

Staff Recommendation

REZ-23-03 – Map Amendment Rezoning Application

Staff finds the proposed rezoning request REZ-23-03 is consistent with the Comprehensive Plan and recommends Approval of it.

ANX 22-04 – Voluntary Annexation Petition

Staff finds the petition for Voluntary Annexation (see Attachment 13) to be complete. The Town Clerk has provided a Certificate of Sufficiency dated October 26, 2022 (see Attachment 14).

Consistency and Reasonableness

As noted above under the Comprehensive Plan section of this report, the rezoning request for the subject parcels is consistent with the Future Land Use designation of *Medium Density Residential*. The Concept Site Plan and proposed Conditions of Approval express general consistency with the Community Transportation Plan, Bicycle Plan, and Greenway Plan. Rezoning Map Amendment REZ-23-03 is thus consistent with the Comprehensive Plan and other applicable Plans and is therefore reasonable.

Proposed Motions

- 1. Motion to (approve or deny) Rezoning Map Amendment request REZ-23-03 Hills at Harris Creek.
- 2. (Following Approval) Motion to adopt a Plan Consistency Statement and Statement of Reasonableness for REZ-23-03.

3. Motion to (approve or deny) the Voluntary Annexation Petition received under G.S. 160A-31 for ANX 22-04 – Hills at Harris Creek.

Or

4. Motion to continue the legislative hearing for REZ-23-03 and ANX 22-04 to a future Town Board of Commissioners' meeting.

Attachments

1	Vicinity Map
2	Zoning Map
3	Future Land Use Map
4	Map Amendment Application
5	Concept/Sketch Plan (dated June 2, 2023)
6	Zoning District boundary sketch
7	Zoning District Metes & Bounds (Legal Descriptions)
8	Townhome Mews Sketch
9	Proposed Conditions of Approval
10	Duke Powerline Easement and Greenway Topic
11	Neighborhood Meeting Package – May 23, 2023
12	Traffic Impact Analysis (TIA) sealed dated 2023-06-23
13	Voluntary Annexation Petition and Attachments
14	Voluntary Annexation Town Clerk Certificate of Sufficiency – signed October 5, 2022

ATTACHMENT 1 - VICINITY MAP

Rolesville Gending Community - Capital Connection Ett. 1837

Case: ANX 22-04, REZ 23-03 Address: 0,0,3645 Rock Farm, 5333 Mitchell Mill Road PIN 1757750520, 1757758529, 1757761273, 1757778982 Date: 2023.07.18



ATTACHMENT 2 - EXISTING ZONING



S

ATTACHMENT 3 - FUTURE LAND USE MAP



Case: ANX 22-04, REZ 23-03 Address: 0,0,3645 Rock Farm, 5333 Mitchell Mill Road PIN 1757750520, 1757758529, 1757761273, 1757778982 Date: 2023.07.18





Contact Information

Property Owner Alan Watkins	
Address 3645 Rock Farm Road	City/State/Zip Wake Forest, NC 27587
Phone 919-824-6088	Email c/o: jason@ellisdevgroup.com

Developer Ellis Developments NC, LLC	
Contact Name Jason Pfister, VP of Development	
Address 6801 Falls of Neuse Road, Suite 108	City/State/Zip Raleigh NC 27615
Phone 919-824-6088	Email Jason@ellisdevgroup.com

Property Information

Address 5333 Mitchell Mill Road, Wake Forest, NC 27587 & O Mitchell Mill Road, Wake Forest, NC 27587

Wake County PIN(s) 175750520, 1757758529, 1757761273, 1757778982

Current Zoning District Wake R30	Requested Zoning District	NG-CD & RH-C	Ð	
Total Acreage 115.94 acres	_	NC-CZ	F	RH-CZ

Owner Signature

I hereby certify that the information contained herein is true and completed. I understand that if any item is found to be otherwise after evidentiary hearing before the Town Board of Commissioners, that the action of the Board may be igvalidated.

Signature	Con Watken	Date	3/2	12	023	5
-			7			

STATE OF NORTH CAROLINA

COUNTY OF Wake

I, a Notary Public, do hereby certify that Alan Watkins

personally app	eared before me this day and acknowled	lged the due execution of the fo	regoing instrument. This
the		day of march Print	20_23
My commissio	n expires <u>1/8/2028</u>	Notary Public Wake	
Signature		Seal Pitt CAROLINA	
/	Town of Roles	sville Planning	
/	PO Box 250 / Rolesville, North Carolina	27571 / RolesvilleNC.gov / 919	.554.6517



Date _____

Map Amendment Application

Contact Information

Property Owner Randy Watkins	
Address 145 Olde Liberty Drive	City/State/Zip Youngsville, NC 27596
Phone 919-824-6088	Email c/o: jason@ellisdevgroup.com

Developer Ellis Developments NC, LLC	
Contact Name Jason Pfister, VP of Development	
Address 6801 Falls of Neuse Road, Suite 108	City/State/Zip Raleigh NC 27615
Phone 919-824-6088	Email jason@ellisdevgroup.com

Property Information

Address 5333 Mitchell Mill Road, Wake Forest, NC 27587 & O Mitchell Mill Road, Wake Forest, NC 27587

Wake County PIN(s) 175750520, 1757758529, 1757761273, 1757778982

Current Zoning District Wake R30	Requested Zoning District	NC-CD-& RH-CD	
Total Acreage 115.94 acres		NC-CZ+	RM-CZ

Owner Signature

I hereby certify that the information contained herein is true and completed. I understand that if any item is found to be otherwise after evidentiary hearing before the Town Board of Commissioners, that the action of the Board may be ipvalidated.

Signature Ray Watt	Date <u>3/2/2023</u>
--------------------	----------------------

STATE OF NORTH CAROLINA	
I, a Notary Public, do hereby certify that Randy Watkins	
personally appeared before me this day and acknowle	adged the due execution of the foregoing instrument. This
the	day of March Printing 2023
My commission expires 1/8/2028	Notary Public Wake
Signature	- Seal NP
Town of Rolesville Plan hing	
PO Box 250 / Rolesville, North Carolina 27571 / RolesvilleNC.gov / 919.554.6517	



Date

Map Amendment Application

Contact Information

Property Owner Ellis Land Investment Company, LLC	
Address 6801 Falls of Neuse Road, Suite 108	City/State/Zip Raleigh NC 27615
Phone 919-824-6088	Email c/o: jason@ellisdevgroup.com

 Developer
 Ellis Developments NC, LLC

 Contact Name
 Jason Pfister, VP of Development

 Address
 6801 Falls of Neuse Road, Suite 108

 City/State/Zip
 Raleigh NC 27615

 Phone
 919-824-6088

 Email jason@ellisdevgroup.com

Property Information

Address 0 Mitchell Mill Road, Wake Forest, NC 27587	
Wake County PIN(s) 175750520, 1757758529, 1757761273	3, 1757778982
Current Zoning District Wake R30	Requested Zoning District-NC-CD & RH-CD N(-c2+
Total Acreage 115.94 acres	KHVEC

Owner Signature

I hereby certify that the information contained herein is t	rue and completed. I understand that if any item is	
found to be otherwise after evidentiary hearing before the	Pown Board of Commissioners, that the action of	the
Board may be invalidated		
Signature	Date	
STATE OF NORTH CAROLINA		
I, a Notary Public, do hereby certify that Stephen Ellis, Man	ager	
personally appeared before me this day and acknowled	lged the due execution of the foregoing instrument.	This
the 2nd	day of March 111111111111111111111111111111111111	
My commission expires 01/08/2028	INTEON C. WOMILL	
Signature	Seal County	
Town of Roles PO Box 250 / Rolesville, North Carolina	27571 / RolesvilleNO:gov / 919.554.6517	



Date _____

Map Amendment Application

Contact Information

Property Own <mark>er Laura Watkins</mark>	
Address 3544 Donlin Drive	City/State/Zip Wake Forest, NC 27587
Phone 919-824-6088	Email c/o: jason@ellisdevgroup.com

Developer Ellis Developments NC, LLC	
Contact Name Jason Pfister, VP of Development	
Address 6801 Falls of Neuse Road, Suite 108	City/State/Zip Raleigh NC 27615
Phone 919-824-6088	Email jason@ellisdevgroup.com

Property Information

Address 3645 Rock Farm Road, Wake Forest, NC 27587	
Wake County PIN(s) 175750520, 1757758529, 1757761273, 1	757778982
Current Zoning District Wake R30	_ Requested Zoning District <u>NC-CD & RH-CD</u>
Total Acreage 115.94 acres	NC-CZ+ RH-CZ

Owner Signature

I hereby certify that the information contained herein is true and completed. I understand that if any item is found to be otherwise after evidentiary hearing before the Town Board of Commissioners, that the action of the

Board may be invalidated	
Signature ANOTO	Date OIMar 2023

STATE OF NORTH CAROLINA	
I, a Notary Public, do hereby certify that Laura Watkins	
personally appeared before me this day and acknowled	Iged the stille execution of the foregoing instrument. This
the ISF	_ day of March 2023
My commission expires <u>1/8/2028</u>	- Wotary Public
Signature	Sear 77 CAROLING
Town of Rolesville Planning	
PO Box 250 / Rolesville, North Carolina 27571 / RolesvilleNC.gov / 919.554.6517	



Date _____

Map Amendment Application

Contact Information

Property Owner Randall Watkins	
Address 3544 Donlin Drive	City/State/Zip Wake Forest, NC 27587
Phone 919-824-6088	Email c/o: jason@ellisdevgroup.com

Developer Ellis Developments NC, LLC	
Contact Name	
Address 6801 Falls of Neuse Road, Suite 108	City/State/Zip Raleigh NC 27615
Phone 919-824-6088	Email jason@ellisdevgroup.com

Property Information

Address 3645 Rock Farm Road, Wake Forest, NC 27587	
Wake County PIN(s) 175750520, 1757758529, 1757761273, 17	57778982
Current Zoning District Wake R30	Requested Zoning District NC-CD & RH-CD
Total Acreage 115.94 acres	NC-CZ + RH-CZ

Owner Signature

I hereby certify that the information contained herein is true and completed. I understand that if any item is found to be otherwise after evidentiary hearing before the Town Board of Commissioners, that the action of the Board may be invalidated.

Board may be invalidated.	-11/00
Signature	Date <u>3/1/2</u> /
Topo	
STATE OF NORTH CAROLINA	
COUNTY OF Wake	
I, a Notary Public, do hereby certify that Randall Watkins	

personally app the	beared before me this day and	day of Mach C. Print 2023	This
My commissio	on expires 1/8/2028	Notary Public Wake County	
Signature	A	Seal 2 A Case OL MP	
1	Тс	own of Rolesville Planning munitive	
v	PO Box 250 / Rolesville, No	rth Carolina 27571 / RolesvilleNC.gov / 919.554.6517	



Metes and Bounds Description of Property

Will be completed upon completion of survey

·····



Rezoning Justification

See attached



Property Owner Information

Wake County PIN	Property Owner	Mailing Address	Zip Code
1757758529	Ellis Land Investment Company, LLC	6801 Falls of Neuse Road, Sulte 108, Raleigh, NC	27615
1757761273	Randall and Laura Watkins	3544 Donlin Drive, Wake Forest, NC	27587
1757750520	Alan and Randy Watkins	3609 Rock Farm Road, Wake Forest, NC	27587
1757778982	Alan and Randy Watkins	3609 Rock Farm Road, Wake Forest, NC	27587
		2	
· · · · · · · · · · · · · · · · · · ·			

The proposed rezoning of the parcels described above is made in careful consideration of the Rolesville Comprehensive Plan 2017. The Future Land Use Map designates these parcels as Medium Density Residential, and we believe that a combination of the Mixed Use Neighborhood Center Conditional District and Residential High Density Conditional District would allow for the best variety of housing types to meet this objective while integrating seamlessly with the surrounding community. The proposed community includes a mix of housing types (single-family detached homes and townhomes) and is within the density levels recommended by the Comprehensive Plan.

Mixed-use neighborhoods such as the one proposed herein are routinely applauded for their connectivity and walkability and the multiple housing types appeal to various income levels while still maintaining the appeal and quality of the neighborhood. The proposed rezoning is in accordance with the Comprehensive Plan and reasonable and in the public interest. We request your support for the proposed zoning.

ATTACHMENT 5 - CONCEPT/SKETCH PLAN REZ-23-03

SCALE

STRONGROCK



EXHIBIT ONE: CONCEPT PLAN (SKETCH PLAN)





Strong Rock Engineering Group, PLLC Raleigh, NC 27624 PO Box 99552 984.200.1932 (p) Company License # P-2166

EXHIBIT 3:

HILLS at HARRIS CREEK

V2 - REZ-23-03 rcvd 6-2-23

Metes and Bounds description of Parcel 1: NC-CZ

as shown on Exhibit 2: Zoning Diagram

Point of Beginning: Commencing at a point on the Future (post 35' Dedication) Northern Right-Of-Way of Mitchell Mill Road: THENCE, North 19°37'29" East, 153.43 feet to a point; THENCE, North 18°40'39" East, 227.11 feet to a point; THENCE, North 19°31'15" East, 230.80 feet to a point; THENCE, North 83°06'35" West, 376.14 feet to a point; THENCE, North 43°56'42" East, 539.17 feet to a point; THENCE, South 43°56'42" West, 539.17 feet; THENCE, South 16°32'05" East, 42.44 feet to a point; THENCE, South 27°36'38" East, 42.80 feet to a point; THENCE, South 68°58'38" East, 54.77 feet to a point; THENCE, South 77°04'07" East, 46.50 feet to a point; THENCE, South 82°03'47" East, 43.34 feet to a point; THENCE, South 63°10'34" East, 53.99 feet to a point; THENCE, South 78°58'51" East, 55.03 feet to a point; THENCE, South 73°28'18" East, 39.46 feet to a point; THENCE, South 75°12'22" East, 64.19 feet to a point; THENCE, South 79°08'05" East, 30.68 feet to a point; THENCE, South 64°08'20" East, 56.08 feet to a point; THENCE, South 0°08'08" East, 955.98 feet to the beginning of a non-tangent curve concave southerly, said curve has a radius of 13,079.66 feet, to which a radial line bears North 14°33'02" East;

THENCE, westerly along said curve through a central angle of 2°57'39" an arc distance of 675.93 feet to the point of beginning;

Containing 586,556.07 square feet / 13.47 acres, more or less.



Hills at Mitchell Mill

Metes and Bounds description of Parcel 2: RH-CZ

as shown on Exhibit 2: Zoning Diagram

Point of Beginning: Commencing at a point on the Future (post 35' Dedication) Northern Right-Of-Way of Mitchell Mill Road: THENCE (1) North 0°08'08" West, 955.98 feet to a point; THENCE (2) North 64°08'20" West, 56.08 feet to a point; THENCE (3) North 79°08'05" West, 30.68 feet to a point; THENCE (4) North 75°12'22" West, 64.19 feet to a point; THENCE (5) North 73°28'18" West, 39.46 feet to a point; THENCE (6) North 78°58'51" West, 55.03 feet to a point; THENCE (7) North 63°10'34" West, 53.99 feet to a point; THENCE (8) North 82°03'47" West, 43.34 feet to a point; THENCE (9) North 77°04'07" West, 46.50 feet to a point; THENCE (10) North 68°58'38" West, 54.77 feet to a point; THENCE (11) North 27°36'38" West, 42.80 feet to a point; THENCE (12) North 16°32'05" West, 42.44 feet to a point; THENCE (13) North 43°56'42" East, 651.84 feet to a point; THENCE (14) North 3°44'09" West, 2,963.58 feet to a point; THENCE (15) South 88°59'15" East, 0.54 feet to a point; THENCE (16) North 75°53'40" East, 340.47 feet to a point; THENCE (17) North 65°27'07" East, 350.10 feet to a point; THENCE (18) South 40°38'56" East, 133.25 feet to a point; THENCE (19) South 80°06'11" East, 62.70 feet to a point; THENCE (20) South 4°46'37" East, 4,426.21 feet to a point; THENCE (21) South 24°08'12" East, 83.90 feet to a point; THENCE (22) South 0°25'29" East, 493.75 feet to a point; THENCE (23) North 71°34'19" West, 3.27 feet to a point; THENCE (24) North 74°14'58" West, 53.84 feet to a point; THENCE (25) North 75°50'32" West, 260.45 feet to a point; THENCE (26) North 75°16'13" West, 150.67 feet to the beginning of a non-tangent curve concave southerly, said curve has a radius of 13,079.66 feet, to which a radial line bears North 17°04'26" East; THENCE (27) westerly along said curve through a central angle of 2°31'24" an arc distance of 576.04 feet;

Containing 4,381,947.13 square feet / 100.60 acres, more or less.



Exhibit FIVE

Hills at Harris Creek (REZ-23-03)

Neighborhood Center Conditional Zoning District (NC-CZ) and Residential High Density Conditional (RH-CZ) Zoning District **Conditions of Approval**

Conditions Applicable to the entire property:

- 1. The subject property shall be developed generally in accordance with the concept plan attached hereto as Exhibit 1 and incorporated herein as if fully set out. The approximately 13-acre portion of the subject property, further described as Parcel 1 on the attached Exhibit 2 attached hereto, shall be zoned NC-CZ and the remaining 102-acre portion of the property, further described as Parcel 2 on Exhibit 2 attached hereto, shall be zoned RH-CZ. The improvements described herein may be developed in phases in accordance with a phasing plan approved by the Town of Rolesville.
- 2. <u>Dwelling Units</u>: The total number of dwellings on the subject property shall not exceed 270 dwelling units and no more than 115 of these dwelling units shall be permitted to be Dwellings, Single Family, Attached (Townhouse.)
- 3. No more than fifteen (15) gross acres of the RH-CZ zoned district shall be assigned to use for Dwellings, Single Family, Attached (Townhouse).
- 4. <u>Affordable Housing</u>: Prior to the issuance of the first building permit for a dwelling unit, the property owner shall donate Twenty Thousand Dollars and No Cents (\$20,000.00} to Homes for Heroes or another non-profit organization with a substantially similar purpose statement.
- 5. <u>Pollinator Plantings</u>: At least three acres of the landscaping planted within the power line easement on the subject property shall utilize plant materials that are listed as Native Pollinator Plants on North Carolina Wildlife Federation ("NCWF") or other resources for native plants recommended by the NCWF. Where evergreen plantings or street trees are required by the Rolesville Land Development Ordinance as the same may be amended from time to time, pollinator plantings shall not be required. Nothing herein shall be construed to limit the plant materials permitted on individual residential lots.
- 6. <u>Recreational Amenities</u>: The following recreational amenities shall be provided generally as shown on the attached Exhibit 1 as a part of the development of the subject property and dedicated to the community's homeowner's association except for those areas offered to and accepted by the Town of Rolesville:
 - a. A swimming pool and cabana, including changing rooms and restrooms shall be constructed prior to the issuance of the 150th building permit for a dwelling unit;
 - b. At least one fenced playground shall be constructed prior to the issuance of the 150th building permit for a dwelling unit;
 - c. At least one fenced dog park shall be constructed prior to the issuance of the 150th building permit for a dwelling unit;

- d. At least one (1) garden park shall be provided prior to the issuance of the 200th building permit for a dwelling unit.
- e. At least one (1) pickle ball court shall be provided prior to issuance of the 250th building permit for a dwelling unit; and
- 7. <u>Transportation Improvements</u>: The property owner shall install all required roadway and transportation improvements set forth in the Traffic Impact Analysis report associated with this project in order to address the transportation impacts reasonably expected to be generated by the development. All transportation improvements shall be installed in accordance with future phasing plans approved by the Town.
- 8. All homes shall include either crawl space foundations or stem wall foundation . Any stem wall foundations shall have an average of at least eighteen inches {18") in height across the front facade of the home and shall have brick or stone veneer on all sides facing a public street.
- 9. The minimum gross building square footage shall be 2,000 square feet for Single Family Detached dwellings.
- 10. Prior to the issuance of a building permit for the 150th dwelling unit, the property owner shall contribute Twenty-Five Thousand Dollars (\$25,000.00) to the Town of Rolesville to be used by the Town of Rolesville to install a traffic light at the intersection of the US 401 Bypass and Jonesville Road.

Conditions Applicable to Dwellings, Single Family, Attached (Townhouse) use only (In Both NC & RH Districts):

- 11. The minimum gross building square footage for townhomes shall be 1,500 square feet and include at least a two (2) car garage.
- 12. No Dwelling, Single Family, Attached (Townhouse) building shall exceed three (3) stories.

Conditions Applicable to the NC-CZ District only:

13. All uses permitted in the Neighborhood Center Conditional Zoning (NC-CZ) district shall be permitted except *Dwellings, Multiple Family* (ie apartments).

ATTACHMENT 10 - DUKE Powerline/Greenway within Easement Topic

From:	Pait, Bruce E.
To:	Jason Pfister
Cc:	keith@strongrockgroup.com
Subject:	Duke Energy Q AND A Vinci WO 49291749
Date:	Saturday, May 27, 2023 8:04:14 AM
Attachments:	image001.png
	comment parks v1 rez-23-03 concept plan-markup.pdf
	DE MYWORLD DEPICTION.pdf
	<u>1 A UGET GDLF-STD-TRM-00004 - 5-2020.pdf</u>
	PICTURE - EQUIPMENT WORKING H 1.pdf

Proposed improvements and development within or adjacent to Duke Energy's 180 foot wide transmission level power easement.

Line: OL294 Durham – Wake 500kV at Towers # 100A to # 104A

To: Jason Pfister (919-824-6088) VP – Development - Ellis Development Group | 6801 Falls of Neuse Rd, STE 108, Raleigh, NC 27615

Cc: Stephen Ellis – Ellis Development Group, Keith Spalding-Robbins – Strong Rock Group Dear Jason:

-1. Duke Energy highly discourages a parallel paved trail at this location. Duke Energy has plans to replace the large lattice towers and parallel trails at this location will be in the way of heavy equipment and could likely be damaged. See the "Equipment working" picture attached above.

-2. Duke Energy power easements are not compatible with dumping grounds and abandoned or junk vehicles. The junk car on the site should be removed as soon as possible by the property owners. These types of encroachments represent hazards to Duke Energy workers, property owners and the general public.

-3. Easement documents:

We have not performed a title search for the property and only a full title search to the beginning of the records would capture all encumbrances on the property. The attached easement was referenced in our maintenance records for the property's location. We are providing this as a courtesy but make no guarantees as to any easement rights on the property, including rights for corridors that are currently unoccupied. We strongly suggest that you engage a licensed attorney to complete a full title search on the property to confirm any rights that Duke Energy has on the property.

The maintenance records that I have seen include but may not be limited to the following easements: Book 2830 page 24, Book 2878 page 411 of the Wake County Registry.

I hope this information is beneficial to you and your team.

Sincerely,

Bruce 919-219-9567

Bruce E Pait



Sr. Asset Protection Program Manager Transmission Department Carolinas East & Carolinas West - Zone 7 4690 Simms Creek Road | MC: US 1N T&D OPS | Raleigh, NC 27616 Work: 919.431.4831 | Cell: 919.219.9567 Bruce.Pait@duke-energy.com

For more information about Transmission Asset Protection: Trees & Rights of Way @ Duke-Energy.com





PIN_NUM OWNER	ADDR1	ADDR2	ZIP
1757675786 CARLE, SCOTT CARLE, THERESA	PO BOX 371	WAKE FOREST NC 27588-0371	0
1757669828 GRO PEG PROPERTIES LLC	481 AIRPORT RD	LOUISBURG NC 27549-6806	27587
1757771603 GRO PEG PROPERTIES LLC	481 AIRPORT RD	LOUISBURG NC 27549-6806	27587
1757684697 GRO PEG PROPERTIES LLC	481 AIRPORT RD	LOUISBURG NC 27549-6806	27587
1757664956 UNDERHILL, GROVER ARCHIE JR	5229 MITCHELL MILL RD	WAKE FOREST NC 27587-7249	0
1757694902 JONES, CHARLES E	3816 JONESVILLE RD	WAKE FOREST NC 27587-8180	27587
1757974973 JONES PROPERTIES, LLC	3905 MANLY FARM RD	WAKE FOREST NC 27587-8494	27587
1757758529 ELLIS LAND INVESTMENT COMPANY, LLC	6801 FALLS OF NEUSE RD STE 108	RALEIGH NC 27615-5387	27587
1757641415 GOODNIGHT, CECIL L GOODNIGHT, JUDY J	1201 ROLESVILLE RD	WAKE FOREST NC 27587-6957	27587
1757956528 HOSS FARM LLC	3920 MARLOWE ST	HOUSTON TX 77005-2046	27587
1757944104 JONES, CHARLES SPENCER	3905 MANLY FARM RD	WAKE FOREST NC 27587-8494	27587
1757856303 JONES, CHARLES SPENCER JONES, SHARON	3905 MANLY FARM RD	WAKE FOREST NC 27587-8494	27587
1757950297 MILLS, JOSEPHUS DANIELS JR	5517 MITCHELL MILL RD	WAKE FOREST NC 27587-7254	0
1757645809 UNITY THREE BUILDERS INC	104 S MAIN ST STE A	ROLESVILLE NC 27571-9657	0
1757778982 WATKINS, ALAN WATKINS, RANDY	3609 ROCK FARM RD	WAKE FOREST NC 27587-6872	27587
1758802673 DEROUSSE, TERRANCE	5005 UPCHURCH LN	WAKE FOREST NC 27587-6374	27587
1758806894 DEROUSSE, TERRANCE	5005 UPCHURCH LN	WAKE FOREST NC 27587-6374	27587
GUDURU, KAVITHA EQUITY TRUST COMPANY FBO			
1758604688 SRIVATSA SRINATH	849 RIVER SONG PL	CARY NC 27519-0882	27587
1757856755 JONES PROPERTIES LLC	3905 MANLY FARM RD	WAKE FOREST NC 27587-8494	27587
1757859764 VASQUEZ, SUSIE A	3532 MANLY FARM RD	WAKE FOREST NC 27587-9664	27587
1757665301 UNDERHILL, GROVER ARCHIE JR	5229 MITCHELL MILL RD	WAKE FOREST NC 27587-7249	27587
1757857937 JONES, CHARLES SPENCER	3905 MANLY FARM RD	WAKE FOREST NC 27587-8494	27587
1757868104 ANDERSON, BOBBY G	3605 MANLY FARM RD	WAKE FOREST NC 27587-8489	27587
1757866385 LANGWORTHY, ROBERT JOSEPH	3607 MANLY FARM RD	WAKE FOREST NC 27587-8489	27587
1757962198 BATSTONE, JEREMY C	3608 MANLY FARM RD	WAKE FOREST NC 27587-8488	27587
1757866660 GOLD, SHARIE	3609 MANLY FARM RD	WAKE FOREST NC 27587-8489	27587
1757657746 WATKINS, ALAN DWAIN	3609 ROCK FARM RD	WAKE FOREST NC 27587-6872	27587
1757668628 UNDERHILL, GROVER ARCHIE JR	5229 MITCHELL MILL RD	WAKE FOREST NC 27587-7249	27587
1757658917 NEWELL, RONALD W NEWELL, MARIE J	3617 ROCK FARM RD	WAKE FOREST NC 27587-6872	27587
1757770001 FERLITO, CHRISTOPHER J MORRIS, SARAH L	3620 GRO PEG LN	WAKE FOREST NC 27587	27587
1757772106 FERLITO, CHRISTOPHER J MORRIS, SARAH L	3620 GRO PEG LN	WAKE FOREST NC 27587	27587
1757963564 NICHOLAS, ALLAN W NICHOLAS, JUDY T	3628 MANLY FARM RD	WAKE FOREST NC 27587-8488	27587

1757571035 FOWLER, JAMES ROBERT III BRIGHT, JILL F
1758706961 QUINTO, JEFFERY L QUINTO, BARBARA B
1758709579 DAVIS, WILLIAM C DAVIS, KAREN M
1758707313 FORRESTER, JENNIFER N FORRESTER, HAYES G
1758704715 ASSOCIATE PROPERTIES LLC
1758805145 JONES PROPERTIES LLC
1757893647 RGA CONSULTING LLC
1757897740 RGA CONSULTING LLC
1757893394 JONES, CHARLES SPENCER
1757894101 JONES, CHARLES SPENCER
1757897244 MCKNIGHT, H BRENT JR MCKNIGHT, ASHLEY E H
1757790646 CARLE, SCOTT CARLE, THERESA
1757887850 MCLAMB, REBECCA R
1757696434 CARLE, SCOTT CARLE, THERESA
1757883981 JONES, CHARLES SPENCE
1757781960 CARLE, SCOTT CARLE, THERESA
1757884608 JONES, CHARLES SPENCE
1757884420 JONES PROPERTIES LLC
1757887363 CORTES, FREDY MORENO
1757884137 JONES PROPERTIES LLC
1757780258 CARLE, SCOTT CARLE, THERESA
1757874938 ADAMS, BENJAMIN ADAMS, WHITNEY
1757874787 LEFRANCOIS, STEPHEN D
1757878878 JONES PROPERTIES LLC
1757875349 BEATTIE, LENORA M
1757972579 JONES PROPERTIES LLC
1757879378 LEFRANCOIS, THOMAS D LEFRANCOIS, JANET R
1757973384 LEFRANCOIS, THOMAS D LEFRANCOIS, JANET R
1757876015 LEFRANCOIS, MICHAEL L LEFRANCOIS, TONIA
1757973137 LEFRANCOIS, THOMAS D LEFRANCOIS, JANET R
1757973045 LEFRANCOIS, THOMAS D LEFRANCOIS, JANET R
1757866852 LEFRANCOIS CONSTRUCTION CO INC
1757963868 LEFRANCOIS CONSTRUCTION CO INC
1757761273 WATKINS, RANDALL WATKINS, LAURA

3737 MANLY FARM RD 3720 MANLY FARM RD 3720 MANLY FARM RD 3716 MANLY FARM RD 3716 MANLY FARM RD 5005 UPCHURCH LN 4933 UPCHURCH LN 4932 UPCHURCH LN 4929 UPCHURCH LN 5025 UPCHURCH LN **3905 MANLY FARM RD** 3904 MANLY FARM RD 3904 MANLY FARM RD 3905 MANLY FARM RD 3904 MANLY FARM RD PO BOX 371 3816 MANLY FARM RD PO BOX 371 3905 MANLY FARM RD 3905 MANLY FARM RD 3905 MANLY FARM RD 5103 WETLANDS DR 3905 MANLY FARM RD PO BOX 371 3741 MANLY FARM RD 3905 MANLY FARM RD 3733 MANLY FARM RD 3905 MANLY FARM RD 3717 MANLY FARM RD **3720 MANLY FARM RD** 3716 MANLY FARM RD 3544 DONLIN DR 7400 FOWLER RD 3905 MANLY FARM RD PO BOX 371

27587	ZEBULON NC 27597-8318
27587	WAKE FOREST NC 27587-6374
27587	WAKE FOREST NC 27587-6385
27587	WAKE FOREST NC 27587-6385
27587	WAKE FOREST NC 27587-6385
27587	WAKE FOREST NC 27587-6374
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-8493
27587	WAKE FOREST NC 27587-8493
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-8493
27587	WAKE FOREST NC 27588-0371
27587	WAKE FOREST NC 27587-8491
27587	WAKE FOREST NC 27588-0371
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27588-0371
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-8494
27587	RALEIGH NC 27610-1564
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27588-0371
27587	WAKE FOREST NC 27587-8490
27587	WAKE FOREST NC 27587-8490
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-8490
27587	WAKE FOREST NC 27587-8494
27587	WAKE FOREST NC 27587-9665
27587	WAKE FOREST NC 27587-9665
27587	WAKE FOREST NC 27587-8490
27587	WAKE FOREST NC 27587-9665
27587	WAKE FOREST NC 27587-3521

1757650296 GOODNIGHT, CECIL L GOODNIGHT, JUDY J
1757660324 MT CALVARY HOLINESS CHURCH OF WAKE CO INC
1757653216 PARTIN, BETTY H
1757653765 UNDERHILL, G ARCHIE JR UNDERHILL, TERESSA C
1757654381 PARTIN, BETTY H
1757654161 TIGNOR, GLENN ALLEN TIGNOR, FRANCES P
1757656586 PREDDY, GENADIUS MAC PREDDY, MATTIE F
1757656231 UNITY THREE BUILDERS INC
1757646703 UNITY THREE BUILDERS INC
1757657280 SUTTON, WILLIE RAY SUTTON, PAMELA Z
1757738451 SOUTHVIEW INVESTMENT LLC
1757750520 WATKINS, ALAN WATKINS, RANDY
1757754163 HAMPTON, MELODY L

1757948723 WATKINS, REX I WATKINS, ELLA MAE

1757943559 JONES, CHARLES SPENCER JONES, SHARON K

1757946408 WHITE, AARON ANTHONY WHITE, KRISTY IVORY

5608 FIXIT SHOP RD

12695 SW 84TH AVENUE RD

1757859243 MILLS, JOSEPHEUS DANIELS

1757849656 JONES, CHARLES SPENCER

3905 MANLY FARM RD **5400 MITCHELL MILL RD** 3609 ROCK FARM RD 5320 MITCHELL MILL RD 5304 MITCHELL MILL RD **3921 JONESVILLE RD** 3905 MANLY FARM RD STE 802 5305 MITCHELL MILL RD **5229 MITCHELL MILL RD** 5517 MITCHELL MILL RD 10030 GREEN LEVEL CHURCH RD 104 S MAIN ST STE A 104 S MAIN ST STE A 5300 MITCHELL MILL RD 5300 MITCHELL MILL RD **1201 ROLESVILLE RD**

WAKE FOREST NC 27587-9632	MIAMI FL 33156-5803	WAKE FOREST NC 27587-8494	WAKE FOREST NC 27587-7254	WAKE FOREST NC 27587-8494	WAKE FOREST NC 27587-7252	WAKE FOREST NC 27587-6872	CARY NC 27519-8195	WARE FUREST NC 27587-7250	ROLESVILLE NC 27571-9657	ROLESVILLE NC 27571-9657	WAKE FOREST NC 27587-7251	WAKE FOREST NC 27587-7250	WAKE FOREST NC 27587-7250	WAKE FOREST NC 27587-7249	WAKE FOREST NC 27587-7250	WAKE FOREST NC 27587-8183	
27587	27587	27587	27587	27587	27587	27587	27587	18617	27587	27587	27587	27587	27587	27587	27587	27587	



6801 Falls of Neuse Road, Suite 108 Raleigh, NC 27615

Commercial ste Wildflow Beadow Wildflow Beadow Or Headow Or Headow

Notification of Neighborhood Meeting for Pending Annexation/ Rezoning

You have received this notice because you own property within 500 feet of a property for which an application to rezone or subdivide property has been filed. This notice is to inform you of an upcoming information meeting on this proposed annexation and rezoning.

Dear Property Owner:

Please be advised that a formal application has been submitted to amend the zoning classification for four properties (Wake County Property Identification Numbers 1757750520, 1757758529, 1757761273, and 1757778982) located at 5326 Mitchell Mill Road. The project consists of approximately 115.94 acres which is currently zoned R-30: Residential under the jurisdiction of Wake County. These applications are being presented by Ellis Developments Group on behalf of the property owners.

Ellis Developments Group has applied to rezone the parcels to a combination of NC-CZ (Neighborhood Center Conditional Zoning) and RH-CZ (Residential High Density Conditional Zoning) under the Town of Rolesville's Land Development Ordinance to allow for the construction of a mixed-use residential development featuring a mix of detached homes and townhomes, along with commercial outparcels along Mitchell Mill Road. We believe the proposed rezoning at this location is consistent with the Town of Rolesville Future Land Use Map which calls for Medium Density Residential in this area. In compliance with the requirements of the Town of Rolesville's Land Development Ordinance Code, this Neighborhood Meeting will be held to provide you with an opportunity to review a conceptual plan for the project and to give you an opportunity to ask any questions you may have about the project.

The Neighborhood Meeting will be held on Tuesday, May 23, 2023 from 7:00-8:00 p.m. at the Rolesville Community Center located at 514 Southtown Circle, Rolesville, NC 27571. Should you have questions prior to the meeting, please feel free to contact me via telephone at 919-824-6088 or email at jason@ellisdevgroup.com.

Sincerely

Jason Plister Vice President of Development

COMMUNITY MEETING SIGN-IN SHEET							
Project:	Hills at Harris Creek	Meeting Date:	May 23, 2023				
Applicant:	Ellis Developments Group	Location:	Rolesville Community Center				

Name	Address	Phone	E-Mail
BARMA	4932 OPCNEUCH	26919 266	BBQUINTO 2 YMALL
Ant	WAKO JOROS	9782	
PUNIO	the totes	1100119	bttoartist and look
ΑΛ.	5300 mitchell	11919 067	eripa integritar. or
BAurtin	1) ake Forest	6392	
55.0	3737 Marcy Frank	4 919-369	Stere LEFTARCOS Stroom
STEVE		1917	54 gruan l. com
LEFrancors	Wake Forest		
[nike	3717 many Form Koc	911	Miles Le Francos C
LeFrancois	Walke torst	BL2-GITI	Bull south not


Hills at Harris Creek: Neighborhood Meeting May 23, 2023 Rolesville Community Center



About Us



Who are We?

Ellis Developments Group – Land Acquisition and Development Company

Where are We?

- Headquarters in Raleigh with an office in Charlotte
- Projects throughout the Carolinas

What is our Role?

- Develop and deliver high-quality residential development projects that meet demand for growth
- Foster relationships with landowners, municipalities, and community members to develop projects that enhance communities Develop and deliver high quality residential development projects that enhance communities and meet demand for growth

Proposed Rezoning	PROPOSED CHANGES	Zoning Change to Neighborhood Center- Conditional Zoning and Residential High Density Conditional Zoning		Proposed construction of 152 single family homes and 115 townhomes (2.3 units/acre) Approximately 2-acre commercial site (proposed day care facility)		
	CURRENT STATUS	Rural residential land	Acreage: 115.94 acres	Current Zoning: Wake County R-30 (allows approximately 75 lots)		

Dronorad Razoning



Site Plan





Updates Since Prior Meeting

Reduced units from 318 homes to 267 homes

Minimum lot size increased from 5000 sf to 7500 sf

Improved open space with pollinator garden, bike paths and dog park

Greenway trail integrated into wetland areas

Reduced intensity of commercial parcel



External Road Improvements

A. Right In/Right Out only Entrance.

B. Former Rock Farm Road parcels with Separate Drives

- C. Three Lanes total across site.
- D. Left Turn Lane to Main Entrance
- E. Improvements include dedication of new ROW
- F. New Right Turn and lane with wider turn for Manly Farm Road





Manly Road Improvements

- A. New Right Turn Lane In from East
- B. New Turn Radius at Corners
- C. Passing Lane
- D. New Right Turn Out on Mitchell Mill
- E. Three Dedicated Lanes
- F. All Improvements within Existing ROW



Comprehensive Plan 2017





- **Consistent with Comprehensive Plan**
- Future Land Use Map designates these parcels at Medium Density Residential (3-5 units/acre)
- Consistent with residential character of adjacent properties





39

Land Use & Zoning



COMMUNITY POOL AND CABANA





DOG PARK





GREENWAY TRAILS





POLLINATOR/WILDFLOWER GARDEN





LANDSCAPED BOULEVARD STREETSCAPES







PICKLEBALL COURT



INTEGRATED BIKE PATHS





Commercial Parcel





Site Plan



Hills at Harris Creek Neighborhood Meeting Report

Ellis Developments Group (EDG) hosted an informational meeting for the proposed project on May 23, 2023 at the Rolesville Community Center. Five neighboring residents attended the meeting (attendance sheet attached). During the meeting, EDG presented an overview of the proposed project and provided the attendees with an opportunity to ask questions and express any concerns about the project. More specifically, the following topics were discussed in detail:

- Process and timeline
 - Overview of the rezoning process and upcoming hearings, as well as overall anticipated construction timeline.
- Overview of project
 - Unit mix, overall goal for neighborhood look and feel
 - 116 acres; 115 THs; 152 SF homes
 - Density: 2.3 units/acre
 - Described amenities green space design/walkability of community
 - Commercial: tentative plans for childcare center
- Comparison to prior version of site plan and need for second neighborhood meeting
 - Reduced units from 318 homes to 267 homes
 - Minimum lot size increased from 5000 sf to 7500 sf
 - Improved open space with pollinator garden, bike paths and dog park
 - Greenway trail integrated into wetland areas
 - Reduced intensity of commercial parcel
- Traffic
 - EDG responsible for all traffic improvements called for in the TIA
 - Widen Mitchell Mill Road for entire frontage of site
 - Left turn lanes into development from Mitchell Mill
 - Right in, right out at each entrance with tapered lane to help residents of Manly Farm Road
 - Measures to control traffic including traffic calming boulevard entrance

RAMEY KEMP ASSOCIATES

TOGETHER WE ARE LIMITLESS



Hills at Harris Creek Traffic Impact Analysis Rolesville, North Carolina



rameykemp.com

TRAFFIC IMPACT ANALYSIS

FOR

HILLS AT HARIS CREEK

LOCATED

IN

ROLESVILLE, NORTH CAROLINA

Prepared For: Town of Rolesville 502 Southtown Circle Rolesville, NC 27571

Prepared By: Infrastructure Consulting Services, Inc. *dba* **Ramey Kemp Associates** 5808 Faringdon Place Raleigh, NC 27609 License #F-1489



JUNE 2023

Prepared By: DAR

RKA Project No. 20498 - 005

Reviewed By: <u>JAE</u>

TRAFFIC IMPACT ANALYSIS HILLS AT HARRIS CREEK ROLESVILLE, NORTH CAROLINA

EXECUTIVE SUMMARY

1. Development Overview

A Traffic Impact Analysis (TIA) was conducted for the proposed Hills at Harris Creek development in accordance with the Town of Rolesville (Town) Land Development Ordinance (LDO) and North Carolina Department of Transportation (NCDOT) capacity analysis guidelines. The proposed development is to be located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane in Rolesville, North Carolina. This TIA is an update to the approved TIA that was sealed on May 19, 2022. This updated study reflects changes to the proposed land use types and densities and a change to the site access at the westernmost driveway from full-movement to right-in/right-out. No other scope changes were made from the previous TIA.

The proposed development is anticipated to be completed in 2027 and is expected to consist of 155 single-family homes, 115 townhomes, and a 12,000 sq. ft. day care center. Site access is proposed via one (1) full-movement and one (1) right-in/right-out (RIRO) driveway connection along Mitchell Mill Road.

The study analyzes traffic conditions during the weekday AM and PM peak hours for the following scenarios:

- 2022 Existing Traffic Conditions
- 2027 No-Build Traffic Conditions
- 2027 Build Traffic Conditions



2. Existing Traffic Conditions

The study area for the TIA was determined through coordination with NCDOT and the Town and consists of the following existing intersections:

- US 401 Bypass and Jonesville Road
- US 401 Bypass and Eastern U-Turn Location
- Mitchell Mill Road and Jonesville Road / Peebles Road

Existing peak hour traffic volumes were determined based on traffic counts conducted at the study intersection listed above, in November of 2021 during typical weekday AM (7:00 AM – 9:00 AM) and PM (4:00 PM – 6:00 PM) peak periods, while schools were in session for in-person learning.

Previously collected counts from the year 2021 were projected to the 2022 existing analysis year using a compounded annual growth rate of 2%. Weekday AM and PM traffic volumes were balanced between study intersections, where appropriate.

3. Site Trip Generation

The proposed development is assumed to consist of 155 single-family homes, 115 townhomes, and a 12,000 sq. ft. day care center. Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 11th Edition. Table E-1 provides a summary of the trip generation potential for the site.



Land Use (ITE Code)	Intensity	Daily Traffic (vpd)	Weekday AM Peak Hour Trips (vph)			Weekday PM Peak Hour Trips (vph)		
			Enter	Exit	Total	Enter	Exit	Total
Single-Family Home (210)	155 DU	1,510	28	83	111	95	55	150
Single-Family Attached Housing (220)	115 DU	812	14	45	59	44	26	70
Day Care Center (565)	12 KSF	572	70	62	132	63	70	133
Total Trips		2,894	112	190	302	202	151	353
Pass-By Trips: Day Care Center (44% PM)			-	-	-	-30	-30	-60
Total Primary Trips			112	190	302	172	121	293

Table E-1: Site Trip Generation

4. Future Traffic Conditions

Through coordination with NCDOT and the Town, it was determined that an annual growth rate of 2% would be used to generate 2027 projected weekday AM and PM peak hour traffic volumes. The following adjacent developments were identified to be considered under future conditions:

- Cobblestone Crossing Mixed-Use
- Young Street PUD
- Wheeler Tract
- Louisbury Road Assemblage
- Kalas / Watkins Family Property

5. Capacity Analysis Summary

The analysis considered weekday AM and PM peak hour traffic for 2022 existing, 2027 no-build, and 2027 build conditions. Refer to Section 7 of the TIA for the capacity analysis summary performed at each study intersection.



6. Recommendations

Based on the findings of this study, specific geometric and traffic control improvements have been identified at study intersections. The improvements are summarized below and are illustrated in Figure E-1.

Recommended Improvements by Developer

Required Frontage Improvements per Rolesville Community Transportation Plan

• Widen one-half section of Mitchell Mill Road along the site frontage to this roadway's ultimate section (4-lane median divided).

US 401 Bypass and Jonesville Road

• Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.

US 401 Bypass and Eastern U-Turn Location

• Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.

Mitchell Mill Road and Jonesville Road / Peebles Road

- Construct a southbound (Jonesville Road) left-turn lane with at least 100 feet of storage and appropriate decel and taper.
 - It should be noted that this improvement was also identified by the 5109 Mitchell Mill Road TIA.
- Construct a westbound (Mitchell Mill Road) right-turn lane with at least 100 feet of storage and appropriate decel and taper.
- Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.



Mitchell Mill Road and Site Access 1

- Construct the southbound approach (Site Access 1) as a right-in/right-out with one ingress lane and one egress lane.
- Provide stop-control for the southbound approach (Site Access 1).
- Construct a concrete median on Mitchell Mill Road that restricts access to rightin/right-out.

Mitchell Mill Road and Site Access 2

- Construct the southbound approach (Site Access 2) with one ingress lane and one egress lane.
- Provide stop-control for the southbound approach (Site Access 2).
- Construct an eastbound (Mitchell Mill Road) left-turn lane with at least 150 feet of storage and appropriate decel and taper.
- Construct a westbound (Mitchell Mill Road) right-turn lane with at least 100 feet of storage and appropriate decel and taper.





TABLE OF CONTENTS

1. INTRODUCTION1					
1.1.Site Location and Study Area 2					
1.2.Proposed Land Use and Site Access 2					
1.3.Adjacent Land Uses 2					
1.4.Existing Roadways 3					
2. 2022 EXISTING PEAK HOUR CONDITIONS7					
2.1.2022 Existing Peak Hour Traffic Volumes					
2.2.Analysis of 2022 Existing Peak Hour Traffic Conditions					
3. 2027 NO-BUILD PEAK HOUR CONDITIONS					
3.1.Ambient Traffic Growth					
3.2.Adjacent Development Traffic					
3.3.Future Roadway Improvements10					
3.4.2027 No-Build Peak Hour Traffic Volumes11					
3.5. Analysis of 2027 No-Build Peak Hour Traffic Conditions					
4. SITE TRIP GENERATION AND DISTRIBUTION					
4.1.Trip Generation15					
4.2.Site Trip Distribution and Assignment16					
5. 2027 BUILD TRAFFIC CONDITIONS					
5.1.2027 Build Peak Hour Traffic Volumes24					
5.2. Analysis of 2027 Build Peak Hour Traffic Conditions24					
6. TRAFFIC ANALYSIS PROCEDURE					
6.1.Adjustments to Analysis Guidelines26					
7. CAPACITY ANALYSIS 27					
7.1.US 401 Bypass and Jonesville Road27					
7.2.US 401 Bypass and Eastern U-Turn Location					
7.3.Mitchell Mill Road and Jonesville Road / Peebles Road					
7.4.Mitchell Mill Road and Site Access 135					
7.5.Mitchell Mill Road and Site Access 2					
8. CONCLUSIONS					
9. RECOMMENDATIONS 39					



LIST OF FIGURES

Figure 1 – Site Location Map	4
Figure 2 – Preliminary Site Plan	5
Figure 3 – Existing Lane Configurations	6
Figure 4 – 2022 Existing Peak Hour Traffic	8
Figure 5 – 2027 Projected Peak Hour Traffic	12
Figure 6 – Adjacent Development Trips	13
Figure 7 – 2027 No-Build Peak Hour Traffic	14
Figure 8a – Residential Site Trip Distribution	17
Figure 8b – Day Care Center Site Trip Distribution	18
Figure 9a – Residential Site Trip Assignment	19
Figure 9b –Day Care Center Site Trip Assignment	20
Figure 10 – Pass-By Site Trip Distribution	21
Figure 11 – Pass-by Site Trip Assignment	22
Figure 12 – Total Site Trip Assignment	23
Figure 13 – 2027 Build Peak Hour Traffic	25
Figure 14 – Recommended Lane Configurations	41



LIST OF TABLES

Table 1: Existing Roadway Inventory 3
Table 2: Adjacent Development Information 10
Table 3: Trip Generation Summary15
Table 4: Highway Capacity Manual – Levels-of-Service and Delay
Table 5: Analysis Summary of US 401 Bypass and Jonesville Road27
Table 6: Analysis Summary of US 401 Bypass and Eastern U-Turn Location30
Table 7: Analysis Summary of Mitchell Mill Road and Jonesville Road / Peebles Road
Table 8: Analysis Summary of Mitchell Mill Road and Site Access 135
Table 9: Analysis Summary of Mitchell Mill Road and Site Access 2



TECHNICAL APPENDIX

- Appendix A: Scoping Documentation
- Appendix B: Traffic Counts
- Appendix C: Adjacent Development Information
- Appendix D: Capacity Calculations US 401 Bypass & Jonesville Road
- Appendix E: Capacity Calculations US 401 Bypass & Eastern U-Turn Location
- Appendix F: Capacity Calculations Mitchell Mill Road & Jonesville Road / Peebles Road
- Appendix G: Capacity Calculations Mitchell Mill Road & Site Access 1
- Appendix H: Capacity Calculations Mitchell Mill Road & Site Access 2
- Appendix I: Turn Lane Warrants
- Appendix J: MUTCD / ITRE Signal Warrant Analysis



TRAFFIC IMPACT ANALYSIS HILLS AT HARRIS CREEK ROLESVILLE, NORTH CAROLINA

1. INTRODUCTION

The contents of this report present the findings of the Traffic Impact Analysis (TIA) conducted for the proposed Hills at Harris Creek development in Rolesville, North Carolina. The proposed development, anticipated to be completed in 2027, is located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane in Rolesville, North Carolina. The purpose of this study is to determine the potential impacts to the surrounding transportation system created by traffic generated by the proposed development, as well as recommend improvements to mitigate the impacts.

This TIA is an update to the approved TIA that was sealed on May 19, 2022. This updated study reflects changes to the proposed land use types and densities and a change to the site access at the westernmost driveway from full-movement to right-in/right-out. No other scope changes were made from the previous TIA.

The proposed development is expected to consist of 155 single-family homes, 115 townhomes, and a 12,000 sq. ft. day care center.

The study analyzes traffic conditions during the weekday AM and PM peak hours for the following scenarios:

- 2022 Existing Traffic Conditions
- 2027 No-Build Traffic Conditions
- 2027 Build Traffic Conditions



1.1. Site Location and Study Area

The proposed development is located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane in Rolesville, North Carolina. Refer to Figure 1 for the site location map. The study area for the TIA was determined through coordination with the North Carolina Department of Transportation (NCDOT) and the Town of Rolesville (Town) and consists of the following existing intersections:

- US 401 Bypass and Jonesville Road
- US 401 Bypass and Eastern U-Turn Location
- Mitchell Mill Road and Jonesville Road / Peebles Road

Refer to Appendix A for the approved scoping documentation.

1.2. Proposed Land Use and Site Access

The site is to be located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane. The proposed development, anticipated to be completed in 2027, is assumed to consist of the following uses:

- 155 single-family homes
- 115 townhomes
- 12,000 sq. ft. day care center

Site access is proposed via one (1) full-movement and one (1) right-in/right-out (RIRO) driveway connection along Mitchell Mill Road. Refer to Figure 2 for a copy of the preliminary site plan.

1.3. Adjacent Land Uses

The proposed development is located in an area consisting primarily of undeveloped land and residential development.



1.4. Existing Roadways

Existing lane configurations (number of traffic lanes on each intersection approach), storage capacities, and other intersection and roadway information within the study area are shown in Figure 3. Table 1 provides a summary of this information, as well.

Road Name	Route Number	Typical Cross- Section	Speed Limit	Maintained By	2019 AADT (vpd)	
US 401 Bypass		4-lane divided	55 mph	NCDOT	17,500	
Jonesville Road	SR 2226	2-lane undivided	35 mph / 45 mph	NCDOT	2,210*	
Mitchell Mill Road	SR 2224	C 2224 2-lane undivided 45 mph		NCDOT	4,000	
Peebles Road	SR 2929	2-lane undivided	45 mph	NCDOT	1,700*	

Table 1: Existing Roadway Inventory

*ADT based on 2022 existing traffic volumes and assuming the weekday PM peak hour volume is 10% of the average daily traffic.









2. 2022 EXISTING PEAK HOUR CONDITIONS

2.1. 2022 Existing Peak Hour Traffic Volumes

Existing peak hour traffic volumes were determined based on previously collected traffic counts conducted at the study intersections listed below, in November of 2021 during typical weekday AM (7:00 AM – 9:00 AM) and PM (4:00 PM – 6:00 PM) peak periods, while schools were in session for in-person learning:

- US 401 Bypass and Jonesville Road
- US 401 Bypass and Eastern U-Turn Location
- Mitchell Mill Road and Jonesville Road / Peebles Road

Previously collected counts from the year 2021 were projected to the 2022 existing analysis year using a compounded annual growth rate of 2%. Weekday AM and PM traffic volumes were balanced between study intersections, where appropriate. Refer to Figure 4 for 2022 existing weekday AM and PM peak hour traffic volumes. A copy of the count data is located in Appendix B of this report.

2.2. Analysis of 2022 Existing Peak Hour Traffic Conditions

The 2022 existing weekday AM and PM peak hour traffic volumes were analyzed to determine the current levels of service at the study intersections under existing roadway conditions. The results of the analysis are presented in Section 7 of this report.




3. 2027 NO-BUILD PEAK HOUR CONDITIONS

In order to account for the growth of traffic and subsequent traffic conditions at a future year, no-build traffic projections are needed. No-build traffic is the component of traffic due to the growth of the community and surrounding area that is anticipated to occur regardless of whether the proposed development is constructed. No-build traffic is comprised of existing traffic growth within the study area and additional traffic created as a result of adjacent approved developments.

3.1. Ambient Traffic Growth

Through coordination with NCDOT and the Town, it was determined that an annual growth rate of 2% would be used to generate 2027 projected weekday AM and PM peak hour traffic volumes. Refer to Figure 5 for 2027 projected peak hour traffic.

3.2. Adjacent Development Traffic

Through coordination with NCDOT and the Town, the following adjacent developments were identified to be included as an approved adjacent development in this study:

- Cobblestone Crossing Mixed-Use
- Young Street PUD
- Wheeler Tract
- Louisbury Road Assemblage
- Kalas / Watkins Family Property

Table 2, on the following page, provides a summary of the adjacent developments.



Development Name	Location	Build- Out Year	Land Use / Intensity	TIA Performed
Cobblestone Crossing Mixed- Use	Northwest quadrant of the intersection of Main Street and Young Street	2023	180 multi-family homes 18,200 sq. ft. municipal flex space 50,000 sq. ft. general retail	March 2021 by RKA
Young Street PUD	Along both sides of US 401 Bypass west of Young Street	2025	96 single-family homes 525 single-family homes 320 multi-family homes 122,800 sq. ft. general retail	June 2019 by Kimley Horn
Wheeler Tract	Northeast quadrant of the intersection of Rolesville Road and Mitchell Mill Road		233 single-family homes 125 multi-family homes	June 2019 by RKA
Louisbury Road Assemblage	West of Louisbury Road and south of Stells Road	2025	152 single-family homes	May 2020 by RKA
Kalas / Watkins Family Property	Along the west side of Rolesville Road, north of Mitchell Mill Road		439 single-family homes 96 multi-family homes	August 2019 by Stantec

Table 2: Adjacent Development Information

Adjacent development trips are shown in Figure 6. Adjacent development information can be found in Appendix C.

3.3. Future Roadway Improvements

Based on coordination with the NCDOT and the Town, it was determined there were no future roadway improvements to consider under future conditions with this study. It should be noted that per the Rolesville Community Transportation Plan (dated May 2022), the ultimate cross-section of Jonesville Road is identified as a 2-lane roadway with a center two-way-left-turn-lane (TWLTL) and Mitchell Mill Road is identified as a 4-lane median-divided roadway.



3.4. 2027 No-Build Peak Hour Traffic Volumes

The 2027 no-build traffic volumes were determined by projecting the 2022 existing peak hour traffic to the year 2027 and adding the adjacent development trips. Refer to Figure 7 for an illustration of the 2027 no-build peak hour traffic volumes at the study intersections.

3.5. Analysis of 2027 No-Build Peak Hour Traffic Conditions

The 2027 no-build AM and PM peak hour traffic volumes at the study intersections were analyzed with existing geometric roadway conditions and traffic control. The analysis results are presented in Section 7 of this report.









4. SITE TRIP GENERATION AND DISTRIBUTION

4.1. Trip Generation

The proposed development is assumed to consist of 155 single-family homes, 115 townhomes, and a 12,000 sq. ft. day care center. Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 11th Edition. Table 3 provides a summary of the trip generation potential for the site.

Land Use (ITE Code)	Intensity	Daily Traffic (vpd)	Weekday AM Peak Hour Trips (vph)			Weekday PM Peak Hour Trips (vph)		
			Enter	Exit	Total	Enter	Exit	Total
Single-Family Home (210)	155 DU	1,510	28	83	111	95	55	150
Single-Family Attached Housing (220)	115 DU	812	14	45	59	44	26	70
Day Care Center (565)	12 KSF	572	70	62	132	63	70	133
Total Trips	2,894	112	190	302	202	151	353	
Pass-By Trips: Day ((44% PM		-	-	-	-30	-30	-60	
Total Primary Trips			112	190	302	172	121	293

Table 3: Trip Generation Summary

It is estimated that the proposed development will generate approximately 2,894 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 302 trips (112 entering and 190 exiting) will occur during the weekday AM peak hour and 353 trips (202 entering and 151 exiting) will occur during the weekday PM peak hour.

Pass-by trips were also taken into consideration in this study. Pass-by trips are made by the traffic already using the adjacent roadway, entering the site as an intermediate stop on their way to another destination. Pass-by trips are expected to account for approximately 60 trips (30 entering and 30 exiting) during the weekday PM peak hour. It should be noted that the



pass-by trips were balanced, as it is likely that these trips would enter and exit in the same hour.

The total primary site trips are the calculated site trips after the reduction for pass-by trips. There are anticipated to be approximately 302 primary site trips (112 entering and 190 exiting) during the weekday AM peak hour and 293 primary site trips (172 entering and 121 exiting) during the weekday PM peak hour.

4.2. Site Trip Distribution and Assignment

Trip distribution percentages used in assigning site traffic for this development were estimated based on a combination of existing traffic patterns, population centers adjacent to the study area, and engineering judgment.

It is estimated that site trips will be regionally distributed as follows:

- 30% to/from the west via US 401 Bypass
- 15% to/from the east via US 401 Bypass
- 10% to/from the south via Peebles Road
- 35% to/from the west via Mitchell Mill Road
- 10% to/from the east via Mitchell Mill Road

The residential site trip distribution is shown in Figure 8a and the day care center site trip distribution is shown in Figure 8b. Refer to Figures 9a and 9b for the residential site trip assignment and day care center site trip assignment, respectively.

The pass-by site trips were distributed based on existing traffic patterns with consideration given to the proposed driveway access and site layout. Refer to Figure 10 for the pass-by site trip distribution. Pass-by site trips are shown in Figure 11.

The total site trips were determined by adding the primary site trips and the pass-by site trips. Refer to Figure 12 for the total peak hour site trips at the study intersections.

















5. **2027 BUILD TRAFFIC CONDITIONS**

5.1. 2027 Build Peak Hour Traffic Volumes

To estimate traffic conditions with the site fully built-out, the total site trips were added to the 2027 no-build traffic volumes to determine the 2027 build traffic volumes. Refer to Figure 13 for an illustration of the 2027 build peak hour traffic volumes with the proposed site fully developed.

5.2. Analysis of 2027 Build Peak Hour Traffic Conditions

Study intersections were analyzed with the 2027 build traffic volumes using the same methodology previously discussed for existing and no-build traffic conditions. Intersections were analyzed with improvements necessary to accommodate future traffic volumes. The results of the capacity analysis for each intersection are presented in Section 7 of this report.





6. TRAFFIC ANALYSIS PROCEDURE

Study intersections were analyzed using the methodology outlined in the *Highway Capacity Manual* (HCM), 6th Edition published by the Transportation Research Board. Capacity and level of service are the design criteria for this traffic study. A computer software package, Synchro (Version 11), was used to complete the analyses for most of the study area intersections. Please note that the unsignalized capacity analysis does not provide an overall level of service for an intersection; only delay for an approach with a conflicting movement.

The HCM defines capacity as "the maximum hourly rate at which persons or vehicles can reasonably be expected to traverse a point or uniform section of a lane or roadway during a given time period under prevailing roadway, traffic, and control conditions." Level of service (LOS) is a term used to represent different driving conditions and is defined as a "qualitative measure describing operational conditions within a traffic stream, and their perception by motorists and/or passengers." Level of service varies from Level "A" representing free flow, to Level "F" where breakdown conditions are evident. Refer to Table 4 for HCM levels of service and related average control delay per vehicle for both signalized and unsignalized intersections. Control delay as defined by the HCM includes "initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay". An average control delay of 50 seconds at a signalized intersection results in LOS "D" operation at the intersection.

UNSIGNA	ALIZED INTERSECTION	SIGNALIZED INTERSECTION		
LEVEL OF SERVICE	AVERAGE CONTROL DELAY PER VEHICLE (SECONDS)	LEVEL OF SERVICE	AVERAGE CONTROL DELAY PER VEHICLE (SECONDS)	
А	0-10	А	0-10	
В	10-15	В	10-20	
С	15-25	С	20-35	
D	25-35	D	35-55	
Е	35-50	Е	55-80	
F	>50	F	>80	

Table 4: Highway Capacity Manual - Levels-of-Service and Delay

6.1. Adjustments to Analysis Guidelines

Capacity analysis at all study intersections was completed according to Town LDO and NCDOT Congestions Management Guidelines.



7. CAPACITY ANALYSIS

7.1. US 401 Bypass and Jonesville Road

The existing unsignalized intersection of US 401 Bypass Road and Jonesville Road was analyzed under 2022 existing, 2027 no-build, and 2027 build traffic conditions with the lane configurations and traffic control shown in Table 5. Refer to Table 5 for a summary of the analysis results. Refer to Appendix D for the Synchro capacity analysis reports.

ANALYSIS	A P P R	LANE	WEEKDAY AM PEAK HOUR LEVEL OF SERVICE		WEEKDAY PM PEAK HOUR LEVEL OF SERVICE	
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
	EB	2 TH, 1 RT				
	WB*	1 LT 1 DT	C^1	N/A	E1	N/A
2022 Existing			B ²		C ²	
0	EB ^{**}		F1	N/A	C1	NT / A
	SB	2 IN, I KI 1 RT	 F2		 B ²	N/A
	EB	2 TH, 1 RT				
	WB*	1 LT	D^1	N/A	F^1	N/A
	NB	1 RT	B ²		E ²	.,
2027 No-Build	EB**	1 LT	F ¹		E1	
	WB	2 TH, 1 RT		N/A		N/A
	SB	1 RT	F ²	,	B2	,
	EB	2 TH, 1 RT				
2027 Build	WB*	1 LT	D^1	N/A	\mathbf{F}^{1}	N/A
	NB	1 RT	C ²		F ²	-
	EB**	1 LT	F ¹		E1	
	WB	2 TH, 1 RT		N/A		N/A
	SB	1 RT	F ²		B2	

 Table 5: Analysis Summary of US 401 Bypass and Jonesville Road

*Synchro analyzed the WB left-turns as SB through movements due to the nature of the superstreet and synchro limitations.

**Synchro analyzed the EB left-turns as NB through movements due to the nature of the superstreet and synchro limitations.

1. Level of service for major-street left-turn movement.

2. Level of service for minor-street approach.

Capacity analysis of 2022 existing traffic conditions indicates that the major-street left-turn movements and minor-street approaches are expected to operate at LOS D or better with the exception of the eastbound left-turn movement during the weekday AM peak hour (LOS F),



the westbound left-turn movement during the weekday PM peak hour (LOS E), and the southbound minor-street approach during the weekday AM peak hour (LOS E).

Under 2027 no-build and 2027 build traffic conditions, the major-street left-turn movements are expected to operate at LOS E/F during the weekday AM and PM peak hours with the exception of the westbound left-turn movement during the weekday AM peak hour (LOS D) under 2027 no-build and 2027 build traffic conditions. The minor-street approaches are expected to operate at LOS E/F during the weekday AM and PM peak hours with the exception of the northbound approach during the weekday AM peak hour (LOS B/C) and the southbound approach during the weekday PM peak hour (LOS B) under 2027 no-build and 2027 build traffic conditions. It should be noted that the proposed development is expected to account for approximately 5% of the total traffic at this intersection during both the weekday AM and PM peak hours. The proposed development is expected to account for approximately 36% and 28% of the overall northbound approach traffic at this intersection during the weekday AM and PM peak hours, respectively.

Due to the poor levels-of-service expected at this intersection, a traffic signal was considered under 2027 build traffic conditions to achieve acceptable levels of service. Weekday AM and PM peak hour traffic volumes were utilized in evaluating the potential need for signalization based on the guidelines contained within the *Manual on Uniform Traffic Control Devices* (MUTCD) and within the *Guidelines for Signalization of Intersections with Two or Three Approaches Final Report*, published by ITRE. Based on a review of signal warrants at this intersection, the peak hour warrant (warrant 3) from the MUTCD is expected to be met for both the weekday AM and PM peak hours under 2027 no-build and build traffic conditions. It is not expected that this intersection would satisfy the MUTCD 8-hour (warrant 1) or 4-hour (warrant 2) warrants, which NCDOT favors for installation of a traffic signal. These longer period warrants are not typically met for residential areas due to the distinct peak traffic periods for these types of development. Based on a review of ITRE 95th percentile queue length calculations, the northbound right-turn movement demand is expected to exceed capacity during the weekday PM peak hour under 2027 no-build and 2027 build traffic



conditions. Refer to Appendix J for a copy of the MUTCD warrants and the ITRE 95th percentile queue length calculations.

Based on the Town's LDO, improvements must be identified to maintain no-build levels-ofservice under build traffic conditions or to limit the degradation to less than a five percent increase in total delay on any approach for those operating at failing levels-of-service under no-build traffic conditions. Therefore, additional turn-lanes were considered for the northbound right-turn and westbound left-turn movements at this intersection to achieve acceptable operation per the Town's LDO. However, additional turn-lanes are not a realistic or practical improvement at an unsignalized intersection operating with superstreet configurations.

Based on the Town's LDO, it is recommended that this intersection be monitored for signalization and a full signal warrant analysis be conducted prior to the full build-out of the proposed development and install a traffic signal if warranted and approved by the Town and NCDOT. With signalization, it is expected that this intersection will operate at acceptable levels-of-service during the weekday AM and PM peak hours.



7.2. US 401 Bypass and Eastern U-Turn Location

The existing unsignalized intersection of US 401 Bypass and Eastern U-Turn Location was analyzed under 2022 existing, 2027 no-build, and 2027 build traffic conditions with the lane configurations and traffic control shown in Table 6. Refer to Table 6 for a summary of the analysis results. Refer to Appendix E for the Synchro capacity analysis reports.

ANALYSIS	A P P ANALYSIS R LANE		WEEKI PEAK LEVEL OF	DAY AM HOUR SERVICE	WEEKDAY PM PEAK HOUR LEVEL OF SERVICE	
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
2022 Existing	EB* WB	1 UT 2 TH	C1 	N/A	B1 	N/A
2027 No-Build	EB* WB	1 UT 2 TH	E1 	N/A	B1 	N/A
2027 Build	EB* WB	1 UT 2 TH	F ¹	N/A	B1 	N/A

Table 6: Analysis Summary of US 401 Bypass and Eastern U-TurnLocation

*Synchro analyzed the EB u-turn as a NB left-turn movement due to the nature of the superstreet and synchro limitations.

1. Level of service for major-street u-turn movement.

Capacity analysis of 2022 existing and 2027 no-build traffic conditions indicates that the major-street u-turn movement is expected to operate at LOS C or better during the weekday AM and PM peak hours, with the exception of the weekday AM peak hour under 2027 no-build conditions (LOS E).

Under 2027 build traffic conditions, the major-street u-turn movement is expected to operate at LOS F during the weekday AM peak hour and at LOS B during the weekday PM peak hour. It should be noted that the proposed development is expected to account for approximately 3% of the total traffic at this intersection during the weekday AM and PM peak hours. The proposed development is expected to account for approximately 36% and 33% of the overall northbound approach traffic at this intersection during the weekday AM and PM peak hours, respectively.



RAMEY KEMP ASSOCIATES

Due to the poor levels-of-service expected at this intersection, a traffic signal was considered under 2027 build traffic conditions to achieve acceptable levels of service. Weekday AM and PM peak hour traffic volumes were utilized in evaluating the potential need for signalization based on the guidelines contained within the Manual on Uniform Traffic Control Devices (MUTCD) and within the Guidelines for Signalization of Intersections with Two or Three Approaches Final Report, published by ITRE. Based on a review of signal warrants at this intersection, the peak hour warrant (warrant 3) from the MUTCD is expected to be met for the weekday AM peak hour under 2027 no-build and 2027 build traffic conditions. It is not expected that this intersection would satisfy the MUTCD 8-hour (warrant 1) or 4-hour (warrant 2) warrants, which NCDOT favors for installation of a traffic signal. These longer period warrants are not typically met for residential areas due to the distinct peak traffic periods for these types of development. Based on a review of ITRE 95th percentile queue length calculations, the eastbound u-turn movement demand is expected to exceed capacity during the weekday AM peak hour under 2027 no-build and 2027 build traffic conditions. Refer to Appendix J for a copy of the MUTCD warrants and the ITRE 95th percentile queue length calculations.

Based on the Town's LDO, improvements must be identified to maintain no-build levels-ofservice under build traffic conditions or to limit the degradation to less than a five percent increase in total delay on any approach for those operating at failing levels-of-service under no-build traffic conditions. Therefore, additional turn-lanes were considered for the eastbound u-turn movement at this intersection to achieve acceptable operation per the Town's LDO. However, additional turn-lanes are not a realistic or practical improvement at an unsignalized intersection operating with superstreet configurations.

Based on the Town's LDO, it is recommended that this intersection be monitored for signalization and a full signal warrant analysis be conducted prior to the full build-out of the proposed development and install a traffic signal if warranted and approved by the Town and NCDOT. With signalization, it is expected that this intersection will operate at acceptable levels-of-service during the weekday AM and PM peak hours.



7.3. Mitchell Mill Road and Jonesville Road / Peebles Road

The existing unsignalized intersection of Mitchell Mill Road and Jonesville Road / Peebles Road was analyzed under 2022 existing, 2027 no-build, and 2027 build traffic conditions with the lane configurations and traffic control shown in Table 7. Refer to Table 7 for a summary of the analysis results. Refer to Appendix F for the Synchro capacity analysis reports.

ANALYSIS	A P P R LANE		WEEKDAY AM PEAK HOUR LEVEL OF SERVICE		WEEKDAY PM PEAK HOUR LEVEL OF SERVICE	
SCENARIO O A C H	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
2022 Existing	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	$egin{array}{c} B^1 \ B^1 \ B^1 \ B^1 \ B^1 \end{array}$	B (13)	$\begin{array}{c} B^1\\ A^1\\ A^1\\ A^1\end{array}$	B (11)
2027 No-Build	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	$\begin{array}{c} C^1 \\ F^1 \\ B^1 \\ B^1 \end{array}$	F (51)	$\begin{array}{c} C^1 \\ C^1 \\ B^1 \\ B^1 \end{array}$	C (19)
2027 Build	EB WB NB SB	1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT 1 LT-TH-RT	$\begin{array}{c} C^1 \\ F^1 \\ B^1 \\ C^1 \end{array}$	F (142)	$F^1 \\ E^1 \\ C^1 \\ C^1$	F (53)
2027 Build - Improved	EB WB NB SB	1 LT-TH-RT 1 LT-TH, 1 RT 1 LT-TH-RT 1 LT , 1 TH-RT	$\begin{array}{c} C^1 \\ F^1 \\ C^1 \\ C^1 \end{array}$	F (103)	$F^1 \\ D^1 \\ C^1 \\ B^1$	F (61)

Table 7: Analysis Summary of Mitchell Mill Road and Jonesville Road / Peebles Road

Improvements by the developer are shown in bold.

1. Level of service for all-way stop controlled approach.

Capacity analysis of 2022 existing and 2027 no-build traffic conditions indicates that the intersection is expected to operate at an overall LOS C or better during the weekday AM and PM peak hours, with the exception of the weekday AM peak hour under 2027 no-build traffic conditions (LOS F). Under 2027 build traffic conditions, this intersection is expected to operate at an overall LOS F during the weekday AM and PM peak hours. It should be noted that the



proposed development is expected to account for approximately 19% and 20% of the total traffic at this intersection during the weekday AM and PM peak hours, respectively. The proposed development is expected to account for approximately 13% and 11% of the overall eastbound approach traffic and 21% and 23% of the overall westbound approach at this intersection during the weekday AM and PM peak hours, respectively.

Turn lanes were considered at this intersection in order to mitigate the proportional impact that the proposed site traffic is expected to have at this intersection and to improve overall operations. An exclusive left-turn lane on the southbound approach (Jonesville Road) and right-turn lane on the westbound approach (Mitchell Mill Road) are recommended by the developer. It should be noted that an exclusive southbound left-turn lane was also identified in the 5109 Mitchell Mill Road TIA. With these improvements, the intersection is expected to continue operating at an overall LOS F during the weekday AM and PM peak hours.

It should be noted that the overall intersection delay is expected to increase during the weekday PM peak hour as a result of the recommended improvements to the southbound and westbound approaches. This increase in delay is attributable to minor increases in delays for all approaches caused by adding additional lanes to an all-way stop-controlled intersection. No feasible improvements other than signalization would be expected to decrease delays further at this intersection.

Due to the poor levels-of-service expected at this intersection, a traffic signal was considered under 2027 build traffic conditions to achieve acceptable levels-of-service. The peak hour warrant (warrant 3) from the *Manual on Uniform Traffic Control Devices* (MUTCD) was considered. Based on a review of the peak hour signal warrant at this intersection, the intersection is expected to meet the peak hour warrant for the weekday AM peak hour under 2027 no-build traffic conditions and both the weekday AM and PM peak hours under 2027 build traffic conditions. It is not expected that this intersection would satisfy the MUTCD 8hour (warrant 1) or 4-hour (warrant 2) warrants, which NCDOT favors for installation of a traffic signal. These longer period warrants are not typically met for residential areas due to



the distinct peak traffic periods for these types of development. Refer to Appendix J for a copy of the MUTCD warrants.

Based on the Town's LDO, it is recommended that this intersection be monitored for signalization and a full signal warrant analysis be conducted prior to the full build-out of the proposed development and install a traffic signal if warranted and approved by the Town and NCDOT. With signalization, it is expected that this intersection will operate at acceptable levels-of-service during the weekday AM and PM peak hours.



7.4. Mitchell Mill Road and Site Access 1

The proposed intersection of Mitchell Mill Road and Site Access 1 was analyzed under 2027 build traffic conditions with the lane configurations and traffic control shown in Table 8. Refer to Table 8 for a summary of the analysis results. Refer to Appendix G for the synchro capacity analysis reports.

Table 8: Analysis Summary of Mitchell Mill Road and Site Access 1

ANALYSIS	A P P R	LANE	WEEKI PEAK LEVEL OF	DAY AM HOUR SERVICE	WEEKI PEAK LEVEL OF	DAY PM HOUR SERVICE
SCENARIO	O A C H	CONFIGURATIONS	Approach	Overall (seconds)	Approach	Overall (seconds)
2027 Build	EB WB SB	1 TH 1 TH -RT 1 RT	 C ²	N/A	 B ²	N/A

Improvements to lane configurations by the developer are shown in bold.

1. Level of service for major-street left-turn movement.

2. Level of service for minor-street approach.

Capacity analysis of 2027 build traffic conditions indicates that the minor-street approach is expected to operate at LOS C or better during the weekday AM and PM peak hours.

Based on the estimated low volume of right-turn movements into the proposed development at this intersection, an exclusive right-turn lane is not recommended.



7.5. Mitchell Mill Road and Site Access 2

The proposed intersection of Mitchell Mill Road and Site Access 2 was analyzed under 2027 build traffic conditions with the lane configurations and traffic control shown in Table 9. Refer to Table 9 for a summary of the analysis results. Refer to Appendix H for the synchro capacity analysis reports.

Α WEEKDAY AM WEEKDAY PM Ρ PEAK HOUR PEAK HOUR Ρ LEVEL OF SERVICE LEVEL OF SERVICE ANALYSIS R LANE 0 **SCENARIO** CONFIGURATIONS Overall Α Overall Approach Approach С (seconds) (seconds) н EB **1 LT**, 1 TH \mathbf{A}^1 \mathbf{A}^1 2027 Build N/A N/A WB 1 TH, 1 RT C^2 C^2 SB 1 LT-RT

Table 9: Analysis Summary of Mitchell Mill Road and Site Access 2

Improvements to lane configurations by the developer are shown in bold.

1. Level of service for major-street left-turn movement.

2. Level of service for minor-street approach.

Capacity analysis of 2027 build traffic conditions indicates that the major-street left-turn movement is expected to operate at LOS A during the weekday AM and PM peak hours. The minor-street approach is expected to operate at LOS C during the weekday AM and PM peak hours.

Right and left-turn lanes were considered based on the NCDOT *Policy on Street and Driveway Access to North Carolina Highways*. The policy states that turn lanes should be considered on roads that carry at least 4,000 vehicles per day. Mitchell Mill Road is expected to carry more than 4,000 vehicles per day. Based on previous coordination with NCDOT, an eastbound left turn lane and a westbound right turn lane is recommended to be constructed by the developer on Mitchell Mill Road at Site Access 2. Refer to Appendix I for a copy of the turn lane warrants.



8. CONCLUSIONS

This Traffic Impact Analysis was conducted to determine the potential traffic impacts of the proposed Hills at Harris Creek development located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane in Rolesville, North Carolina. The development is expected to consist of 155 single-family homes, 115 townhomes, and a 12,000 sq. ft. day care center and to be built-out in 2027. Site access is proposed via one (1) full-movement and one (1) right-in/right-out driveway connection along Mitchell Mill Road.

The study analyzes traffic conditions during the weekday AM and PM peak hours for the following scenarios:

- 2022 Existing Traffic Conditions
- 2027 No-Build Traffic Conditions
- 2027 Build Traffic Conditions

Trip Generation

It is estimated that the proposed development will generate approximately 302 primary trips (112 entering and 190 exiting) during the weekday AM peak hour and 293 primary trips (172 entering and 121 exiting) during the weekday PM peak hour.

Rolesville Community Transportation Plan

Per the Rolesville Community Transportation Plan (CTP), the ultimate cross-section of Mitchell Mill Road is identified as a 4-lane median-divided roadway. It is recommended that the proposed development widen one-half section of Mitchell Mill Road along the site frontage in accordance with the Town's CTP.



Adjustments to Analysis Guidelines

Capacity analysis at all study intersections was completed according to NCDOT Congestion Management Guidelines. Refer to section 6.1 of this report for a detailed description of any adjustments to these guidelines made throughout the analysis.

Intersection Capacity Analysis Summary

All the study area intersections (including the proposed site driveways) are expected to operate at acceptable levels-of-service under existing and future year conditions with the exception of those identified in Section 7 of this report.



9. **RECOMMENDATIONS**

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions. See a more detailed description of the recommended improvements below. Refer to Figure 14 for an illustration of the recommended lane configurations for the proposed development.

Recommended Improvements by Developer

Required Frontage Improvements per Rolesville Community Transportation Plan

• Widen one-half section of Mitchell Mill Road along the site frontage to this roadway's ultimate section (4-lane median divided).

US 401 Bypass and Jonesville Road

• Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.

US 401 Bypass and Eastern U-Turn Location

• Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.

Mitchell Mill Road and Jonesville Road / Peebles Road

- Construct a southbound (Jonesville Road) left-turn lane with at least 100 feet of storage and appropriate decel and taper.
 - It should be noted that this improvement was also identified by the 5109
 Mitchell Mill Road TIA.
- Construct a westbound (Mitchell Mill Road) right-turn lane with at least 100 feet of storage and appropriate decel and taper.
- Conduct a full signal warrant analysis prior to full build-out of the proposed development and install a traffic signal if warranted and approved by NCDOT and the Town.



Mitchell Mill Road and Site Access 1

- Construct the southbound approach (Site Access 1) as a right-in/right-out with one ingress lane and one egress lane.
- Provide stop-control for the southbound approach (Site Access 1).
- Construct a concrete median on Mitchell Mill Road that restricts access to rightin/right-out.

Mitchell Mill Road and Site Access 2

- Construct the southbound approach (Site Access 2) with one ingress lane and one egress lane.
- Provide stop-control for the southbound approach (Site Access 2).
- Construct an eastbound (Mitchell Mill Road) left-turn lane with at least 150 feet of storage and appropriate decel and taper.
- Construct a westbound (Mitchell Mill Road) right-turn lane with at least 100 feet of storage and appropriate decel and taper.





APPENDIX A

SCOPING DOCUMENTATION

Andrew Eagle

From:	Warren, Jeremy L <jlwarren@ncdot.gov></jlwarren@ncdot.gov>
Sent:	Tuesday, May 23, 2023 12:17 PM
То:	Andrew Eagle; Nolfo, Matthew J
Cc:	Jessica McClure; Daniel Reisfeld; Elabarger, Michael S; Gruber, Meredith; Jason Pfister;
	Lineberger, Nicholas C; Darnell, Trevor S
Subject:	RE: [External] RE: Hills at Harris Creek

Andrew, We would prefer to see an updated TIA with the new volumes and the RIRO. Thanks,

Jeremy Warren, P.E. Assistant Division Maintenance Engineer Division 5 North Carolina Department of Transportation

ilwarren@ncdot.gov

4009 District Drive Raleigh, NC 27607



Email correspondence to and from this address is subject to the North Carolina Public Records Law and may be disclosed to third parties.

From: Andrew Eagle <AEagle@rameykemp.com>
Sent: Monday, May 22, 2023 9:34 AM
To: Warren, Jeremy L <jlwarren@ncdot.gov>; Nolfo, Matthew J <mjnolfo@ncdot.gov>
Cc: Jessica McClure <jmcclure@rameykemp.com>; Daniel Reisfeld <dreisfeld@rameykemp.com>; Elabarger, Michael S
<michael.elabarger@rolesville.nc.gov>; Gruber, Meredith <meredith.gruber@rolesville.nc.gov>; Jason Pfister
<jason@ellisdevgroup.com>
Subject: RE: [External] RE: Hills at Harris Creek

CAUTION: External email. Do not click links or open attachments unless verified. Report suspicious emails with the Report Message button located on your Outlook menu bar on the Home tab.

Jeremy/Matt,

It's my understanding NCDOT prefers Access A to be RIRO. We can update the TIA to match this configuration. Also, the developer now plans the commercial area to be 12,000 square feet of Day Care instead of 14,000 square feet of retail. This results in 30 more entering trips and 20 more exiting trips during the AM peak hour when compared to the original TIA. During the PM peak hour the Day Care scenario results in 11 fewer entering trips and 4 more exiting trips.

Does NCDOT want the TIA updated to reflect the Day Care trip generation as well as the RIRO?
Andrew Eagle, PE, PTOE Senior Traffic Engineering Project Manager D 704 220 6847 | C 704 467 0325



From: Andrew Eagle
Sent: Monday, May 8, 2023 2:59 PM
To: Warren, Jeremy L <jlwarren@ncdot.gov>; Nolfo, Matthew J <mjnolfo@ncdot.gov>
Cc: Jessica McClure <JMCClure@rameykemp.com>; Daniel Reisfeld <dreisfeld@rameykemp.com>; Elabarger, Michael S
<michael.elabarger@rolesville.nc.gov>; Gruber, Meredith <meredith.gruber@rolesville.nc.gov>; Jason Pfister
<jason@ellisdevgroup.com>
Subject: RE: [External] RE: Hills at Harris Creek

The link below can be used to download the TIA, site plan, and Synchro files. Please review and let me know if you have any questions/comments. Thank you!

20498-0005 - Hills at Harris Creek - 05-08-2023

Andrew Eagle, PE, PTOE Senior Traffic Engineering Project Manager D 704 220 6847 | C 704 467 0325



From: Warren, Jeremy L <<u>ilwarren@ncdot.gov</u>>
Sent: Friday, April 14, 2023 8:53 AM
To: Andrew Eagle <<u>AEagle@rameykemp.com</u>>; Nolfo, Matthew J <<u>mjnolfo@ncdot.gov</u>>
Cc: Jessica McClure <<u>JMCClure@rameykemp.com</u>>; Daniel Reisfeld <<u>dreisfeld@rameykemp.com</u>>
Subject: RE: [External] RE: Hills at Harris Creek

The study area should be the same so an updated TIA with an explanation should suffice.

Jeremy Warren, P.E. Assistant Division Maintenance Engineer Division 5 North Carolina Department of Transportation

jlwarren@ncdot.gov

4009 District Drive Raleigh, NC 27607



Email correspondence to and from this address is subject to the North Carolina Public Records Law and may be disclosed to third parties.

From: Andrew Eagle <<u>AEagle@rameykemp.com</u>>
Sent: Friday, April 14, 2023 8:37 AM
To: Warren, Jeremy L <<u>jlwarren@ncdot.gov</u>>; Nolfo, Matthew J <<u>mjnolfo@ncdot.gov</u>>
Cc: Jessica McClure <<u>jmcclure@rameykemp.com</u>>; Daniel Reisfeld <<u>dreisfeld@rameykemp.com</u>>
Subject: [External] RE: Hills at Harris Creek

CAUTION: External email. Do not click links or open attachments unless you verify. Send all suspicious email as an attachment to <u>Report Spam.</u>

Jeremy,

NCDOT approved the Hills at Harris Creek TIA in June of last year. The site plan has been changed, resulting in less density and fewer trips. The Town of Rolesville wants the TIA updated to match the latest site plan. Do you want an updated MOU, or can we revise the TIA and submit it via email with an explanation of the changes?

...and a heads up due to the similar names, this is a different project from Harris Creek Farm. NCDOT recently approved the MOU for that one on April 3.

Thanks,

Andrew Eagle, PE, PTOE Senior Traffic Engineering Project Manager D 704 220 6847 | C 704 467 0325



From: Warren, Jeremy L <<u>ilwarren@ncdot.gov</u>>
Sent: Friday, June 17, 2022 1:47 PM
To: Tucker Fulle <<u>tfulle@rameykemp.com</u>>
Cc: Nolfo, Matthew J <<u>mjnolfo@ncdot.gov</u>>; Brennan, Sean P <<u>spbrennan@ncdot.gov</u>>
Subject: FW: Hills at Harris Creek

Please see congestions comments.

Jeremy Warren, P.E. District Engineer Division 5, District 1 North Carolina Department of Transportation

919 733 3213 office jlwarren@ncdot.gov

4009 District Drive Raleigh, NC 27607



Email correspondence to and from this address is subject to the North Carolina Public Records Law and may be disclosed to third parties.

From: Walker, Braden M <<u>bmwalker1@ncdot.gov</u>>
Sent: Thursday, June 16, 2022 9:32 AM
To: Warren, Jeremy L <<u>ilwarren@ncdot.gov</u>>
Cc: Lacy, Kevin <<u>jklacy1@ncdot.gov</u>>; Brennan, Sean P <<u>spbrennan@ncdot.gov</u>>; Grant, John H <<u>jhgrant@ncdot.gov</u>>;
Keilson, David P <<u>dpkeilson@ncdot.gov</u>>; Ishak, Doumit Y <<u>dishak@ncdot.gov</u>>; Bunting, Clarence B
<<u>cbunting@ncdot.gov</u>>; Jones, Brandon H <<u>bhjones@ncdot.gov</u>>; Parrott, Tracy N <<u>tnparrott@ncdot.gov</u>>; Holmes,
Benjamin W <<u>bwholmes@ncdot.gov</u>>; Mcneal, Douglas R <<u>dmcneal@ncdot.gov</u>>; Nolfo, Matthew J<<<u>minolfo@ncdot.gov</u>>

Subject: Hills at Harris Creek

Attached is a PDF copy of our final review letter for Hills at Harris Creek. This letter is only being distributed electronically and should be considered as the official documentation. If you have any questions, please contact me or Clarence Bunting.

Thank you,

Braden M. Walker, PE. Congestion Management Project Design Engineer Traffic Management Unit North Carolina Department of Transportation

919 814 5078 office bmwalker1@ncdot.gov

750 N. Greenfield Parkway Garner, NC 27529



Email correspondence to and from this address is subject to the North Carolina Public Records Law and may be disclosed to third parties.

Email correspondence to and from this sender is subject to the N.C. Public Records Law and may be disclosed to third parties.

Email correspondence to and from this sender is subject to the N.C. Public Records Law and may be disclosed to third parties.

Email correspondence to and from this sender is subject to the N.C. Public Records Law and may be disclosed to third parties.

Email correspondence to and from this sender is subject to the N.C. Public Records Law and may be disclosed to third parties.

RAMEY KEMP ASSOCIATES

TOGETHER WE ARE LIMITLESS

T 919 872 5115 5808 Faringdon Pl, Raleigh, NC 27609

March 24, 2022

Meredith Gruber, PLA, AICP Town of Rolesville - Planning Director PO Box 250 502 Southtown Circle Rolesville, NC 27571 <u>meredith.gruber@rolesville.nc.gov</u> [Sent via Email]

Reference:	Hills at Harris Creek
	Rolesville, North Carolina

Subject: Memorandum of Understanding for TIA Report

Dear Ms. Gruber:

The following is a Memorandum of Understanding (MOU) outlining the proposed scope of work and assumptions related to the Traffic Impact Analysis (TIA) for the proposed Hills at Harris Creek development in Rolesville, North Carolina. The proposed development is to be located north of Mitchell Mill Road, west of Manly Farm Road, and east of Gro Peg Lane in Rolesville, NC. The development is expected to consist of 211 single-family homes, 109 townhomes, and 3.626 acres of commercial development. This MOU reflects the assumptions outlined during the initial coordination between Ramey Kemp Associates (RKA), the Town of Rolesville (Town), and the North Carolina Department of Transportation (NCDOT). Refer to the attached site location map. Site access to the proposed development is expected to be provided via two (2) full-movement driveway connections along Mitchell Mill Road. Refer to the attachments for a copy of the preliminary site plan.

The proposed development, anticipated to be completed in 2027, is expected to consist of 211 singlefamily homes, 109 townhomes, and 3.626 acres of commercial development. It should be noted that the commercial development land use(s) and intensity are not known at this time. Therefore, 7,000 square feet (sq. ft.) of general retail space per acre of land [approximately 25,400 sq. ft.] was assumed for the commercial development in this study. The proposed development is assumed to consist of the following land uses:

- 211 single-family homes
- 109 townhomes
- 25,400 sq. ft. of general retail



Study Area

Based on a coordination with NCDOT and Town staff, the study area is proposed to consist of the following intersections:

- Mitchell Mill Road & Jonesville Road / Peebles Road (unsignalized)
- US 401 Bypass and Jonesville Road (unsignalized)
- US 401 Bypass and Eastern U-Turn Location (unsignalized)
- Mitchell Mill Road and Site Driveways (2)

Existing Traffic Volumes

Existing peak hour traffic volumes will be determined based on previously collected traffic counts at the study intersections below, in November 2021 during typical weekday AM (7:00 AM – 9:00 AM) and PM (4:00 PM – 6:00 PM) peak periods, while schools were in session for in-person learning:

- Mitchell Mill Road & Jonesville Road / Peebles Road
- US 401 Bypass and Jonesville Road
- US 401 Bypass and Eastern U-Turn Location

These previously collected counts will be projected to the year 2022 using a compounded annual growth rate of 2%. Refer to the attachments for an illustration of 2022 existing peak hour traffic volumes.

Background Traffic Volumes

Based on coordination with NCDOT and the Town, background traffic volumes will be determined by projecting 2022 existing traffic volumes to the year 2027 using a 2% annual growth rate. Additionally, it was determined that the following adjacent developments are to be included in this study:

- Cobblestone Crossing Mixed-Use
- Young Street PUD
- Wheeler Tract
- Louisbury Road Assemblage
- Kalas / Watkins Family Property

Future Roadway Improvements

Based on coordination with the Town and NCDOT, it was determined that there are no future roadway improvements within the study area to consider under future traffic conditions.

Trip Generation

Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 11th Edition. Refer to



Table 1, on the following page, for a summary of the proposed site trip generation for full buildout of the proposed development.

	l able 1:	I rip Gen	eration S	ummary					
Land Use (ITE Code)	Intensity	Daily Traffic	AM P	Weekday eak Hour (vph)	Trips	Weekday PM Peak Hour Trips (vph)			
		(vpu)	Enter	Exit	Total	Enter	Exit	Total	
Single-Family Home (210)	211 DU	2,010	38	109	147	126	74	200	
Multi-Family Home (Low-Rise) (220)	109 DU	770	14	43	57	42	25	67	
Retail (<40 KSF) (822)	25.4* KSF	1,300	32	21	53	75	76	151	
Total Trips		4,080	84	173	257	243	175	418	
Internal Captu (2% AM, 1% PN	re A)**		-2	-3	-5	-5	-3	-8	
Total External Trips			82	170	252	238	172	410	
Pass-By Trips: Shopping Center (34% PM)			-	_	_	-25	-25	-50	
Total Primary T	rips		82	170	252	213	147	360	

T-1.1. 1. T...

*Since the commercial development is unknown at this time, 7,000 SF of general retail space per acre of land [3.626 acres in total] was assumed for this land use.

**Utilizing methodology contained in the NCHRP Report 684.

It is estimated that the proposed development will generate approximately 4,080 site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 257 trips (84 entering and 173 exiting) will occur during the weekday AM peak hour and 418 trips (243 entering and 175 exiting) will occur during the weekday PM peak hour.

Internal capture of trips between the retail and residential land uses was considered in this study. Internal capture is the consideration for trips that will be made within the site between different land uses, so the vehicle technically never leaves the internal site but can still be considered as a trip to that specific land use. Based on NCHRP Report 684 methodology, weekday AM and PM peak hour internal capture rates of 2% and 1%, respectively, were applied to the trips generated from the development. The internal capture reductions are expected to account for approximately 5 trips (2 entering and 3 exiting) during the weekday AM peak hour and 8 trips (5 entering and 3 exiting) during the weekday PM peak hour. Refer to the attached NCHRP internal capture reports for reference.

Pass-by trips will also be taken into consideration in this study. Pass-by trips are made by the traffic already using the adjacent roadway, entering the site as an intermediate stop on their way to another destination. Pass-by percentages are applied to site trips after adjustments for internal capture. Passby trips are expected to account for approximately 50 trips (25 entering and 25 exiting) during the



weekday PM peak hour. It should be noted that the pass-by trips were balanced, as it is likely that these trips would enter and exit in the same hour.

The total primary trips are the calculated site trips after the reduction for internal capture and pass-by trips. Primary site traffic is expected to generate approximately 252 trips (82 entering and 170 exiting) during the weekday AM peak hour, and 360 trips (213 entering and 147 exiting) during the weekday PM peak hour.

Trip Distribution and Assignment

Site trips are distributed based on the locations of existing traffic patterns, population centers adjacent to the study area, and engineering judgment. A summary of the overall distributions is below.

Residential

- 30% to/from the west via US 401 Bypass
- 15% to/from the east via US 401 Bypass
- 10% to/from the south via Peebles Road
- 35% to/from the west via Mitchell Mill Road
- 10% to/from the east via Mitchell Mill Road

Commercial

- 25% to/from the west via US 401 Bypass
- 15% to/from the east via US 401 Bypass
- 10% to/from the south via Peebles Road
- 40% to/from the west via Mitchell Mill Road
- 10% to/from the east via Mitchell Mill Road

Refer to the attached site trip distribution figures.

Analysis Scenarios

All capacity analyses will be performed utilizing Synchro (Version 10.3). All study intersections will be analyzed during the weekday AM and PM peak hours under the following proposed traffic scenarios:

- 2022 Existing Traffic Conditions
- 2027 No-Build Traffic Conditions
- 2027 Build Traffic Conditions



<u>Report</u>

The TIA report will be prepared based on the Town and NCDOT requirements.

If you find this memorandum of understanding acceptable, please let me know so that we may include it in the TIA report. If you have any questions or concerns, please do not hesitate to contact me.

Sincerely, *Ramey Kemp Associates*,

Under Kompute

Michael Karpkinski, P.E. Traffic Engineering Project Manager

Attachments:

Site Location Map Site Plan 2022 Existing Traffic Volumes Figure NCHRP 684 Internal Capture Reports Proposed Site Trip Distribution Figures









	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Hills at Harris Creek		Organization:	Ramey Kemp & Associates						
Project Location:	Rolesville, NC		Performed By:	TF						
Scenario Description:	AM Street Peak Hour		Date:	3/18/2022						
Analysis Year:	20xx Build		Checked By:							
Analysis Period:	AM Street Peak Hour		Date:							

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)

						-		
Land Lise	Development Data (For Information Only)				Estimated Vehicle-Trips ³			
Land Use	ITE LUCs ¹	Quantity	Units		Total	Entering	Exiting	
Office								
Retail	822	25,400	sq.ft.			32	21	
Restaurant								
Cinema/Entertainment								
Residential	210,220	211,109	units			52	152	
Hotel								
All Other Land Uses ²								
					0	84	173	

	Table 2-A: Mode Split and Vehicle Occupancy Estimates									
Land Use		Entering Tri	os		Exiting Trips					
	Veh. Occ.4	% Transit	% Non-Motorized		Veh. Occ. ⁴	% Transit	% Non-Motorized			
Office	1.10	0%	0%		1.10	0%	0%			
Retail	1.10	0%	0%		1.10	0%	0%			
Restaurant	1.10	0%	0%		1.10	0%	0%			
Cinema/Entertainment	1.10	0%	0%		1.10	0%	0%			
Residential	1.10	0%	0%		1.10	0%	0%			
Hotel	1.10	0%	0%		1.10	0%	0%			
All Other Land Uses ²	1.10	0%	0%		1.10	0%	0%			

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)								
Origin (From)				Destination (To)				
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office								
Retail								
Restaurant								
Cinema/Entertainment								
Residential								
Hotel								

Table 4-A: Internal Person-Trip Origin-Destination Matrix*										
Origin (From)		Destination (To)								
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		0	0	0	0	0				
Retail	0		0	0	1	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	0	2	0	0		0				
Hotel	0	0	0	0	0					

Table 5-A: Computations Summary				Table 6-A: Internal Trip Capture Percentages by Land Use			
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Trips	
All Person-Trips	282	92	190	Office	N/A	N/A	
Internal Capture Percentage	2%	3%	2%	Retail	6%	4%	
		-		Restaurant	N/A	N/A	
External Vehicle-Trips⁵	251	81	170	Cinema/Entertainment	N/A	N/A	
External Transit-Trips ⁶	0	0	0	Residential	2%	1%	
External Non-Motorized Trips ⁶	0	0	0	Hotel	N/A	N/A	

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

Г

Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Project Name:	Hills at Harris Creek
Analysis Period:	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends									
Land Use	Tab	ole 7-A (D): Enter	ing Trips		Table 7-A (O): Exiting Trips				
	Veh. Occ.	Vehicle-Trips	Person-Trips*	1	Veh. Occ.	Vehicle-Trips	Person-Trips*		
Office	1.10	0	0		1.10	0	0		
Retail	1.10	32	35		1.10	21	23		
Restaurant	1.10	0	0		1.10	0	0		
Cinema/Entertainment	1.10	0	0		1.10	0	0		
Residential	1.10	52	57	1	1.10	152	167		
Hotel	1.10	0	0	1	1.10	0	0		

	Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)										
Origin (From)	Destination (To)										
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		0	0	0	0	0					
Retail	7		3	0	3	0					
Restaurant	0	0		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	3	2	33	0		0					
Hotel	0	0	0	0	0						

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)									
Origin (From)				Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		11	0	0	0	0			
Retail	0		0	0	1	0			
Restaurant	0	3		0	3	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	6	0	0		0			
Hotel	0	1	0	0	0				

	Table 9-A (D): Internal and External Trips Summary (Entering Trips)									
Destination Land Use		Person-Trip Esti	mates			External Trips by Mode*				
	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²			
Office	0	0	0		0	0	0			
Retail	2	33	35		30	0	0			
Restaurant	0	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0	0			
Residential	1	56	57		51	0	0			
Hotel	0	0	0	1	0	0	0			
All Other Land Uses ³	0	0	0		0	0	0			

	Table 9-A (O): Internal and External Trips Summary (Exiting Trips)									
Origin Land Use		Person-Trip Esti	mates			External Trips by Mode*				
	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²			
Office	0	0	0		0	0	0			
Retail	1	22	23		20	0	0			
Restaurant	0	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0	0			
Residential	2	165	167		150	0	0			
Hotel	0	0	0		0	0	0			
All Other Land Uses ³	0	0	0		0	0	0			

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator *Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Hills at Harris Creek		Organization:	Ramey Kemp & Associates						
Project Location:	Rolesville, NC		Performed By:	TF						
Scenario Description:	AM Street Peak Hour		Date:	3/18/2022						
Analysis Year:	20xx Build		Checked By:							
Analysis Period:	PM Street Peak Hour		Date:							

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)									
Land Use	Developm	ent Data (For Ini	formation Only)			Estimated Vehicle-Trips ³			
	ITE LUCs ¹	Quantity	Units		Total	Entering	Exiting		
Office									
Retail	822	25,400	sq.ft.			75	76		
Restaurant									
Cinema/Entertainment									
Residential	210,220	211,109	units			168	99		
Hotel									
All Other Land Uses ²									
					0	243	175		

	Table 2-P: Mode Split and Vehicle Occupancy Estimates									
		Entering Trip	os			Exiting Trips				
Lanu Use	Veh. Occ.4	% Transit	% Non-Motorized		Veh. Occ. ⁴	% Transit	% Non-Motorized			
Office	1.10	0%	0%		1.10	0%	0%			
Retail	1.10	0%	0%		1.10	0%	0%			
Restaurant	1.10	0%	0%		1.10	0%	0%			
Cinema/Entertainment	1.10	0%	0%		1.10	0%	0%			
Residential	1.10	0%	0%		1.10	0%	0%			
Hotel	1.10	0%	0%		1.10	0%	0%			
All Other Land Uses ²	1.10	0%	0%		1.10	0%	0%			

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)				Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		4000	4000		4000				
Retail					4000				
Restaurant					4000				
Cinema/Entertainment					4000				
Residential		4000	4000						
Hotel					4000				

Table 4-P: Internal Person-Trip Origin-Destination Matrix*										
Origin (From)	Destination (To)									
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		0	0	0	0	0				
Retail	0		0	0	2	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	0	1	0	0		0				
Hotel	0	0	0	0	0					

Table 5-P	Table 5-P: Computations Summary				Table 6-P: Internal Trip Capture Percentages by Land Use			
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Trips		
All Person-Trips	461	268	193	Office	N/A	N/A		
Internal Capture Percentage	1%	1%	2%	Retail	1%	2%		
				Restaurant	N/A	N/A		
External Vehicle-Trips ⁵	414	241	173	Cinema/Entertainment	N/A	N/A		
External Transit-Trips ⁶	0	0	0	Residential	1%	1%		
External Non-Motorized Trips ⁶	0	0	0	Hotel	N/A	N/A		

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made ⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Project Name:	Hills at Harris Creek
Analysis Period:	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends									
Land Use	Table	7-P (D): Entering	g Trips		Table 7-P (O): Exiting Trips				
	Veh. Occ.	Vehicle-Trips	Person-Trips*		Veh. Occ.	Vehicle-Trips	Person-Trips*		
Office	1.10	0	0		1.10	0	0		
Retail	1.10	75	83		1.10	76	84		
Restaurant	1.10	0	0		1.10	0	0		
Cinema/Entertainment	1.10	0	0		1.10	0	0		
Residential	1.10	168	185		1.10	99	109		
Hotel	1.10	0	0		1.10	0	0		

	Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)									
Origin (From)	Destination (To)									
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		0	0	0	0	0				
Retail	2		24	3	2	4				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	4	5	2	0		3				
Hotel	0	0	0	0	0					

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)										
Origin (From)	Destination (To)									
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		1	0	0	7	0				
Retail	0		0	0	85	0				
Restaurant	0	42		0	30	0				
Cinema/Entertainment	0	3	0		7	0				
Residential	0	1	0	0		0				
Hotel	0	2	0	0	0					

	Tab	le 9-P (D): Interi	nal and External T	rips	s Summary (Entering Tri	ips)	
	Pe	rson-Trip Estima	ates			External Trips by Mode*	
Destination Land Ose	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²
Office	0	0	0		0	0	0
Retail	1	82	83		75	0	0
Restaurant	0	0	0	1	0	0	0
Cinema/Entertainment	0	0	0		0	0	0
Residential	2	183	185		166	0	0
Hotel	0	0	0		0	0	0
All Other Land Uses ³	0	0	0		0	0	0

	Tal	ole 9-P (O): Inter	rnal and External 1	「rip	s Summary (Exiting Tri	ps)	
	Pe	erson-Trip Estima	ates			External Trips by Mode*	
Origin Land Ose	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²
Office	0	0	0		0	0	0
Retail	2	82	84		75	0	0
Restaurant	0	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0	0
Residential	1	108	109		98	0	0
Hotel	0	0	0		0	0	0
All Other Land Uses ³	0	0	0		0	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P ²Person-Trips ³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator *Indicates computation that has been rounded to the nearest whole number.





APPENDIX B

TRAFFIC COUNTS



File Name : Rolesville(US 401 and Jonesville)AM Peak Site Code : Start Date : 11/9/2021 Page No : 1

Grou	ps F	Printed-	Cars	+ -	Trucks	

		Jonesv	ille Roa	ıd		US	401			Jonesv	ille Roa	d		US	401		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
07:00 AM	63	0	0	63	24	380	21	425	23	0	0	23	18	182	3	203	714
07:15 AM	42	0	0	42	39	362	24	425	37	0	0	37	11	125	7	143	647
07:30 AM	51	0	0	51	80	318	23	421	48	0	0	48	24	136	15	175	695
07:45 AM	65	0	0	65	38	249	16	303	25	0	0	25	25	135	10	170	563
Total	221	0	0	221	181	1309	84	1574	133	0	0	133	78	578	35	691	2619
08:00 AM	61	0	0	61	26	236	13	275	23	0	0	23	30	120	10	160	519
08:15 AM	36	0	0	36	12	233	9	254	16	0	0	16	13	94	9	116	422
08:30 AM	24	0	0	24	10	213	5	228	9	0	0	9	6	91	3	100	361
08:45 AM	28	0	0	28	9	145	5	159	10	0	0	10	11	85	2	98	295
Total	149	0	0	149	57	827	32	916	58	0	0	58	60	390	24	474	1597
Grand Total	370	0	0	370	238	2136	116	2490	191	0	0	191	138	968	59	1165	4216
Apprch %	100	0	0		9.6	85.8	4.7		100	0	0		11.8	83.1	5.1		
Total %	8.8	0	0	8.8	5.6	50.7	2.8	59.1	4.5	0	0	4.5	3.3	23	1.4	27.6	
Cars +	366	0	0	366	233	2094	114	2441	188	0	0	188	135	916	57	1108	4103
% Cars +	98.9	0	0	98.9	97.9	98	98.3	98	98.4	0	0	98.4	97.8	94.6	96.6	95.1	97.3
Trucks	4	0	0	4	5	42	2	49	3	0	0	3	3	52	2	57	113
% Trucks	1.1	0	0	1.1	2.1	2	1.7	2	1.6	0	0	1.6	2.2	5.4	3.4	4.9	2.7



File Name : Rolesville(US 401 and Jonesville)AM Peak Site Code : Start Date : 11/9/2021 Page No : 2

		Jonesvi	lle Roa	d		US	401			Jonesvi	lle Roa	d		US	401		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	alysis Fro	om 07:0	0 AM to	o 08:45 A	M - Pea	ak 1 of 1											
Peak Hour for	Entire In	tersecti	on Begi	ins at 07:	00 AM												
07:00 AM	63	0	0	63	24	380	21	425	23	0	0	23	18	182	3	203	714
07:15 AM	42	0	0	42	39	362	24	425	37	0	0	37	11	125	7	143	647
07:30 AM	51	0	0	51	80	318	23	421	48	0	0	48	24	136	15	175	695
07:45 AM	65	0	0	65	38	249	16	303	25	0	0	25	25	135	10	170	563
Total Volume	221	0	0	221	181	1309	84	1574	133	0	0	133	78	578	35	691	2619
% App. Total	100	0	0		11.5	83.2	5.3		100	0	0		11.3	83.6	5.1		
PHF	.850	.000	.000	.850	.566	.861	.875	.926	.693	.000	.000	.693	.780	.794	.583	.851	.917





File Name : Rolesville(US 401 and Jonesville)PM Peak Site Code : Start Date : 11/9/2021 Page No : 1

• · · · · · · · · · · · · · · · · · · ·	Groups	Printed-	Cars + -	Trucks
---	--------	----------	----------	--------

		Jonesvi	ille Roa	ıd		US	401			Jonesv	ille Roa	d		US	401		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:00 PM	47	0	0	47	13	124	6	143	21	0	0	21	37	217	22	276	487
04:15 PM	34	0	0	34	13	119	6	138	26	0	0	26	15	231	20	266	464
04:30 PM	30	0	0	30	19	118	12	149	32	0	0	32	12	291	28	331	542
04:45 PM	15	0	0	15	22	137	6	165	32	0	0	32	8	303	30	341	553
Total	126	0	0	126	67	498	30	595	111	0	0	111	72	1042	100	1214	2046
05:00 PM	37	0	0	37	10	143	7	160	23	0	0	23	23	322	30	375	595
05:15 PM	30	0	0	30	22	146	11	179	36	0	0	36	15	257	26	298	543
05:30 PM	39	0	0	39	20	145	3	168	34	0	0	34	23	262	14	299	540
05:45 PM	24	0	0	24	10	112	9	131	22	0	0	22	11	227	21	259	436
Total	130	0	0	130	62	546	30	638	115	0	0	115	72	1068	91	1231	2114
																	1
Grand Total	256	0	0	256	129	1044	60	1233	226	0	0	226	144	2110	191	2445	4160
Apprch %	100	0	0		10.5	84.7	4.9		100	0	0		5.9	86.3	7.8		
Total %	6.2	0	0	6.2	3.1	25.1	1.4	29.6	5.4	0	0	5.4	3.5	50.7	4.6	58.8	
Cars +	252	0	0	252	127	1020	60	1207	223	0	0	223	142	2051	191	2384	4066
% Cars +	98.4	0	0	98.4	98.4	97.7	100	97.9	98.7	0	0	98.7	98.6	97.2	100	97.5	97.7
Trucks	4	0	0	4	2	24	0	26	3	0	0	3	2	59	0	61	94
% Trucks	1.6	0	0	1.6	1.6	2.3	0	2.1	1.3	0	0	1.3	1.4	2.8	0	2.5	2.3



File Name : Rolesville(US 401 and Jonesville)PM Peak Site Code : Start Date : 11/9/2021 Page No : 2

		Jonesvi	lle Roa	d		US	401			Jonesvi	lle Roa	d		US	401		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	alysis Fro	om 04:0	0 PM to	o 05:45 F	M - Pea	k 1 of 1							-				
Peak Hour for	Entire In	tersecti	on Begi	ins at 04:	30 PM												
04:30 PM	30	0	0	30	19	118	12	149	32	0	0	32	12	291	28	331	542
04:45 PM	15	0	0	15	22	137	6	165	32	0	0	32	8	303	30	341	553
05:00 PM	37	0	0	37	10	143	7	160	23	0	0	23	23	322	30	375	595
05:15 PM	30	0	0	30	22	146	11	179	36	0	0	36	15	257	26	298	543
Total Volume	112	0	0	112	73	544	36	653	123	0	0	123	58	1173	114	1345	2233
% App. Total	100	0	0		11.2	83.3	5.5		100	0	0		4.3	87.2	8.5		
PHF	.757	.000	.000	.757	.830	.932	.750	.912	.854	.000	.000	.854	.630	.911	.950	.897	.938





File Name : Rolesville(US 401 and Eastern U Turn)AM Peak Site Code : Start Date : 11/9/2021 Page No : 1

			+ - Trucks	ps Printed- Cars +	Gro		
		US 401			US 401		
		Eastbound			Westbound		
Int. Total	App. Total	UTrn	Thru	App. Total	UTrn	Thru	Start Time
631	210	12	198	421	0	421	07:00 AM
570	160	24	136	410	0	410	07:15 AM
577	185	36	149	392	0	392	07:30 AM
433	154	17	137	279	0	279	07:45 AM
2211	709	89	620	1502	0	1502	Total
403	150	20	130	253	0	253	08:00 AM
354	111	13	98	243	0	243	08:15 AM
324	101	7	94	223	0	223	08:30 AM
241	94	9	85	147	0	147	08:45 AM
1322	456	49	407	866	0	866	Total
3533	1165	138	1027	2368	0	2368	Grand Total
		11.8	88.2		0	100	Apprch %
	33	3.9	29.1	67	0	67	Total %
3427	1109	136	973	2318	0	2318	Cars +
97	95.2	98.6	94.7	97.9	0	97.9	% Cars +
106	56	2	54	50	0	50	Trucks
3	4.8	1.4	5.3	2.1	0	2.1	% Trucks



File Name : Rolesville(US 401 and Eastern U Turn)AM Peak Site Code : Start Date : 11/9/2021 Page No : 2

		US 401			US 401		
		Westbound			Eastbound		
Start Time	Thru	UTrn	App. Total	Thru	UTrn	App. Total	Int. Total
Peak Hour Analysis From 07:00	O AM to 08:45 AM	1 - Peak 1 of 1					
Peak Hour for Entire Intersection	on Begins at 07:0	0 AM					
07:00 AM	421	0	421	198	12	210	631
07:15 AM	410	0	410	136	24	160	570
07:30 AM	392	0	392	149	36	185	577
07:45 AM	279	0	279	137	17	154	433
Total Volume	1502	0	1502	620	89	709	2211
% App. Total	100	0		87.4	12.6		
PHF	.892	.000	.892	.783	.618	.844	.876





File Name : Rolesville(US 401 and Eastern U Turn)PM Peak Site Code : Start Date : 11/9/2021 Page No : 1

		Gr	oups Printed- Cars	+ - Trucks			
		US 401			US 401		
		Westbound			Eastbound		
Start Time	Thru	UTrn	App. Total	Thru	UTrn	App. Total	Int. Total
04:00 PM	130	0	130	240	12	252	382
04:15 PM	128	0	128	237	15	252	380
04:30 PM	129	0	129	311	19	330	459
04:45 PM	149	0	149	317	19	336	485
Total	536	0	536	1105	65	1170	1706
05:00 PM	149	0	149	342	8	350	499
05:15 PM	160	0	160	284	19	303	463
05:30 PM	161	0	161	273	22	295	456
05:45 PM	120	0	120	235	12	247	367
Total	590	0	590	1134	61	1195	1785
Grand Total	1126	0	1126	2239	126	2365	3491
Apprch %	100	0		94.7	5.3		
Total %	32.3	0	32.3	64.1	3.6	67.7	
Cars +	1101	0	1101	2175	125	2300	3401
% Cars +	97.8	0	97.8	97.1	99.2	97.3	97.4
Trucks	25	0	25	64	1	65	90
% Trucks	2.2	0	2.2	2.9	0.8	2.7	2.6



File Name : Rolesville(US 401 and Eastern U Turn)PM Peak Site Code : Start Date : 11/9/2021 Page No : 2

		US 401			US 401		
		Westbound			Eastbound		
Start Time	Thru	UTrn	App. Total	Thru	UTrn	App. Total	Int. Total
Peak Hour Analysis From 04:00	0 PM to 05:45 PM	/I - Peak 1 of 1					
Peak Hour for Entire Intersection	on Begins at 04:3	0 PM					
04:30 PM	129	0	129	311	19	330	459
04:45 PM	149	0	149	317	19	336	485
05:00 PM	149	0	149	342	8	350	499
05:15 PM	160	0	160	284	19	303	463
Total Volume	587	0	587	1254	65	1319	1906
% App. Total	100	0		95.1	4.9		
PHF	.917	.000	.917	.917	.855	.942	.955





File Name : Rolesville(Jonesville and Mitchell Mill)AM Peak Site Code : Start Date : 11/30/2021 Page No : 1

Groups Printed- Cars + - Trucks

		Peeble	es Road	ł		Mitch			Peeble	es Road	l						
		South	bound			West			North	bound							
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
07:00 AM	4	17	13	34	8	73	5	86	6	11	3	20	0	74	1	75	215
07:15 AM	4	36	7	47	8	101	2	111	3	26	1	30	0	32	1	33	221
07:30 AM	6	34	5	45	16	87	3	106	0	24	0	24	1	33	1	35	210
07:45 AM	2	43	6	51	8	49	1	58	2	15	0	17	1	24	4	29	155
Total	16	130	31	177	40	310	11	361	11	76	4	91	2	163	7	172	801
08:00 AM	7	31	12	50	4	53	1	58	1	8	2	11	0	28	3	31	150
08:15 AM	12	17	3	32	1	37	1	39	1	7	0	8	1	24	1	26	105
08:30 AM	6	4	2	12	3	49	2	54	1	4	2	7	0	19	0	19	92
08:45 AM	1	13	3	17	4	32	1	37	1	3	1	5	1	18	2	21	80
Total	26	65	20	111	12	171	5	188	4	22	5	31	2	89	6	97	427
Grand Total	42	195	51	288	52	481	16	549	15	98	9	122	4	252	13	269	1228
Apprch %	14.6	67.7	17.7		9.5	87.6	2.9		12.3	80.3	7.4		1.5	93.7	4.8		
Total %	3.4	15.9	4.2	23.5	4.2	39.2	1.3	44.7	1.2	8	0.7	9.9	0.3	20.5	1.1	21.9	
Cars +	42	195	50	287	52	479	16	547	15	98	9	122	4	249	13	266	1222
% Cars +	100	100	98	99.7	100	99.6	100	99.6	100	100	100	100	100	98.8	100	98.9	99.5
Trucks	0	0	1	1	0	2	0	2	0	0	0	0	0	3	0	3	6
% Trucks	0	0	2	0.3	0	0.4	0	0.4	0	0	0	0	0	1.2	0	1.1	0.5



File Name : Rolesville(Jonesville and Mitchell Mill)AM Peak Site Code : Start Date : 11/30/2021 Page No : 2

		Peeble	s Road	1		Mitch	ell Mill			Peeble	s Road						
		South			West			North	bound								
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00 AM																	
07:00 AM	4	17	13	34	8	73	5	86	6	11	3	20	0	74	1	75	215
07:15 AM	4	36	7	47	8	101	2	111	3	26	1	30	0	32	1	33	221
07:30 AM	6	34	5	45	16	87	3	106	0	24	0	24	1	33	1	35	210
07:45 AM	2	43	6	51	8	49	1	58	2	15	0	17	1	24	4	29	155
Total Volume	16	130	31	177	40	310	11	361	11	76	4	91	2	163	7	172	801
% App. Total	9	73.4	17.5		11.1	85.9	3		12.1	83.5	4.4		1.2	94.8	4.1	-	
PHF	.667	.756	.596	.868	.625	.767	.550	.813	.458	.731	.333	.758	.500	.551	.438	.573	.906





File Name : Rolesville(Jonesville and Mitchell Mill)PM Peak Site Code : Start Date : 11/30/2021 Page No : 1

Groups Printed- Cars + - Trucks

		Peeble	es Road	ł		Mitch			Peeble	es Road	ł						
		South	nbound			West	bound			North	bound						
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
04:00 PM	7	11	13	31	6	25	1	32	1	14	1	16	2	44	6	52	131
04:15 PM	6	11	4	21	2	27	2	31	1	17	3	21	1	62	4	67	140
04:30 PM	3	13	3	19	4	30	2	36	0	27	1	28	3	64	3	70	153
04:45 PM	2	8	5	15	4	37	0	41	3	18	0	21	3	71	3	77	154
Total	18	43	25	86	16	119	5	140	5	76	5	86	9	241	16	266	578
05:00 PM	1	15	6	22	5	31	0	36	3	19	2	24	1	78	5	84	166
05:15 PM	3	15	6	24	4	23	0	27	3	26	1	30	4	89	7	100	181
05:30 PM	5	11	9	25	8	36	0	44	1	27	2	30	5	62	3	70	169
05:45 PM	1	7	4	12	2	21	1	24	2	13	2	17	4	55	6	65	118
Total	10	48	25	83	19	111	1	131	9	85	7	101	14	284	21	319	634
Grand Total	28	91	50	169	35	230	6	271	14	161	12	187	23	525	37	585	1212
Apprch %	16.6	53.8	29.6		12.9	84.9	2.2		7.5	86.1	6.4		3.9	89.7	6.3		
Total %	2.3	7.5	4.1	13.9	2.9	19	0.5	22.4	1.2	13.3	1	15.4	1.9	43.3	3.1	48.3	
Cars +	28	91	50	169	35	229	6	270	14	161	12	187	23	524	37	584	1210
% Cars +	100	100	100	100	100	99.6	100	99.6	100	100	100	100	100	99.8	100	99.8	99.8
Trucks	0	0	0	0	0	1	0	1	0	0	0	0	0	1	0	1	2
% Trucks	0	0	0	0	0	0.4	0	0.4	0	0	0	0	0	0.2	0	0.2	0.2



File Name : Rolesville(Jonesville and Mitchell Mill)PM Peak Site Code : Start Date : 11/30/2021 Page No : 2

		Peeble	s Road	ł		Mitch	ell Mill			Peeble	s Road	1					
		South			West	bound			North	bound							
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
eak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	2	8	5	15	4	37	0	41	3	18	0	21	3	71	3	77	154
05:00 PM	1	15	6	22	5	31	0	36	3	19	2	24	1	78	5	84	166
05:15 PM	3	15	6	24	4	23	0	27	3	26	1	30	4	89	7	100	181
05:30 PM	5	11	9	25	8	36	0	44	1	27	2	30	5	62	3	70	169
Total Volume	11	49	26	86	21	127	0	148	10	90	5	105	13	300	18	331	670
% App. Total	12.8	57	30.2		14.2	85.8	0		9.5	85.7	4.8		3.9	90.6	5.4		
PHF	.550	.817	.722	.860	.656	.858	.000	.841	.833	.833	.625	.875	.650	.843	.643	.828	.925



APPENDIX C

ADJACENT DEVELOPMENT INFORMATION

TRAFFIC IMPACT ANALYSIS

FOR

COBBLESTONE CROSSING MIXED-USE

LOCATED

IN

ROLESVILLE, NORTH CAROLINA

Prepared For: Town of Rolesville 502 Southtown Circle Rolesville, NC 27571

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

MARCH 2021



Prepared By: TF Reviewed By: MK

RKA Project No. 20498





9. RECOMMENDATIONS

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions. See a more detailed description of the recommended improvements below. Refer to Figure 14 for an illustration of the recommended lane configuration for the proposed development.

Improvements by STIP U-6241

STIP U-6241 is expected to realign Burlington Mills Road and install a traffic signal at the relocated intersection on Main Street. STIP U-6241 is also expected to provide improvements to the pedestrian and bike facilities along Main Street and add a concrete median island along Main Street west of Rogers Road. These improvements associated with STIP U-6241 will alter the existing lane configurations at the study intersections along Main Street.

Recommended Improvements by Developer

Main Street and Site Drive 1

- Construct the southbound approach with one ingress and two egress lanes.
- Provide stop control for the southbound approach.
- Install an eastbound left-turn lane with at least 125 feet of storage and appropriate decel and taper.

Young Street and Site Drive 2

- Construct the eastbound approach with one ingress and egress lane.
- Provide stop control for the eastbound approach.



Transportation Consulting forward.


Revised Traffic Impact Analysis for

Young Street PUD

Rolesville, North Carolina

Prepared for: Ashton Woods

Raleigh, North Carolina

Prepared by:

Kimley-Horn and Associates, Inc. NC License #F-0102 421 Fayetteville Street, Suite 600 Raleigh, NC 27601 (919) 677-2000

> June 2019 015956012





THIS DOCUMENT, TOGETHER WITH THE CONCEPTS AND DESIGNS PRESENTED HEREIN, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY KIMLEY-HORN AND ASSOCIATES, INC. SHALL BE WITHOUT LIABILITY TO KIMLEY-HORN AND ASSOCIATES, INC.





7.0 Recommendations

Residential Build-out

The following improvements are recommended to be performed to accommodate projected site traffic volumes at build-out of the residential portion of the development:

US 401 Bypass:

• Coordinate the traffic signals at the intersections of US 401 at Young Street and the Superstreet U-turns

Young Street at Quarry Road/North Site Driveway:

- Construct a northbound left-turn lane on Young Street with 100 feet of storage and appropriate tapers
- Construct a southbound right-turn lane on Young Street with 100 feet of storage and appropriate tapers
- Restripe the existing westbound left-turn lane on Quarry Road to a shared left/through lane
- Provide an exclusive left-turn lane with 275 feet of storage and appropriate tapers and a shared through/right lane on the North Site Driveway
- Install a traffic signal when warranted

Young Street at Central Site Driveway:

- Construct a northbound left-turn lane on Young Street with 100 feet of storage and appropriate tapers
- Construct a southbound right-turn lane on Young Street with 100 feet of storage and appropriate tapers
- Provide exclusive left and right-turn lanes on the Central Site Driveway with 125 feet of storage and appropriate tapers for the left-turn lane

Young Street at Rolesville High School Driveway/South Site Driveway:

- Construct a northbound left-turn lane on Young Street with 50 feet of storage and appropriate tapers
- Provide one egress lane on the South Site Driveway

Rolesville Road at Mitchell Mill Road:

• Install a traffic signal when warranted

Analyses indicate that with the recommended improvements in place, all of the study intersections except for Young Street at Century Farm Road and Young Street at Rolesville High School Driveway/South Site Driveway are expected to operate at an acceptable LOS at build-out of the residential-only phase of the development.

Kimley »Horn

Analyses indicate that the intersection of Young Street at Century Farm Road is expected to operate with long delays on the minor street approach (Century Farm Road) in the AM peak hour at project build-out. However, it is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours while the majority of the traffic moving through the intersection on the major street experiences little or no delay. SimTraffic traffic simulations indicate that no queuing issues are expected at this intersection.

Analyses indicate that the intersection of Young Street at the Rolesville High School Driveway/South Site Driveway is expected to operate with long delays on the minor street approach (Rolesville High School Driveway) in the AM peak hour and school PM peak hour with or without the proposed project in place in the study year 2025. SimTraffic traffic simulations also indicate the possibility of long queues on the westbound left-turn movement at this intersection in the AM peak hour and school PM peak hour. However, it is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. This intersection is not expected to meet 4-hour or 8-hour MUTCD traffic signal warrants.

Commercial Build-out

The following additional improvements are recommended to be performed in addition to those recommended above for the residential phase to accommodate projected site traffic volumes when the retail portion of the site is developed:

US 401 Bypass Eastbound at Young Street:

• Extend the storage of the existing eastbound right-turn lane on US 401 Bypass by approximately 175 feet to provide 400 feet of storage and appropriate tapers

Young Street at Quarry Road/North Site Driveway:

- Construct a northbound right-turn lane on Young Street with 100 feet of storage and appropriate tapers
- Modify the traffic signal to accommodate the additional laneage

Analyses indicate that with the recommended improvements in place, all of the study intersections except for Young Street at Century Farm Road, Young Street at the Central Site Driveway, and Young Street at Rolesville High School Driveway/South Site Driveway are expected to operate at acceptable LOS at commercial build-out of the development.

Analyses indicate that the intersection of Young Street at Century Farm Road is expected to operate with long delays on the minor street approach (Century Farm Road) in the AM peak hour at project build-out. It is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. SimTraffic

Kimley »Horn

traffic simulations indicate that short queues are likely on the minor street approach in the AM peak hour at commercial build-out.

Analyses indicate that the intersection of Young Street at the Central Site Driveway is expected to operate with long delays on the minor street approach (Central Site Driveway) in the AM peak hour in the commercial build-out traffic condition. It is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. SimTraffic traffic simulations indicate the possibility of long queues on the eastbound left-turn movement at this intersection in the AM peak hour in the commercial build-out condition.

Analyses indicate that the intersection of Young Street at the Rolesville High School Driveway/South Site Driveway is expected to operate with long delays on the minor street approach (Rolesville High School Driveway) in the AM peak hour and school PM peak hour with or without the proposed project in place in the study year 2025. SimTraffic traffic simulations also indicate the possibility of long queues on the westbound left-turn movement at this intersection in the AM peak hour and school PM peak hour. However, it is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. This intersection is not expected to meet 4-hour or 8-hour MUTCD traffic signal warrants.

As shown in the analysis, the impact of site traffic associated with the commercial build-out of this proposed PUD is generally consistent with the currently-approved PUD for the site. The proposed PUD is expected to generate no more than 50 additional peak hour trips in each of the studied peak hours compared to the approved PUD, and delays at commercial build-out of both plans are generally consistent at each of the study intersections.

The recommended laneage for the development is shown on Figure 17.



TRAFFIC IMPACT ANALYSIS

FOR

WHEELER TRACT

LOCATED

IN

ROLESVILLE, NC

Prepared For: Hopper Communities 173 Paraggi Court Clayton, NC 27527

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

June 2019



Prepared By: CAB Reviewed By: JTR

RKA Project No. 19045





9. **RECOMMENDATIONS**

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions. See a more detailed description of the recommended improvements below. Refer to Figure 11 for an illustration of the recommended lane configuration for the proposed development.

Recommended Improvements by Developer

Rolesville Road and Mitchell Mill Road

• Monitor intersection for signalization.

Rolesville Road and Site Drive 1

- Provide site access via a full movement intersection with one ingress lane and one egress lane.
- Provide stop control for westbound Site Drive 1 approach.
- Provide a designated southbound left-turn lane with at least 100 feet of storage and appropriate deceleration and taper.

Mitchell Mill Road and Site Drive 2

- Provide site access via a full movement intersection with one ingress lane and one egress lane.
- Provide stop control for southbound Site Drive 2 approach.





TRAFFIC IMPACT ANALYSIS

FOR

LOUISBURY ROAD ASSEMBLAGE

LOCATED

IN

RALEIGH, NC

Prepared For: McAdams Company 2905 Meridian Parkway Durham, NC 27713

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

andrew Ryle Rithe 04 SNGINEE 5/8/20

5/8/2020

May 2020

Prepared By: DT

Reviewed By: DR

RKA Project No. 19418





12. RECOMMENDATIONS

Based on the findings of this study, specific geometric improvements have been identified and are recommended to accommodate future traffic conditions. See a more detailed description of the recommended improvements below. Refer to Figure 9 for an illustration of the recommended lane configuration for the proposed development.

Recommended Improvements by Developer

Mitchell Mill Road and Louisbury Road

• Monitor for signalization after site is constructed.

US 401 and Louisbury Road

- Per NCDOT, extend northbound left turn lane to 175' of storage.
- Monitor for signalization after site is constructed.

Louisbury Road and Site Drive 1

- Provide site access via full movement intersection with one (1) ingress lane and one (1) egress lane.
- Per NCDOT, provide northbound left turn lane with 100' of storage.
- Provide stop control for eastbound approach.

Louisbury Road and Site Drive 2

- Provide site access via full movement intersection with one (1) ingress lane and one (1) egress lane.
- Provide stop control for eastbound approach.



Transportation Consulting that moves us forward.





Kalas / Watkins Family Property Traffic Impact Analysis

Rolesville Road, Rolesville, North Carolina

August 24, 2019

Prepared for:

Mitchell Mill Road Investors LLC PO Box 3557 Cary, NC 27519

Prepared by:

Stantec Consulting Services Inc.

801 Jones Franklin Road Suite 300 Raleigh, NC 27606

Sign-off Sheet

This document entitled Kalas / Watkins Family Property Traffic Impact Analysis was prepared by Stantec Consulting Services Inc. ("Stantec") for the account of Mitchell Mill Road Investors LLC (the "Client"). Any reliance on this document by any third party is strictly prohibited. The material in it reflects Stantec's professional judgment in light of the scope, schedule and other limitations stated in the document and in the contract between Stantec and the Client. The opinions in the document are based on conditions and information existing at the time the document was published and do not take into account any subsequent changes. In preparing the document, Stantec did not verify information supplied to it by others. Any use which a third party makes of this document is the responsibility of such third party. Such third party agrees that Stantec shall not be responsible for costs or damages of any kind, if any, suffered by it or any other third party as a result of decisions made or actions taken based on this document.

Prepared by

(signature)

Maggie Rogers

Reviewed by

2

(signature)

Matt Peach, PE, PTOE 2mg Approved by

(signature)

Christa Greene, PE



Introduction August 24, 2019

1.0 INTRODUCTION

The purpose of this report is to evaluate the transportation impacts of the proposed Kalas / Watkins Family Property development located on the west side of Rolesville Road just north of Mitchell Mill Road in Rolesville, NC. The project location is shown below in Figure 1.



Figure 1: Site Location

KALAS / WATKINS FAMILY PROPERTY TRAFFIC IMPACT ANALYSIS

Trip Generation and Distribution August 24, 2019



Figure 6: Site Trip Assignment



Traffic Analysis August 24, 2019

5.4 2025 BUILD WITH IMPROVEMENTS

Geometric improvements such as the installation of turn-lanes are recommended and therefore analyzed in this scenario. These items are listed below as well as in the recommendations section.

Rolesville Road at Site Driveway A

- Construct Driveway A as a full-movement access point onto Rolesville Road with one ingress lane and one egress lane.
- Construct an exclusive eastbound right-turn lane with 100 feet of full-width storage and appropriate taper on Driveway A.
- Construct an exclusive northbound left-turn lane with 100 feet of full-width storage and appropriate taper on Rolesville Road.
- Construct an exclusive southbound right-turn lane with 100 feet of full-width storage and appropriate taper on Rolesville Road.

Rolesville Road at Site Driveway B / Wheeler Tract Driveway

- Construct Driveway B as a full-movement access point onto Rolesville Road with one ingress lane and one egress lane.
- Construct an exclusive northbound left-turn lane with 100 feet of full-width storage and appropriate taper on Rolesville Road.
- Construct an exclusive southbound right-turn lane with 50 feet of full-width storage and appropriate taper on Rolesville Road.

Rolesville Road at Site Driveway C

- Construct Driveway C as a full-movement access point onto Rolesville Road with one ingress lane and one egress lane.
- Construct an exclusive eastbound right-turn lane with 100 feet of full-width storage and appropriate taper on Driveway C.
- Construct an exclusive northbound left-turn lane with 100 feet of full-width storage and appropriate taper on Rolesville Road.
- Construct an exclusive southbound right-turn lane with 100 feet of full-width storage and appropriate taper on Rolesville Road.

Accordingly, all study area intersections and approaches operate at acceptable levels of service with the following exceptions:

- The east and westbound approaches to the intersection of Rolesville Road at Rolesville High School Driveway / Young Street PUD Southern Driveway operates at LOS F in the AM peak hour. This causes high overall delays at the intersection. Furthermore, the eastbound approach operates at LOS F and westbound approach operates at LOS E in the PM peak hour.
- The east and westbound approaches at the intersection of Rolesville Road at Site Driveway B / Wheeler Tract Driveway operate at LOS E in the AM peak hour.

The east and westbound approaches to the intersection of Rolesville Road at Rolesville High School Driveway / Young Street PUD Southern Driveway performs unacceptably across analysis scenarios. These delays can be



KALAS / WATKINS FAMILY PROPERTY TRAFFIC IMPACT ANALYSIS

Traffic Analysis August 24, 2019

attributed to both the Young Street PUD and High School traffic on the side street approaches. The Kalas / Watkins development is projected to only add through volumes to the intersection and are anticipated to have a minimal impact on overall delays at this intersection.

Delays on the eastbound approach of Site Driveway B at Rolesville Road can be attributed to high thru volumes on Rolesville Road during the AM peak hour. Traffic volumes using this approach are anticipated to be minor (i.e. 15 vehicles in the AM peak hour and 10 vehicles in the PM peak hour) and side street delays should dissipate after High School Traffic passes through the network. Table 8 lists the results of the capacity analysis under the 2025 build-improved traffic conditions. The recommended improvements are illustrated in figure 14.



Figure 14: Recommended Improvements



APPENDIX D

CAPACITY ANALYSIS CALCULATIONS US 401 BYPASS & JONESVILLE ROAD

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		**	1						1		1	
Traffic Vol, veh/h	0	590	80	0	0	0	0	0	136	0	86	0
Future Vol, veh/h	0	590	80	0	0	0	0	0	136	0	86	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-
Veh in Median Storage	, # -	0	-	-	16983	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	656	89	0	0	0	0	0	151	0	96	0

Major/Minor N	/lajor1				Minor1		Ν	/linor2				
Conflicting Flow All	-	0	0		-	-	328	-	656	-		
Stage 1	-	-	-		-	-	-	-	0	-		
Stage 2	-	-	-		-	-	-	-	656	-		
Critical Hdwy	-	-	-		-	-	6.94	-	6.54	-		
Critical Hdwy Stg 1	-	-	-		-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-		-	-	-	-	5.54	-		
Follow-up Hdwy	-	-	-		-	-	3.32	-	4.02	-		
Pot Cap-1 Maneuver	0	-	-		0	0	668	0	384	0		
Stage 1	0	-	-		0	0	-	0	-	0		
Stage 2	0	-	-		0	0	-	0	460	0		
Platoon blocked, %		-	-									
Mov Cap-1 Maneuver	-	-	-		-	-	668	-	384	-		
Mov Cap-2 Maneuver	-	-	-		-	-	-	-	384	-		
Stage 1	-	-	-		-	-	-	-	-	-		
Stage 2	-	-	-		-	-	-	-	460	-		
Approach	EB				NB			SB				
HCM Control Delay, s	0				12			17.5				
HCM LOS					В			С				
Minor Lane/Major Mvm	t N	BLn1	EBT	EBR SBL	n1							
Capacity (veh/h)		668	-	- 3	84							
HCM Lane V/C Ratio	(0.226	-	- 0.2	49							
HCM Control Delay (s)		12	-	- 17	. 5							
HCM Lane LOS		В	-	-	С							
HCM 95th %tile Q(veh)		0.9	-	-	1							

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		**	*						1		1	
Traffic Vol, veh/h	0	873	88	0	0	0	0	0	150	0	95	0
Future Vol, veh/h	0	873	88	0	0	0	0	0	150	0	95	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-
Veh in Median Storage	, # -	0	-	-	16983	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	970	98	0	0	0	0	0	167	0	106	0

Major/Minor N	/lajor1			Minor1		Ν	/linor2			
Conflicting Flow All	- (0		-	-	485	-	970	-	
Stage 1		· -		-	-	-	-	0	-	
Stage 2				-	-	-	-	970	-	
Critical Hdwy				-	-	6.94	-	6.54	-	
Critical Hdwy Stg 1		· -		-	-	-	-	-	-	
Critical Hdwy Stg 2		· -		-	-	-	-	5.54	-	
Follow-up Hdwy				-	-	3.32	-	4.02	-	
Pot Cap-1 Maneuver	0.	· -		0	0	528	0	252	0	
Stage 1	0.	· -		0	0	-	0	-	0	
Stage 2	0.	· -		0	0	-	0	330	0	
Platoon blocked, %	•									
Mov Cap-1 Maneuver				-	-	528	-	252	-	
Mov Cap-2 Maneuver				-	-	-	-	252	-	
Stage 1				-	-	-	-	-	-	
Stage 2				-	-	-	-	330	-	
Approach	EB			NB			SB			
HCM Control Delay, s	0			14.9			29.2			
HCM LOS				В			D			
Minor Lane/Major Mvmt	t NBLn1	EBT	EBR SBLn1							
Capacity (veh/h)	528	-	- 252							
HCM Lane V/C Ratio	0.316	-	- 0.419							

HCM Lane V/C Ratio	0.316	-	- 0.419
HCM Control Delay (s)	14.9	-	- 29.2
HCM Lane LOS	В	-	- D
HCM 95th %tile Q(veh)	1.3	-	- 2

Intersection													
Int Delay, s/veh	5.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		† †	1						1		1		
Traffic Vol, veh/h	0	873	122	0	0	0	0	0	236	0	112	0	
Future Vol, veh/h	0	873	122	0	0	0	0	0	236	0	112	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	970	136	0	0	0	0	0	262	0	124	0	
	-			-	•	-	-			-		,	
Major/Minor M	aior1					Ν	linor1		Ν	linor?			
	սյու	0	0			ľ			/185	VIIIUIZ	070		
Stage 1	-	0	0				-	-	405	-	970	-	
Stage 2	-	-	-				-	-	-	-	070	-	
Critical Educy	-	-	-				-	-	601	-	970 6 54	-	
Critical Hduny Sta 1	-	-	-				-	-	0.94	-	0.04	-	
Critical Hdwy Sty 1	-	-	-				-	-	-	-	- 551	-	
	-	-	-				-	-	2 2 2	-	1 02	-	
Pollow-up Fluwy	-	-	-				-	-	0.02 500	-	4.UZ	-	
	0	-	-				0	0	520	0	Z:5Z	0	
Stage 2	0	-	-				0	0	-	0	220 -	0	
Olaye Z Diatoon blockod %	U	-	-				U	U	-	U	330	U	
May Cap 1 Manauver		-	-						500		2 ⊑0		
Nov Cap-1 Maneuver	-	-	-				-	-	JZ0	-	202	-	
	-	-	-				-	-	-	-	292	-	
Stage 1	-	-	-				-	-	-	-	220	-	
Stage 2	-	-	-				-	-	-	-	330	-	
Approach	EB						NB			SB			
HCM Control Delay, s	0						18.4			32.5			
HCM LOS							С			D			
Minor Lane/Maior Mvmt	Ν	NBLn1	EBT	EBR	SBLn1								
Capacity (veh/h)	-	528			252								
HCM Lane V/C Ratio		0 497	_	_	0 4 9 4								
HCM Control Delay (s)		18.4	_	_	32.5								
HCM Lane LOS		۲. ۲	_	_	<u>ס</u> ס								

HCM Lane LOS

HCM 95th %tile Q(veh)

С

2.7

D

2.5 -

--

-

Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		**	7						7		†	
Traffic Vol, veh/h	0	1220	59	0	0	0	0	0	125	0	37	0
Future Vol, veh/h	0	1220	59	0	0	0	0	0	125	0	37	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-
Veh in Median Storage,	# -	0	-	-	16983	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1356	66	0	0	0	0	0	139	0	41	0

Major/Minor M	1ajor1					Mino	or1		N	/linor2			
Conflicting Flow All	-	0	0				-	-	678	-	1356	-	
Stage 1	-	-	-				-	-	-	-	0	-	
Stage 2	-	-	-				-	-	-	-	1356	-	
Critical Hdwy	-	-	-				-	-	6.94	-	6.54	-	
Critical Hdwy Stg 1	-	-	-				-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-				-	-	-	-	5.54	-	
Follow-up Hdwy	-	-	-				-	-	3.32	-	4.02	-	
Pot Cap-1 Maneuver	0	-	-				0	0	395	0	148	0	
Stage 1	0	-	-				0	0	-	0	-	0	
Stage 2	0	-	-				0	0	-	0	216	0	
Platoon blocked, %		-	-										
Mov Cap-1 Maneuver	-	-	-				-	-	395	-	148	-	
Mov Cap-2 Maneuver	-	-	-				-	-	-	-	148	-	
Stage 1	-	-	-				-	-	-	-	-	-	
Stage 2	-	-	-				-	-	-	-	216	-	
Approach	EB					1	NB			SB			
HCM Control Delay, s	0						19			38.4			
HCM LOS							С			Е			
Minor Lane/Major Mvmt	: NE	3Ln1	EBT	EBR S	SBLn1								
Capacity (veh/h)		395	-	-	148								
HCM Lane V/C Ratio	0	.352	-	-	0.278								
HCM Control Delay (s)		19	-	-	38.4								
HCM Lane LOS		С	-	-	Е								
HCM 95th %tile Q(veh)		1.6	-	-	1.1								

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		**	1						1		1	
Traffic Vol, veh/h	0	1835	65	0	0	0	0	0	138	0	41	0
Future Vol, veh/h	0	1835	65	0	0	0	0	0	138	0	41	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-
Veh in Median Storage,	# -	0	-	-	16983	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	2039	72	0	0	0	0	0	153	0	46	0

Major/Minor	Major1				Minor1		M	Minor2			
Conflicting Flow All	-	0	0		-	-	1020	-	2039	-	
Stage 1	-	-	-		-	-	-	-	0	-	
Stage 2	-	-	-		-	-	-	-	2039	-	
Critical Hdwy	-	-	-		-	-	6.94	-	6.54	-	
Critical Hdwy Stg 1	-	-	-		-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-		-	-	-	-	5.54	-	
Follow-up Hdwy	-	-	-		-	-	3.32	-	4.02	-	
Pot Cap-1 Maneuver	0	-	-		0	0	234	0	56	0	
Stage 1	0	-	-		0	0	-	0	-	0	
Stage 2	0	-	-		0	0	-	0	99	0	
Platoon blocked, %		-	-								
Mov Cap-1 Maneuver	-	-	-		-	-	234	-	56	-	
Mov Cap-2 Maneuver	-	-	-		-	-	-	-	56	-	
Stage 1	-	-	-		-	-	-	-	-	-	
Stage 2	-	-	-		-	-	-	-	99	-	
Approach	EB				NB			SB			
HCM Control Delay, s	0				45.5			186.4			
HCM LOS					E			F			
Minor Lane/Major Mvm	it N	IBLn1	EBT	EBR SBLn1							
Capacity (veh/h)		234	-	- 56							
HCM Lane V/C Ratio		0.655	-	- 0.813							
HCM Control Delay (s)		45.5	-	- 186.4							

Intersection													
Int Delay, s/veh	17.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		† †	1						1		1		
Traffic Vol, veh/h	0	1835	117	0	0	0	0	0	192	0	67	0	
Future Vol, veh/h	0	1835	117	0	0	0	0	0	192	0	67	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	Yield	-	-	None	' <u>-</u>	' <u>-</u>	None	-	'-	None	
Storage Length	-	-	125	-	-	-	-	-	0	-	-	-	
Veh in Median Storage.	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade. %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles. %	2	2	2	2	2	2	2	2	2	2	2	2	
Mymt Flow	0	2039	130	0	0	0	0	0	213	0	74	0	
	•			•	•	·	·	•		·		·	
Maior/Minor M	laior1					Ν	/linor1		ſ	Minor2			
Conflicting Flow All		0	0				-	_	1020	-	2039	-	
Stage 1	-	-	-				-	_		-	0	-	
Stage 2	-	-	-				-	-	-	-	2039	-	
Critical Hdwy	-	_	-				-	-	6 94	-	6.54	-	
Critical Hdwy Sto 1	_	_	_				_	_	- 0.0	_	-	_	
Critical Hdwy Stg 7	_	_	_				_	_	_	_	5 54	_	
Follow-up Hdwy	_	_	_				_	_	3 32	_	4 02	_	
Pot Can-1 Maneuver	0	_	_				0	0	234	0	~ 56	0	
Stage 1	0 0	_	_				0	0 0	207	0	-	0	
Stage 2	0 0	_	_				0	0	_	0	QQ	0	
Platoon blocked %	U	_	_				U	U		U	55	U	
Mov Can-1 Maneuver	_	_	_				_	_	234	_	~ 56	_	
Mov Cap-1 Maneuver	_								204		~ 56	_	
Stane 1	-	-	-				-	-	-	-	50	-	
Stage 2	-	-	-				-	-	-	-	- 00	-	
Slaye Z	-	-	-				-	-	-	-	99	-	
Approach	EB						NB			SB			
HCM Control Delay s	0						82.4			\$ 353			
HCM LOS	•						F			F			
							-			-			
Minor Lane/Major Mvmt	t N	NBLn1	EBT	EBR	SBLn1								
Capacity (veh/h)		234	-	-	56								
HCM Lane V/C Ratio		0.912	-	-	1.329								
HCM Control Delay (s)		82.4	_	-	\$ 353								
HCM Lane LOS		۰2.7 F	-	-	φ 000 F								
HCM 95th %tile O(veh)		77	-	-	66								
		1.1	-	-	0.0								
Notes													

~: Volume exceeds capacity \$: Delay exceeds 300s

ds 300s +: Computation Not Defined *: All major volume in platoon

6

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					† †	1		1				1
Traffic Vol, veh/h	0	0	0	0	1352	185	0	36	0	0	0	225
Future Vol, veh/h	0	0	0	0	1352	185	0	36	0	0	0	225
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	1502	206	0	40	0	0	0	250

Major/Minor		1	Major2		Mi	inor1		Minor2				
Conflicting Flow All			-	-	0	-	1708	-	-	-	751	
Stage 1			-	-	-	-	0	-	-	-	-	
Stage 2			-	-	-	-	1708	-	-	-	-	
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94	
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-	
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32	
Pot Cap-1 Maneuver			0	-	-	0	90	0	0	0	353	
Stage 1			0	-	-	0	-	0	0	0	-	
Stage 2			0	-	-	0	145	0	0	0	-	
Platoon blocked, %				-	-							
Mov Cap-1 Maneuver			-	-	-	-	90	-	-	-	353	
Mov Cap-2 Maneuver			-	-	-	-	90	-	-	-	-	
Stage 1			-	-	-	-	-	-	-	-	-	
Stage 2			-	-	-	-	145	-	-	-	-	
Approach			WB			NB			SB			
HCM Control Delay, s			0			73.7			36.5			
HCM LOS						F			Е			
Minor Lane/Major Mvmt	NBLn1	WBT	WBR :	SBLn1								
Capacity (veh/h)	90	-	-	353								
HCM Lane V/C Ratio	0.444	-	-	0.708								
HCM Control Delay (s)	73.7	-	-	36.5								
HCM Lane LOS	F	-	-	E								
HCM 95th %tile Q(veh)	1.9	-	-	5.2								

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					**	7		1				۲
Traffic Vol, veh/h	0	0	0	0	1796	204	0	40	0	0	0	248
Future Vol, veh/h	0	0	0	0	1796	204	0	40	0	0	0	248
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	1996	227	0	44	0	0	0	276

Major/Minor		Ν	Major2			Minor1			Vinor2					
Conflicting Flow All			-	-	0	-	2223	-	-	-	998			
Stage 1			-	-	-	-	0	-	-	-	-			
Stage 2			-	-	-	-	2223	-	-	-	-			
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94			
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-			
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-			
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32			
Pot Cap-1 Maneuver			0	-	-	0	~ 43	0	0	0	~ 242			
Stage 1			0	-	-	0	-	0	0	0	-			
Stage 2			0	-	-	0	79	0	0	0	-			
Platoon blocked, %				-	-									
Mov Cap-1 Maneuver			-	-	-	-	~ 43	-	-	-	~ 242			
Mov Cap-2 Maneuver			-	-	-	-	~ 43	-	-	-	-			
Stage 1			-	-	-	-	-	-	-	-	-			
Stage 2			-	-	-	-	79	-	-	-	-			
Approach			WB			NB			SB					
HCM Control Delay, s			0			293.8			143.8					
HCM LOS						F			F					
Minor Lane/Major Mvmt	NBLn1	WBT	WBR SE	3Ln1										
Capacity (veh/h)	43	-	-	242										
HCM Lane V/C Ratio	1.034	-	- 1	.139										
HCM Control Delay (s)	293.8	-	- 1	43.8										
HCM Lane LOS	F	-	-	F										
HCM 95th %tile Q(veh)	4.2	-	-	12.5										
Notes														
~: Volume exceeds capacity	\$: De	lay exc	eeds 300)s	+: Com	putatior	Not De	efined	*: All	major v	o l ume ir	n platoon		

Intersection

Int Delay, s/veh	23.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					- 11	1		†				1
Traffic Vol, veh/h	0	0	0	0	1853	204	0	40	0	0	0	248
Future Vol, veh/h	0	0	0	0	1853	204	0	40	0	0	0	248
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage,	# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	2059	227	0	44	0	0	0	276

Major/Minor		I	Major2		Ν	/linor1		Ν	/linor2					
Conflicting Flow All			-	-	0	-	2286	-	-	-	1030			_
Stage 1			-	-	-	-	0	-	-	-	-			
Stage 2			-	-	-	-	2286	-	-	-	-			
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94			
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-			
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-			
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32			
Pot Cap-1 Maneuver			0	-	-	0	~ 39	0	0	0	~ 231			
Stage 1			0	-	-	0	-	0	0	0	-			
Stage 2			0	-	-	0	74	0	0	0	-			
Platoon blocked, %				-	-									
Mov Cap-1 Maneuver			-	-	-	-	~ 39	-	-	-	~ 231			
Mov Cap-2 Maneuver			-	-	-	-	~ 39	-	-	-	-			
Stage 1			-	-	-	-	-	-	-	-	-			
Stage 2			-	-	-	-	74	-	-	-	-			
Approach			WB			NB			SB					
HCM Control Delay, s			0		\$	348.5			165.2					_
HCM LOS			Ū		Ŧ	F			F					
Minor Long/Major Mymt	NDI p1			201 p1										
		VVDI	WDRC											 —
Capacity (ven/n)	39	-	-	231										
HCM Lane V/C Ratio	1.14	-	-	1.193										
HCM Control Delay (s)	\$ 348.5 F	-	-	165.2										
	F	-	-	100										
HUM 95th %tile Q(veh)	4.4	-	-	13.3										
Notes														_
~: Volume exceeds capacity	/ \$: De	elay ex	ceeds 3	00s	+: Com	putatic	on Not I	Defined	*: All	l major	volume	in plato	on	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					**	1		1				7
Traffic Vol, veh/h	0	0	0	0	555	74	0	116	0	0	0	114
Future Vol, veh/h	0	0	0	0	555	74	0	116	0	0	0	114
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	617	82	0	129	0	0	0	127

Major/Minor		1	Major2		М	inor1		М	Minor2			
Conflicting Flow All			-	-	0	-	699	-	-	-	309	
Stage 1			-	-	-	-	0	-	-	-	-	
Stage 2			-	-	-	-	699	-	-	-	-	
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94	
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-	
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32	
Pot Cap-1 Maneuver			0	-	-	0	362	0	0	0	687	
Stage 1			0	-	-	0	-	0	0	0	-	
Stage 2			0	-	-	0	440	0	0	0	-	
Platoon blocked, %				-	-							
Mov Cap-1 Maneuver			-	-	-	-	362	-	-	-	687	
Mov Cap-2 Maneuver			-	-	-	-	362	-	-	-	-	
Stage 1			-	-	-	-	-	-	-	-	-	
Stage 2			-	-	-	-	440	-	-	-	-	
Approach			WB			NB			SB			
HCM Control Delay, s			0			20.3			11.4			
HCM LOS						С			В			
Minor Lane/Major Mvmt	NBLn1	WBT	WBR 3	SBLn1								
Capacity (veh/h)	362	-	-	687								
HCM Lane V/C Ratio	0.356	-	-	0.184								
HCM Control Delay (s)	20.3	-	-	11.4								
HCM Lane LOS	С	-	-	В								
HCM 95th %tile Q(veh)	1.6	-	-	0.7								
6.1

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					† †	1		1				7
Traffic Vol, veh/h	0	0	0	0	843	82	0	128	0	0	0	126
Future Vol, veh/h	0	0	0	0	843	82	0	128	0	0	0	126
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	937	91	0	142	0	0	0	140

Major/Minor		1	Major2		Mi	inor1		М	inor2			
Conflicting Flow All			-	-	0	-	1028	-	-	-	469	
Stage 1			-	-	-	-	0	-	-	-	-	
Stage 2			-	-	-	-	1028	-	-	-	-	
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94	
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-	
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32	
Pot Cap-1 Maneuver			0	-	-	0	233	0	0	0	541	
Stage 1			0	-	-	0	-	0	0	0	-	
Stage 2			0	-	-	0	310	0	0	0	-	
Platoon blocked, %				-	-							
Mov Cap-1 Maneuver			-	-	-	-	233	-	-	-	541	
Mov Cap-2 Maneuver			-	-	-	-	233	-	-	-	-	
Stage 1			-	-	-	-	-	-	-	-	-	
Stage 2			-	-	-	-	310	-	-	-	-	
Approach			WB			NB			SB			
HCM Control Delay, s			0			42			14			
HCM LOS						Е			В			
Minor Lane/Major Mvmt	NBLn1	WBT	WBR	SBLn1								
Capacity (veh/h)	233	-	-	541								
HCM Lane V/C Ratio	0.61	-	-	0.259								
HCM Control Delay (s)	42	-	-	14								
HCM Lane LOS	Е	-	-	В								
HCM 95th %tile Q(veh)	3.6	-	-	1								

Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					^	1		1				1
Traffic Vol, veh/h	0	0	0	0	879	82	0	128	0	0	0	126
Future Vol, veh/h	0	0	0	0	879	82	0	128	0	0	0	126
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	150	-	-	-	-	-	0
Veh in Median Storage,	,# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	0	977	91	0	142	0	0	0	140

Major/Minor		Ν	/lajor2		М	inor1		Μ	inor2			
Conflicting Flow All			-	-	0	-	1068	-	-	-	489	_
Stage 1			-	-	-	-	0	-	-	-	-	
Stage 2			-	-	-	-	1068	-	-	-	-	
Critical Hdwy			-	-	-	-	6.54	-	-	-	6.94	
Critical Hdwy Stg 1			-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2			-	-	-	-	5.54	-	-	-	-	
Follow-up Hdwy			-	-	-	-	4.02	-	-	-	3.32	
Pot Cap-1 Maneuver			0	-	-	0	220	0	0	0	525	
Stage 1			0	-	-	0	-	0	0	0	-	
Stage 2			0	-	-	0	296	0	0	0	-	
Platoon blocked, %				-	-							
Mov Cap-1 Maneuver			-	-	-	-	220	-	-	-	525	
Mov Cap-2 Maneuver			-	-	-	-	220	-	-	-	-	
Stage 1			-	-	-	-	-	-	-	-	-	
Stage 2			-	-	-	-	296	-	-	-	-	
Approach			WB			NB			SB			
HCM Control Delay, s			0			47.1			14.3			_
HCM LOS						Е			В			
Minor Lane/Major Mvmt	NBLn1	WBT	WBR S	SBLn1								
Capacity (veh/h)	220	-	-	525								
HCM Lane V/C Ratio	0.646	-	-	0.267								

14.3

В

-

_

- 1.1

HCM Control Delay (s)

HCM 95th %tile Q(veh)

HCM Lane LOS

47.1

Е

3.9

-

-

-

APPENDIX E

CAPACITY ANALYSIS CALCULATIONS US 401 BYPASS & EASTERN U-TURN LOCATION

Int Delay, s/veh

Int Delay, s/veh	1.3						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations				**	7		
Traffic Vol, veh/h	0	0	0	1532	91	0	
Future Vol, veh/h	0	0	0	1532	91	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage	e, # 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	0	0	1702	101	0	

Major/Minor	Major2	Minor1		
Conflicting Flow All	-	- 851	-	
Stage 1	-	- 0	-	
Stage 2	-	- 851	-	
Critical Hdwy	-	- 6.84	-	
Critical Hdwy Stg 1	-		-	
Critical Hdwy Stg 2	-	- 5.84	-	
Follow-up Hdwy	-	- 3.52	-	
Pot Cap-1 Maneuver	0	- 299	0	
Stage 1	0		0	
Stage 2	0	- 379	0	
Platoon blocked, %		-		
Mov Cap-1 Maneuver	-	- 299	-	
Mov Cap-2 Maneuver	-	- 299	-	
Stage 1	-		-	
Stage 2	-	- 379	-	
Approach	WB	NB		
HCM Control Delay, s	0	23.1		
HCM LOS		С		

Minor Lane/Major Mvmt	NBLn1	WBT
Capacity (veh/h)	299	-
HCM Lane V/C Ratio	0.338	-
HCM Control Delay (s)	23.1	-
HCM Lane LOS	С	-
HCM 95th %tile Q(veh)	1.4	-

Int Delay, s/veh	2						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations				**	5		
Traffic Vol, veh/h	0	0	0	1994	100	0	
Future Vol, veh/h	0	0	0	1994	100	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage	, # 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	0	0	2216	111	0	

Major/Minor	Ν	/lajor2	Ν	/linor1					
Conflicting Flow All		-	-	1108	-		 		
Stage 1		-	-	0	-				
Stage 2		-	-	1108	-				
Critical Hdwy		-	-	6.84	-				
Critical Hdwy Stg 1		-	-	-	-				
Critical Hdwy Stg 2		-	-	5.84	-				
Follow-up Hdwy		-	-	3.52	-				
Pot Cap-1 Maneuver		0	-	204	0				
Stage 1		0	-	-	0				
Stage 2		0	-	278	0				
Platoon blocked, %			-						
Mov Cap-1 Maneuver		-	-	204	-				
Mov Cap-2 Maneuver		-	-	204	-				
Stage 1		-	-	-	-				
Stage 2		-	-	278	-				
Approach		WB		NB					
HCM Control Delay		0		/1 0					
HCM LOS		0		41.9 E					
				E					
Minor Lane/Major Mvmt	NBLn1	WBT							
Capacity (veh/h)	204	-							

HCM Lane V/C Ratio	0.545	-	
HCM Control Delay (s)	41.9	-	
HCM Lane LOS	Е	-	
HCM 95th %tile Q(veh)	2.9	-	

Intersection							
Int Delay, s/veh	5.9						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations				- 44	٦		
Traffic Vol, veh/h	0	0	0	2011	157	0	
Future Vol, veh/h	0	0	0	2011	157	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# 2	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	0	0	2234	174	0	
Maior/Minor		ľ	Maior2	N	/linor1		
Conflicting Flow All			10	'	1117	_	
Stage 1			-	_	0	_	
Stage 2			-	_	1117	_	
Critical Hdwy			-	_	6 84	-	
Critical Hdwy Stg 1			-	_	-	_	
Critical Hdwy Stg 2			-	_	5 84	_	
Follow-up Hdwy			-	_	3.52	_	
Pot Cap-1 Maneuver			0	_	201	0	
Stage 1			0	_		0	
Stage 2			0	_	275	0	
Platoon blocked %			Ŭ	_	210	Ũ	
Mov Cap-1 Maneuver			-	_	201	-	
Mov Cap-2 Maneuver			-	-	201	-	
Stage 1			-	_		_	
Stage 2			-	_	275	_	
olago 2					210		
Approach			WR		NR		
HCM Control Delay			0		81 9		
HCM LOS			0		51.5 F		
					I		
Minor Lane/Major Mymt	,	NRI n1	W/RT				
	1	204					
Capacity (Ven/II)		201	-				
		0100	-				
		01.9 Г	-				
		F C C	-				
		0.0	-				

Int Delay, s/veh	1.1						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations				**	7		
Traffic Vol, veh/h	0	0	0	600	66	0	
Future Vol, veh/h	0	0	0	600	66	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	0	0	667	73	0	

Major/Minor	Ν	/lajor2	I	Vinor1		
Conflicting Flow All		-	-	334	-	
Stage 1		-	-	0	-	
Stage 2		-	-	334	-	
Critical Hdwy		-	-	6.84	-	
Critical Hdwy Stg 1		-	-	-	-	
Critical Hdwy Stg 2		-	-	5.84	-	
Follow-up Hdwy		-	-	3.52	-	
Pot Cap-1 Maneuver		0	-	636	0	
Stage 1		0	-	-	0	
Stage 2		0	-	697	0	
Platoon blocked, %			-			
Mov Cap-1 Maneuver		-	-	636	-	
Mov Cap-2 Maneuver		-	-	636	-	
Stage 1		-	-	-	-	
Stage 2		-	-	697	-	
Approach		WB		NB		
HCM Control Delay, s		0		11.4		
HCM LOS				В		
Minor Lane/Major Mvmt	NBLn1	WBT				
Capacity (veh/h)	636	-				
HCM Lane V/C Ratio	0.115	-				

HCM Control Delay (s)	11.4	-
HCM Lane LOS	В	-
HCM 95th %tile Q(veh)	04	-

Int Delay, s/veh	1							
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				**	5			
Traffic Vol, veh/h	0	0	0	892	73	0		
Future Vol, veh/h	0	0	0	892	73	0		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	-	-	-	0	-		
Veh in Median Storage	, # 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	0	0	0	991	81	0		

Major/Minor	Major2	Minor1		
Conflicting Flow All	-	- 496	-	
Stage 1	-	- 0	-	
Stage 2	-	- 496	-	
Critical Hdwy	-	- 6.84	-	
Critical Hdwy Stg 1	-		-	
Critical Hdwy Stg 2	-	- 5.84	-	
Follow-up Hdwy	-	- 3.52	-	
Pot Cap-1 Maneuver	0	- 503	0	
Stage 1	0		0	
Stage 2	0	- 577	0	
Platoon blocked, %		-		
Mov Cap-1 Maneuver	-	- 503	-	
Mov Cap-2 Maneuver	-	- 503	-	
Stage 1	-		-	
Stage 2	-	- 577	-	
Approach	WB	NB		
HCM Control Delay s	0	13.5		
HCM LOS	Ū	B		
		_		

Minor Lane/Major Mvmt	NBLn1	WBT	
Capacity (veh/h)	503	-	
HCM Lane V/C Ratio	0.161	-	
HCM Control Delay (s)	13.5	-	
HCM Lane LOS	В	-	
HCM 95th %tile Q(veh)	0.6	-	

Int Delay, s/veh 1.6 Movement EBT EBR WBL WBT NBL NBR Lane Configurations M N Image: Configurations M N Lane Configurations M N Traffic Vol, veh/h 0 0 918 109 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Storage Length - - 0 0 - - 0 0 - Storage Length - - 0 0 - - 0 0 - Veh in Median Storage, # 2	Intersection						
Movement EBT EBR WBL WBT NBL NBR Lane Configurations	Int Delay, s/veh	1.6					
Lane Configurations Image: Configurations Image: Configurations Image: Configurations Traffic Vol, veh/h 0 0 918 109 0 Future Vol, veh/h 0 0 0 918 109 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Stop Stop Free None - Veh in Median Storage, # 2 - 0 0 - - Grade, % 0 - - 0 0 - - Veh in Median Storage, # 2 2 2 2 2 2 2 Work Flow 0 0 0 1020 121 0 - - Mejor/Minor Major2 Minor1 - 510 -	Movement	EBT	EBR	WBL	WBT	NBL	NBR
Traffic Vol, veh/h 0 0 918 109 0 Future Vol, veh/h 0 0 918 109 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Stop Stop Stop Stop RT Channelized - None - None - None - None - None Storage Length - - 0 0 - Grade, % Q 0 - 0 0 - Grade, % 0 - 0 - Peak Hour Factor 90 90 90 90 90 90 90 90 90 Heavy Vehicles, % 2 <td>Lane Configurations</td> <td></td> <td></td> <td></td> <td>††</td> <td>1</td> <td></td>	Lane Configurations				† †	1	
Future Vol, veh/h 0 0 0 918 109 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Stop Free Free Free Stop Stop Rece None None Storage Length - - - 0 0 - Veh in Median Storage, # 2 - - 0 0 - Grade, % 0 - - 0 0 - - Peak Hour Factor 90 90 90 90 90 90 90 Heavy Vehicles, % 2	Traffic Vol, veh/h	0	0	0	918	109	0
Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Stop Stop RT Channelized - None - None Storage Length - - 0 - Grade, % 0 - - 0 0 Grade, % 0 - - 0 0 Peak Hour Factor 90 90 90 90 90 Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Mymt Flow 0 0 0 1020 121 0 Major/Minor Major2 Minor1 - - 510 - Conflicting Flow All - - 510 - - - Citical Hdwy Stg 1 - - - - - - - Critical Hdwy Stg 2 - - 5.84 - - - 0 - <td>Future Vol, veh/h</td> <td>0</td> <td>0</td> <td>0</td> <td>918</td> <td>109</td> <td>0</td>	Future Vol, veh/h	0	0	0	918	109	0
Sign Control Stop Stop Free Free Stop Stop RT Channelized - None - None - None Storage Length - - 0 0 - - 0 0 Grade, % 0 - - 0 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 90 90 90 90 Heary Vehicles, % 2 3 3 3 3 3	Conflicting Peds, #/hr	0	0	0	0	0	0
RŤ Channelized - None - None Storage Length - - 0 - Veh in Median Storage, # 2 - 0 0 Grade, % 0 - 0 0 - Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Major/Minor Major2 Minor1 - 510 - Conflicting Flow All - - 510 - Stage 1 - - 6.84 - - Critical Hdwy - - 5.84 - - Follow-up Hdwy - - - 0 - - Pot Cap-1 Maneuver 0 - 493 0 - - 0 -	Sign Control	Stop	Stop	Free	Free	Stop	Stop
Storage Length - - - 0 - Veh in Median Storage, # 2 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 Peak Hour Factor 90 Peak Hour Factor 90 121 0 Major/Minor Major2 Minor1 - - 510 - - - 510 - - 510 -	RT Channelized	-	None	-	None	· -	None
Veh in Median Storage, # 2 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 90 Peak Hour Factor 90 90 90 90 90 90 Major/Minor Major2 2 2 2 2 2 Minor1 Conflicting Flow All - - 510 - Conflicting Flow All - - 510 - Stage 1 - - 510 - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 493 0 Stage 1 0 - - 0 568 0 Ploton blocked, % - - Mov Cap-1 Maneuver - - 493 - - - - Mov Cap-2 Maneuver	Storage Length	-	-	-	-	0	-
Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 2 Mymt Flow 0 0 0 1020 121 0 Major/Minor Major2 Minor1 - - 0 - Stage 1 - - 0 - - - Stage 2 - - 510 - - - Critical Hdwy - - 6.84 - - - - Critical Hdwy Stg 2 - - 5.84 -<	Veh in Median Storage	, # 2	-	-	0	0	-
Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Mymt Flow 0 0 0 1020 121 0 Major/Minor Major2 Minor1 Conflicting Flow All - - 510 - Stage 1 - - 0 - Stage 2 - - 510 - Critical Hdwy - - 6.84 - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - 5.84 Follow-up Hdwy - - 3.52 Pot Cap-1 Maneuver 0 - 0 Stage 1 0 - - Stage 2 0 - 568 0 Platoon blocked, % - - - Mov Cap-1 Maneuver - - - Stage 1 - - - Stage 1 - - - Stage 1 - - - Stage 2 - 568 - Mov Cap-1 Maneuver - - -	Grade, %	0	-	-	0	0	-
Heavy Vehicles, % 2 3	Peak Hour Factor	90	90	90	90	90	90
Mvmt Flow 0 0 0 1020 121 0 Major/Minor Major2 Minor1 Conflicting Flow All - - 510 - Stage 1 - 0 - Stage 2 - 0 - Critical Hdwy - - 6.84 - - - - Critical Hdwy Stg 1 - - - - - - - Follow-up Hdwy - - 3.52 -	Heavy Vehicles, %	2	2	2	2	2	2
Major/Minor Major2 Minor1 Conflicting Flow All - 510 - Stage 1 - 0 - Stage 2 - -510 - Critical Hdwy - -6.84 - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - 5.84 - Colland Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - - Mov Cap-2 Maneuver - 493 - - Stage 1 - - - - - Stage 2 - - 568 - - Mov Cap-2 Maneuver - 493	Mvmt Flow	0	0	0	1020	121	0
Major/Minor Major2 Minor1 Conflicting Flow All - 510 - Stage 1 - 0 - Stage 2 - 510 - Critical Hdwy - 6.84 - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - 5.84 - Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - - Mov Cap-2 Maneuver - 493 - - Stage 1 - - - - - Stage 2 - - 568 - - More Cap-2 Maneuver - - -							
Magentation Magentation Conflicting Flow All - - 510 - Stage 1 - 0 - Stage 2 - 510 - Critical Hdwy - - 6.84 -	Major/Minor		N	Jaior?	N	linor1	
Stage 1 - - 510 - Stage 2 - - 510 - Critical Hdwy - - 6.84 - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - 5.84 - Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platon blocked, % - - - Mov Cap-2 Maneuver - 493 - Stage 1 - - - - Stage 2 - - 568 - Mov Cap-2 Maneuver - - - - Stage 2 - - 568 - McM Control Delay, s 0 14.7 - HCM Control Delay (s) 14.7 - <td>Conflicting Flow All</td> <td></td> <td>ľ</td> <td>najuiz</td> <td>ľ</td> <td>510</td> <td></td>	Conflicting Flow All		ľ	najuiz	ľ	510	
Stage 2 - - 510 - Critical Hdwy Stg 1 - - 6.84 - Critical Hdwy Stg 2 - - 5.84 - Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - Mov Cap-1 Maneuver - 493 - Mov Cap-2 Maneuver - 493 - Stage 1 - - - - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - HCM Control Delay, s 0 14.7 - - Capacity (veh/h) 493 - - - HCM Lane V/C Ratio 0.246	Stane 1			-	-	010	-
Orage 2 - </td <td>Stage 2</td> <td></td> <td></td> <td>-</td> <td>-</td> <td>510</td> <td>-</td>	Stage 2			-	-	510	-
Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Mov Cap-2 Maneuver - - - - Stage 2 - - 568 - Approach WB NB - - - HCM Control Delay, s 0 14.7 - - HCM LOS B - - - - Minor Lane/Major Mvmt NBLn1 WBT - - - <td>Critical Eduar</td> <td></td> <td></td> <td>-</td> <td>-</td> <td>6.84</td> <td>-</td>	Critical Eduar			-	-	6.84	-
Critical Hdwy Stg 2 - - 5.84 - Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - - HCM Control Delay, s 0 14.7 - - HCM LOS B - - - - Minor Lane/Major Mvmt NBLn1 WBT - - -	Critical Hdwy Sta 1			-	-	0.04	-
Follow-up Hdwy - - 3.52 - Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - 0 Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - HCM Control Delay, s 0 14.7 - HCM LOS B - - - Minor Lane/Major Mvmt NBLn1 WBT - - Capacity (veh/h) 493 - - - HCM Lane V/C Ratio 0.246 - - - HCM Lane LOS B - - - - HCM Stib % vite O(veh)	Critical Hdwy Stg 1			-	-	- 5 8/	-
Pot Cap-1 Maneuver 0 - 493 0 Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - 493 - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - HCM Control Delay, s 0 14.7 - HCM LOS B - - - Minor Lane/Major Mvmt NBLn1 WBT - - Capacity (veh/h) 493 - - - HCM Lane V/C Ratio 0.246 - - - HCM Control Delay (s) 14.7 - - - HCM Control Delay (s) 14.7 - - - HCM Control Delay (s) 14.7 - - -						3.52	_
Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 1 - - - - Stage 2 - - 568 - Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - HCM Control Delay, s 0 14.7 - HCM Los B - - - Minor Lane/Major Mvmt NBLn1 WBT - - Capacity (veh/h) 493 - - - HCM Lane LOS B - - - HCM Lane LOS B - -	Pot Can_1 Maneuver			0		/02	0
Stage 1 0 - - 0 Stage 2 0 - 568 0 Platoon blocked, % - - - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB - - HCM Control Delay, s 0 14.7 - HCM LOS B - - - Minor Lane/Major Mvmt NBLn1 WBT - - Capacity (veh/h) 493 - - - HCM Lane V/C Ratio 0.246 - - - HCM Lane LOS B - - - - HCM Lane LOS B - - - -	Stane 1			0		400	0
Stage 2 0 - 300 0 Platoon blocked, % - - 493 - Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB NB NB HCM Control Delay, s 0 14.7 - HCM LOS B - - Minor Lane/Major Mvmt NBLn1 WBT - Capacity (veh/h) 493 - - HCM Lane V/C Ratio 0.246 - - HCM Control Delay (s) 14.7 - - HCM Lane LOS B - - HCM Stitle Q(veh) 1 - -	Stage 2			0		568	0
Mov Cap-1 Maneuver - - 493 - Mov Cap-2 Maneuver - - 493 - Stage 1 - - - - Stage 2 - - 568 - Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 HCM Lane V/C Ratio 0.246 HCM Control Delay (s) 14.7	Platoon blocked %			U	_	000	U
Mov Cap-2 Maneuver - - 493 Stage 1 - - - Stage 2 - - 568 Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 -	Mov Can-1 Maneuver			_	_	493	_
Stage 1 - - - Stage 2 - - 568 Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 -	Mov Cap-2 Maneuver			_	_	493	_
Stage 2 - - 568 Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B -	Stane 1			_	_		_
Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Los 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B -	Stage 2			-	-	568	-
Approach WB NB HCM Control Delay, s 0 14.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B -	Oldye Z			-	-	500	-
HCM Control Delay, s HCM Control Delay, s HCM LOS HCM LOS HCM LOS HCM Los HCM Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM 25th %tile O(veh) 1 -	Approach			WR		NR	
HCM LOS B Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM Lane LOS B -	HCM Control Delay			0		14.7	
Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM 95th %tile O(veh) 1 -	HCM LOS			0		/ R	
Minor Lane/Major Mvmt NBLn1 WBT Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM Stile Q(veh) 1 -						5	
Capacity (veh/h) 493 - HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM 95th % tile Q(veh) 1 -	Minor Lane/Maior Mym	t I	NBI n1	WRT			
HCM Lane V/C Ratio 0.246 - HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM 95th %tile Q(yeb) 1 -	Canacity (veh/h)		/0?				
HCM Control Delay (s) 14.7 - HCM Lane LOS B - HCM 95th % tile Q(yeb) 1 -	HCM Lang V/C Datio		1 2/C N	-			
HCM Softe Delay (5) 14.7 - HCM Lane LOS B - HCM 95th % tile O(veh) 1 -	HCM Control Delay (a)		0.240 1/1 7	-			
HCM 95th %tile O(veh) 1 -	HCM Lane LOS		1.+. <i>1</i> R	-			
	HCM 95th %tile O(veh)		1	-			

APPENDIX F

CAPACITY ANALYSIS CALCULATIONS MITCHELL MILL ROAD & JONESVILLE ROAD / PEEBLES ROAD

Intersection Delay, s/veh Intersection LOS

```
eh 12.7
B
```

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	7	166	4	11	316	41	4	78	11	32	133	16
Future Vol, veh/h	7	166	4	11	316	41	4	78	11	32	133	16
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	8	184	4	12	351	46	4	87	12	36	148	18
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	10.8			14.9			10.1			11.4		
HCM LOS	В			В			В			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	4%	4%	3%	18%
Vol Thru, %	84%	94%	86%	73%
Vol Right, %	12%	2%	11%	9%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	93	177	368	181
LT Vol	4	7	11	32
Through Vol	78	166	316	133
RT Vol	11	4	41	16
Lane Flow Rate	103	197	409	201
Geometry Grp	1	1	1	1
Degree of Util (X)	0.168	0.297	0.577	0.318
Departure Headway (Hd)	5.85	5.433	5.079	5.696
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	612	661	711	629
Service Time	3.904	3.477	3.114	3.743
HCM Lane V/C Ratio	0.168	0.298	0.575	0.32
HCM Control Delay	10.1	10.8	14.9	11.4
HCM Lane LOS	В	В	В	В
HCM 95th-tile Q	0.6	1.2	3.7	1.4

F

Intersection

Intersection Delay, s/veh Intersection LOS

50.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	8	240	4	12	569	45	4	86	12	35	147	18
Future Vol, veh/h	8	240	4	12	569	45	4	86	12	35	147	18
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	9	267	4	13	632	50	4	96	13	39	163	20
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	15.2			82.4			12.5			15		
HCM LOS	С			F			В			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left. %	4%	3%	2%	17%
Vol Thru, %	84%	95%	91%	73%
Vol Right, %	12%	2%	7%	9%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	102	252	626	200
LT Vol	4	8	12	35
Through Vol	86	240	569	147
RT Vol	12	4	45	18
Lane Flow Rate	113	280	696	222
Geometry Grp	1	1	1	1
Degree of Util (X)	0.223	0.481	1.081	0.417
Departure Headway (Hd)	7.412	6.42	5.595	7.067
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	487	566	647	513
Service Time	5.412	4.42	3.663	5.067
HCM Lane V/C Ratio	0.232	0.495	1.076	0.433
HCM Control Delay	12.5	15.2	82.4	15
HCM Lane LOS	В	С	F	В
HCM 95th-tile Q	0.8	2.6	19.6	2

Intersection												
Intersection Delay, s/veh	142.1											
Intersection LOS	F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			41.			4		-	1	
Traffic Vol. veh/h	8	279	4	31	635	131	4	86	23	86	147	18
Future Vol. veh/h	8	279	4	31	635	131	4	86	23	86	147	18
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	9	310	4	34	706	146	4	96	26	96	163	20
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	20.4			242.9			14.7			20.4		
HCM LOS	С			F			В			С		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		4%	3%	4%	34%							
Vol Thru, %		76%	96%	80%	59%							
Vol Right, %		20%	1%	16%	7%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		113	291	797	251							
		4	8	31	86							
Through Vol		86	279	635	147							
		23	4	131	18							
Lane Flow Rate		126	323	886	279							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.26	0.59	1.481	0.545							
Departure Headway (Hd)		8.674	1.307	0.021	8.087							
Convergence, Y/N		10S	105	res	140							
Cap Sonvice Time		41/	490 5 267	1 101	449 6 007							
Service Time		0.074	0.307 0.652	4.101 1.470	0.007							
HCM Control Dolou		0.302	0.003 20 1	1.47Z	0.021 20 /							
HCM Lang LOS		14./ D	20.4	242.9 E	20.4							
HCM 95th_tile 0		D 1	20	۳ ۱ ۲ ۸	20							
			0.0	40.1	J.Z							

Intersection												
Intersection Delay, s/veh	103.4											
Intersection LOS	F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			é.	1		4.		7	Î.	
Traffic Vol. veh/h	8	279	4	31	635	131	4	86	23	86	147	18
Future Vol. veh/h	8	279	4	31	635	131	4	86	23	86	147	18
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles. %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	9	310	4	34	706	146	4	96	26	96	163	20
Number of Lanes	0	1	0	0	1	1	0	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			2			1		
HCM Control Delay	23.8			172.6			15.5			15.6		
HCM LOS	С			F			С			С		
Lane		NBLn1	EBLn1	WBLn1	WBLn2	SBLn1	SBLn2					
Vol Left, %		4%	3%	5%	0%	100%	0%					
Vol Thru, %		76%	96%	95%	0%	0%	89%					
Vol Right, %		20%	1%	0%	100%	0%	11%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		113	291	666	131	86	165					
LT Vol		4	8	31	0	86	0					
Through Vol		86	279	635	0	0	147					
RT Vol		23	4	0	131	0	18					
Lane Flow Rate		126	323	740	146	96	183					
Geometry Grp		6	6	7	7	7	7					
Degree of Util (X)		0.283	0.645	1.385	0.243	0.218	0.39					
Departure Headway (Hd)		8.975	7.746	6.739	6	8.989	8.391					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Сар		403	469	540	595	402	433					
Service Time		6.975	5.746	4.513	3.774	6.689	6.091					
HCM Lane V/C Ratio		0.313	0.689	1.37	0.245	0.239	0.423					
HCM Control Delay		15.5	23.8	204.5	10.7	14.2	16.4					
HCM Lane LOS		С	С	F	В	В	С					
HCM 95th-tile Q		1.1	4.5	33.6	0.9	0.8	1.8					

В

Intersection

Intersection Delay, s/veh Intersection LOS

10.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	18	306	13	4	130	21	5	92	10	27	50	11
Future Vol, veh/h	18	306	13	4	130	21	5	92	10	27	50	11
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	20	340	14	4	144	23	6	102	11	30	56	12
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	12.2			9.5			9.6			9.4		
HCM LOS	В			А			А			А		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	5%	5%	3%	31%
Vol Thru, %	86%	91%	84%	57%
Vol Right, %	9%	4%	14%	12%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	107	337	155	88
LT Vol	5	18	4	27
Through Vol	92	306	130	50
RT Vol	10	13	21	11
Lane Flow Rate	119	374	172	98
Geometry Grp	1	1	1	1
Degree of Util (X)	0.175	0.489	0.233	0.146
Departure Headway (Hd)	5.312	4.702	4.878	5.379
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	668	760	728	659
Service Time	3.407	2.766	2.957	3.477
HCM Lane V/C Ratio	0.178	0.492	0.236	0.149
HCM Control Delay	9.6	12.2	9.5	9.4
HCM Lane LOS	А	В	А	А
HCM 95th-tile Q	0.6	2.7	0.9	0.5

С

Intersection

Intersection Delay, s/veh Intersection LOS

```
s/veh 19.4
```

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	20	436	14	4	339	23	6	102	11	30	55	12
Future Vol, veh/h	20	436	14	4	339	23	6	102	11	30	55	12
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	484	16	4	377	26	7	113	12	33	61	13
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	24.7			17.3			11.8			11.5		
HCM LOS	С			С			В			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	5%	4%	1%	31%
Vol Thru, %	86%	93%	93%	57%
Vol Right, %	9%	3%	6%	12%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	119	470	366	97
LT Vol	6	20	4	30
Through Vol	102	436	339	55
RT Vol	11	14	23	12
Lane Flow Rate	132	522	407	108
Geometry Grp	1	1	1	1
Degree of Util (X)	0.245	0.777	0.621	0.203
Departure Headway (Hd)	6.682	5.357	5.493	6.79
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	541	668	651	531
Service Time	4.682	3.437	3.578	4.796
HCM Lane V/C Ratio	0.244	0.781	0.625	0.203
HCM Control Delay	11.8	24.7	17.3	11.5
HCM Lane LOS	В	С	С	В
HCM 95th-tile Q	1	7.4	4.3	0.8

Intersection												
Intersection Delay, s/veh	53.1											
Intersection LOS	F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$			4.			4	
Traffic Vol. veh/h	20	496	14	12	382	77	6	102	28	108	55	12
Future Vol. veh/h	20	496	14	12	382	77	6	102	28	108	55	12
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	551	16	13	424	86	7	113	31	120	61	13
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	79.1			48.3			15.1			16.9		
HCM LOS	F			E			С			С		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		4%	4%	3%	62%							
Vol Thru, %		75%	94%	81%	31%							
Vol Right, %		21%	3%	16%	7%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		136	530	471	175							
LT Vol		6	20	12	108							
Through Vol		102	496	382	55							
RT Vol		28	14	77	12							
Lane Flow Rate		151	589	523	194							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.329	1.056	0.928	0.422							
Departure Headway (Hd)		8.141	6.455	6.582	8.1							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		444	565	555	447							
Service Time		6.141	4.503	4.582	6.1							
HCM Lane V/C Ratio		0.34	1.042	0.942	0.434							
HCM Control Delay		15.1	79.1	48.3	16.9							
HCM Lane LOS		С	F	E	С							
HCM 95th-tile Q		1.4	16.9	11.6	2.1							

Intersection												
Intersection Delay, s/veh	60.7											
Intersection LOS	F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷.	1		4		7	Î.≯	
Traffic Vol, veh/h	20	496	14	12	382	77	6	102	28	108	55	12
Future Vol, veh/h	20	496	14	12	382	77	6	102	28	108	55	12
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	551	16	13	424	86	7	113	31	120	61	13
Number of Lanes	0	1	0	0	1	1	0	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			1			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			2			1		
HCM Control Delay	112.5			32.4			16.3			14.4		
HCM LOS	F			D			С			В		
Lane		NBLn1	EBLn1	WBLn1	WBLn2	SBLn1	SBLn2					
Vol Left, %		4%	4%	3%	0%	100%	0%					
Vol Thru, %		75%	94%	97%	0%	0%	82%					
Vol Right, %		21%	3%	0%	100%	0%	18%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
I raffic Vol by Lane		136	530	394	77	108	67					
		6	20	12	0	108	0					
Through Vol		102	496	382	0	0	55					
		28	14	0	11	0	12					
Lane Flow Rate		151	589	438	86	120	/4					
Geometry Grp		0 245	0	0 0 0 7 5	1	1	1					
Degree of Util (X)		0.345	1.149	0.835	0.146	0.286	0.165					
Departure Headway (Hd)		8.785 Vee	7.023	7.215 Vaa	6.48 Vaa	9.089	8.439					
Convergence, Y/N		14S	res	res	res	105	100					
Cap Sonico Timo		413 6785	5 0224	1015	007 1 10	090 6 780	420 6 130					
UCM Lana V/C Datio		0.100	0.023 1 10/	4.910	4.10 0.157	0.709	0.139					
HCM Control Dolou		162	1.124	0.07 I	0.104 10.2	0.30Z	U.1/3 10 0					
		10.3	112.3 E	טט./ ר	10.3	10.4	12.0 D					
HCM 05th tile 0		1 5	20 ∕	ت ر و		10	0 A (
		1.0	20.4	0.4	0.0	1.4	0.0					

APPENDIX G

CAPACITY ANALYSIS CALCULATIONS MITCHELL MILL ROAD & SITE ACCESS 1

Intersection							
Int Delay, s/veh	0.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		1	1.			1	
Traffic Vol, veh/h	0	388	765	4	0	32	
Future Vol, veh/h	0	388	765	4	0	32	
Conflicting Peds. #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	-	0	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	431	850	4	0	36	
Major/Minor M	laior1	Ν	Jaior?	N	linor?		
		0	viajuiz	N	nii iui z	850	
Stage 1	-	U	-	U	-	052	
Stage 2	-	-	-	-	-	-	
Critical Educy	-	-	-	-	-	6.22	
Critical Hdwy Sta 1	-	-	-	-	-	0.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	
	-	-	-	-	-	2 2 1 2	
Pollow-up Huwy Pot Con 1 Manauvor	-	-	-	-	-	3.510	
Stage 1	0	-	-	-	0	555	
Stage 2	0	-	-	-	0	-	
Platoon blocked %	U	-	-	-	0	-	
Mov Can-1 Manauver	_	-	-	-	_	350	
Mov Cap-1 Maneuver	-	-	-	-	-	-	
Stane 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	
Slaye 2	-	-	-	-	-	-	
A					05		
Approach	ER V		WB		SB		
HCM Control Delay, s	0		0		16.1		
HCM LOS					С		
Minor Lane/Major Mvmt		EBT	WBT	WBR S	BLn1		
Capacity (veh/h)		-	-	-	359		
HCM Lane V/C Ratio		-	-	-	0.099		
HCM Control Delay (s)		-	-	-	16.1		
HCM Lane LOS		-	-	-	С		
HCM 95th %tile Q(veh)		-	-	-	0.3		

Int Delay, s/veh 0.2 Movement EBL EBT WBT WBR SBL SBR Lane Configuration 	Intersection								
Movement EBL EBT WBT WBT SBL SBR Lane Configurations Image: Configurations	Int Delay, s/veh	0.2							
Lane Configurations Traffic Vol, veh/h 0 632 451 4 0 20 Conflicting Peds, #/h 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Traffic Vol, veh/h 0 632 451 4 0 20 Future Vol, veh/h 0 632 451 4 0 20 Conflicting Pecks, #/hr 0 0 0 0 0 Sign Control Free Free Free Free Stop RT Channelized None None None Storage Length - - 0 Yeh In Median Storage, # 0 0 0 0 - - - 0 Yeh In Median Storage, # 0 0 0 0 - - - 0 - - - 0 - - - - - 0 -	Lane Configurations		1	î.			1		
Future Vol, veh/h 0 632 451 4 0 20 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Free Storage Length - - - 0 - Grade, % - 0 0 - 0 Peak Hour Factor 90 90 90 90 90 Peak Hour Factor 90 90 90 90 90 Peak Hour Factor 90 90 90 90 90 Conflicting Flow All 0 702 501 4 0 22 Mymt Flow 0 702 501 4 0 22 Minort Major/Minor Major Minor2	Traffic Vol. veh/h	0	632	451	4	0	20		
Conflicting Peds, #/hr 0 0 0 0 0 Sign Control Free Free Free Stop Stop RT Channelized - None - 0 Veh in Median Storage, # 0 0 - 0 Grade, % 0 0 - 0 Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Mimor Major2 Minor2 Minor2 - - Conflicting Flow All 0 - 0 - - Stage 1 - - - - - Critical Hdwy - - - - - Stage 2 - - - - - - Follow-up Hdwy - - - 0 - Stage 1 - - - Stage 1 0 - - 0 - Stage 2 - - - Stage 2	Future Vol, veh/h	0	632	451	4	0	20		
Sign Control Free Free Free Free Stop Stop RT Channelized None None None None Storage Length - - - 0 Veh in Median Storage, # 0 0 - 0 Peak Hour Factor 90 90 90 90 Peak Hour Factor 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Minor Major/I Major2 Minor2 Minor2 Conflicting Flow All 0 0 - 0 - 503 Stage 1 - - - - - - Critical Hdwy - - - - - - Critical Hdwy Stg 1 - - - - - - - Follow-up Hdwy - - - - - - - - Stage 1 0 - - 0 - - - -	Conflicting Peds, #/hr	0	0	0	0	0	0		
RT Channelized - None - None Storage Length - - - 0 Veh in Median Storage, # 0 0 - 0 Grade, % - 0 0 - 0 Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Mmit Flow 0 702 501 4 0 22 Major/Minor Major/I Major/Z Minor2 Conflicting Flow All 0 - 0 - 503 Stage 1 - <t< td=""><td>Sign Control</td><td>Free</td><td>Free</td><td>Free</td><td>Free</td><td>Stop</td><td>Stop</td><td></td></t<>	Sign Control	Free	Free	Free	Free	Stop	Stop		
Storage Length - - - 0 Veh in Median Storage, # - 0 0 - 0 Pack Hour Factor 90 90 90 90 90 90 Peak Hour Factor 90 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 2 Minor Major/Minor Major/I Major/Z Minor/2 Conflicting Flow All 0 - 0 - 503 Stage 1 - - - - - - Critical Hdwy Stg 1 - - - - - - Critical Hdwy Stg 2 - - - - - - - Follow-up Hdwy - - - 0 -	RT Channelized	-	None	-	None	-	None		
Veh in Median Storage, # 0 0 - 0 - Grade, % - 0 0 - 0 - Grade, % - 0 0 90 90 90 90 Peak Hour Factor 90 90 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 2 Minor Major/ Major Minor2 Minor2 - - Conflicting Flow All 0 0 - 503 - - - Stage 1 -<	Storage Length	-	-	-	-	-	0		
Grade, % - 0 0 - 0 - Peak Hour Factor 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 Mymt Flow 0 702 501 4 0 22 Minor/Minor Major1 Major2 Minor2 Conflicting Flow All - 0 - 503 Stage 1 - - - - Stage 2 - - - - Critical Hdwy 1 - - - Critical Hdwy Stg 1 - - - - Follow-up Hdwy - - - - Follow-up Hdwy - - - - Stage 1 0 - - 0 - Stage 2 0 - - 0 - Stage 1 - - - - - Stage 2 - - - - - Stag	Veh in Median Storage	e.# -	0	0	-	0	-		
Peak Hour Factor 90 90 90 90 90 90 90 90 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mymt Flow 0 702 501 4 0 22 Major/Minor Major1 Major2 Minor2 Conflicting Flow All - 0 - 503 Stage 1 - - - - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - - - Carl Maneuver 0 - 0 - Stage 1 O - 0 - - - Stage 1 0 - - 0 - Stage 2 0 - - - 569 Mov Cap-1 Maneuver - - - - Vo Cap-2 Maneuver - - - - Stage 1 - - - - Mov Cap-1 Maneuver	Grade. %	-	0	0	-	0	-		
Heavy Vehicles, % 2 3	Peak Hour Factor	90	90	90	90	90	90		
Mvmt Flow 0 702 501 4 0 22 Major/Minor Major1 Major2 Minor2 Conflicting Flow All 0 - 0 - 503 Stage 1 - - - - - - Critical Hdwy - - - - - - Critical Hdwy Stg 1 - - - - - - Critical Hdwy Stg 2 - - - - - - Collow-up Hdwy - - - - 3.318 Pot Cap-1 Maneuver 0 - - 0 - Stage 1 0 - - 0 - Stage 2 0 - - - - Mov Cap-2 Maneuver - - - - - Stage 2 - - - - - - Mor Cap-2 Maneuver	Heavy Vehicles, %	2	2	2	2	2	2		
Major/Minor Major1 Major2 Minor2 Conflicting Flow All 0 0 503 Stage 1 - - - Stage 2 - - - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - - Critical Hdwy Stg 2 - - - Critical Hdwy Stg 2 - - - Conflicting Flow All - 0 - Follow-up Hdwy - - - Critical Hdwy Stg 2 - - - Follow-up Hdwy - - - Stage 1 0 - - Stage 2 0 - - Mov Cap-1 Maneuver - - - Stage 1 - - - Stage 2 - - - Stage 1 - - - Stage 1 - - -	Mvmt Flow	0	702	501	4	0	22		
Major/Minor Major1 Major2 Minor2 Conflicting Flow All 0 0 503 Stage 1 - - - Stage 2 - - - Critical Hdwy - - - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - - Critical Hdwy Stg 2 - - - Critical Hdwy Stg 2 - - - Follow-up Hdwy - - - - Follow-up Hdwy - - 0 - Stage 1 0 - - 0 - Stage 2 0 - - 0 - Platoon blocked, % - - - - - Stage 1 - - - - - Stage 2 - - - - - Stage 1 - - - </td <td> '</td> <td>5</td> <td></td> <td></td> <td>-</td> <td>-</td> <td></td> <td></td>	'	5			-	-			
Maganitation Maganity Maganity Maganity Conflicting Flow All 0 0 503 Stage 1 - - - Stage 2 - - - Critical Hdwy Stg 1 - - - Critical Hdwy Stg 2 - - - Childe Hdwy - - - - Critical Hdwy Stg 2 - - - - Conflicting How Xit Cap -1 Maneuver - - 0 Stage 1 0 - - 0 - Stage 2 0 - - - - Mov Cap-1 Maneuver - - - - - Stage 1 - - - - - - Mov Cap-2 Maneuver	Major/Minor	Maior1	,	Maior?	N	linor?			
Stage 1 - </td <td>Conflicting Flow All</td> <td></td> <td><u>ا</u></td> <td>najuiz</td> <td>N</td> <td></td> <td>503</td> <td></td>	Conflicting Flow All		<u>ا</u>	najuiz	N		503		
Stage 1 - </td <td>Stage 1</td> <td>-</td> <td>0</td> <td>-</td> <td>0</td> <td>-</td> <td>505</td> <td></td>	Stage 1	-	0	-	0	-	505		
Stage 2 - </td <td>Stage 2</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td></td>	Stage 2	-	-	-	-	-	-		
Cirtical Howy Stg 1 - - - 0.22 Critical Howy Stg 2 - - - - Follow-up Hdwy - - - 3.318 Pot Cap-1 Maneuver 0 - - 0 Stage 1 0 - - 0 - Stage 2 0 - - 0 - Platoon blocked, % - - - 569 Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 1 - - - - Stage 2 - - - - Stage 2 - - - - Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM LOS B - - 0.039 HCM Lone V/C Ratio - - 0.039 HCM Stitia O(web) <td>Critical Udway</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>6 22</td> <td></td>	Critical Udway	-	-	-	-	-	6 22		
Critical Howy Stg 2 -	Critical Hduny Sta 1	-	-	-	-	-	0.22		
Childra Huky Sig 2 - - - - - - - 3.318 Pot Cap-1 Maneuver 0 - - 0 569 - - 0 - - - 0 - - - 0 - - - 0 - - - 0 - - - 0 -	Critical Howy Sty 1	-	-	-	-	-	-		
Pol Cap-1 Maneuver 0 0 569 Stage 1 0 0 - Stage 2 0 0 - Platoon blocked, % Mov Cap-1 Maneuver 569 Mov Cap-2 Maneuver 569 Mov Cap-2 Maneuver Stage 1 Stage 2 Stage 2 Stage 2 Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) 569 HCM Los B Minor Lane V/C Ratio 0.039 HCM Control Delay (s) 11.6 HCM LoS B HCM S		-	-	-	-	-	2 2 1 0		
For Cap - Maneuver 0 - - 0 509 Stage 1 0 - - 0 - Platoon blocked, % - - - 0 - Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 2 - - - - Mor Control Delay, s 0 0 11.6 HCM Control Delay, s 0 0 11.6 Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - - 0.1	Pot Con 1 Manauwar	-	-	-	-	-	5.510		
Stage 1 0 - - 0 - Stage 2 0 - - 0 - Platoon blocked, % - - - 0 - Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 2 - - - - Stage 2 - - - - Stage 2 - - - - More Control Delay, s 0 0 11.6 HCM Control Delay, s 0 0 11.6 Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Sth %/lip O(uph) - - - 0 1 - -	Pol Cap-1 Maneuver	0	-	-	-	0	209		
Stage 2 0 - - 0 - Platoon blocked, % - - - 569 Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - - Stage 2 - - - - - Approach EB WB SB - - HCM Control Delay, s 0 0 11.6 - Minor Lane/Major Mvmt EBT WBT WBR SBLn1 - - - 569 MCM Lane V/C Ratio - - - 569 - - 10.039 HCM Control Delay (s) - - - 569 - - 10.039 HCM Control Delay (s) - - - - 11.6 HCM 25H - - - - 11.6 HCM 25H - - - B - - -	Stage 1	0	-	-	-	0	-		
Match blocked, % - - - Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - - Stage 2 - - - - - - Approach EB WB SB - - - - HCM Control Delay, s 0 0 11.6 - - - - Minor Lane/Major Mvmt EBT WBT WBR SBLn1 - - 569 MCM Lane V/C Ratio - - 569 - - 0.039 HCM Control Delay (s) - - 569 - - 11.6 HCM Control Delay (s) - - - 0.1 - - 0.1	Stage Z	0	-	-	-	0	-		
Mov Cap-1 Maneuver - - - 569 Mov Cap-2 Maneuver - - - - Stage 1 - - - - Stage 2 - - - - Approach EB WB SB HCM Control Delay, s 0 0 11.6 HCM LOS B B Minor Lane/Major Mvmt EBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Delay (s) - - 0.1	Platoon blocked, %		-	-	-		500		
Mov Cap-2 Maneuver -	Mov Cap-1 Maneuver	-	-	-	-	-	569		
Stage 1 - <th -<="" <="" td=""><td>Mov Cap-2 Maneuver</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td></td></th>	<td>Mov Cap-2 Maneuver</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td></td>	Mov Cap-2 Maneuver	-	-	-	-	-	-	
Stage 2 - <th -<="" <="" td=""><td>Stage 1</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td></td></th>	<td>Stage 1</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td></td>	Stage 1	-	-	-	-	-	-	
Approach EB WB SB HCM Control Delay, s 0 0 11.6 HCM LOS B B Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - 11.6 HCM Sth % tile O(web) - - 0.1	Stage 2	-	-	-	-	-	-		
Approach EB WB SB HCM Control Delay, s 0 0 11.6 HCM LOS B Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - 11.6 HCM Sth % tile O(veh) - - 0.1									
HCM Control Delay, s 0 0 11.6 HCM LOS B Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - 11.6 HCM Sth % tile O(veh) 0.1 0.1	Approach	EB		WB		SB			
HCM LOS B Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - B	HCM Control Delay, s	0		0		11.6			
Minor Lane/Major Mvmt EBT WBT WBR SBLn1 Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - B	HCM LOS					В			
Minor Lane/Major MvmtEBTWBTWBR SBLn1Capacity (veh/h)569HCM Lane V/C Ratio0.039HCM Control Delay (s)11.6HCM Lane LOSBHCM 95th % tile O(veb)0.1									
Capacity (veh/h) - - 569 HCM Lane V/C Ratio - - 0.039 HCM Control Delay (s) - - 11.6 HCM Lane LOS - - B HCM 95th % tile O(veh) 0.1	Minor Lane/Major Mvr	nt	EBT	WBT	WBR S	SBLn1			
HCM Lane V/C Ratio 0.039 HCM Control Delay (s) 11.6 HCM Lane LOS B HCM 95th % tile O(veb)	Capacity (veh/h)		-	_	-	569			
HCM Control Delay (s) 11.6 HCM Lane LOS B HCM 95th % tile O(veb)	HCM Lane V/C Ratio		-	-	-	0.039			
HCM 95th % tile O(veb)	HCM Control Delay (s)	-	-	-	11 6			
	HCM Lane LOS	/	-	-	-	 B			
	HCM 95th %tile Q(ver	1)	-	-	-	0.1			

APPENDIX H

CAPACITY ANALYSIS CALCULATIONS MITCHELL MILL ROAD & SITE ACCESS 2

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	×.	٨		1	M	
Traffic Vol. veh/h	101	287	626	11	19	139
Future Vol. veh/h	101	287	626	11	19	130
Conflicting Peds #/hr	0	207	020	0	0	100
Sign Control	Froo	Free	Free	Free	Ston	Stop
DT Channelized	1166	Nono	1166	Nono	Stop	Nono
Storage Longth	150	NULLE	-	100	-	NULLE
Voh in Modion Storage	. #	-	-	100	0	-
	;,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	112	319	696	12	21	154
Major/Minor	Major1	Ν	Major2	I	Minor2	
Conflicting Flow All	708	0	-	0	1239	696
Stage 1	-	-	-	-	696	-
Stage 2	-	-	-	-	543	-
Critical Hdwv	4.12	-	-	-	6.42	6.22
Critical Hdwy Sto 1	-	-	-	-	5.42	
Critical Hdwy Sto 2	-	-	-	-	5 42	-
Follow-up Hdwy	2 2 1 8	_	_	_	3 518	3 3 1 8
Pot Can-1 Maneuver	801	_			10/	412
Stage 1	031	-	-	-	104	442
Stage 2	-	-	-	-	490	-
Staye 2	-	-	-	-	90Z	-
Mail One 4 Mail	004	-	-	-	470	440
Mov Cap-1 Maneuver	891	-	-	-	1/0	442
Mov Cap-2 Maneuver	-	-	-	-	170	-
Stage 1	-	-	-	-	433	-
Stage 2	-	-	-	-	582	-
Approach	EB		WB		SB	
HCM Control Delay s	2.5		0		23.1	
HCM LOS	2.0		J		<u></u> .,	
					0	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		891	-	-	-	371
HCM Lane V/C Ratio		0.126	-	-	-	0.473
HCM Control Delay (s)		9.6	-	-	-	23.1
HCM Lane LOS		А	-	-	-	С
HCM 95th %tile Q(veh))	0.4	-	-	-	2.4
	,					

Intersection							
Int Delay, s/veh	3.9						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	+	•	1	Y		
Traffic Vol. veh/h	176	456	353	26	33	98	
Future Vol. veh/h	176	456	353	26	33	98	
Conflicting Peds. #/hr	0	0	0	0	0	0	
Sian Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	_	None	-	None	
Storage Length	150	-	-	100	0	-	
Veh in Median Storage	e.# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles. %	2	2	2	2	2	2	
Mymt Flow	196	507	392	29	37	109	
					•		
Major/Miner	Maira		Mais=0		Alman O		
			viajorz			200	
	421	0	-	0	1291	392	
Stage 1	-	-	-	-	392	-	
Stage 2	-	-	-	-	899	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1138	-	-	-	180	657	
Stage 1	-	-	-	-	683	-	
Stage 2	-	-	-	-	397	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1138	-	-	-	149	657	
Mov Cap-2 Maneuver	-	-	-	-	149	-	
Stage 1	-	-	-	-	566	-	
Stage 2	-	-	-	-	397	-	
Approach	EB		WB		SB		
HCM Control Delay. s	2.5		0		22.2		
HCM LOS					С		
					-		
Minor Long/Major Mum	nt	EDI	СDT				
	ш		CDÍ	VVDI	VOR		
Capacity (ven/h)		1138	-	-	-	353	
HUM Lane V/C Ratio	、	0.1/2	-	-	-	0.412	
HCM Control Delay (s)	8.8	-	-	-	22.2	
HCM Lane LOS	,	A	-	-	-	C	
HCM 95th %tile Q(veh	1)	0.6	-	-	-	2	

APPENDIX I

TURN LANE WARRANTS





Mitchell Mill Road and Site Access 2

2027 Build										
Peak Hour	Approach	Right Turn Volume	Approach Volume	Warranted?						
AM	Westbound	11	637	No						
PM	Westbound	26	379	No						



APPENDIX J

MUTCD / ITRE SIGNAL WARRANT ANALYSIS

Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project Name												
Project/File #	20498	20498 - 05										
Scenario	2027 N	2027 No-Build										
Intersection Information												
Major Street (E/W Road)	US 401 Bypass	Minor Street (N/S Road)	Jonesville Road / WB Left-Over									
Analyzed with	2 or more approach lanes	Analyzed with	1 Approach Lane									
Total Approach Volume	2861 vehicles	Total Approach Volume	424 vehicles									
Total Ped/Bike Volume	0 crossings	0 crossings										
Right turn reduction of	100 percent applied	Right turn reduction of	0 percent applied									

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

	Warrant 1, Eight Ho	ur Vehicular Volume	
	Condition A	Condition B	Condition A+B*
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied
Required values reached for	2 hours	2 hours	2 (Cond. A) & 2 (Cond. B)
Criteria - Major Street (veh/hr)	420	630	336 (Cond. A) & 504 (Cond. B)
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Ho	ur Vehicular Volume
Condition Satisfied?	Not Satisfied
Required values reached for	2 hours
Criteria	See Figure Below

Warrant 3, Peak Ho	ur Vehicular Volume	
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	1941 total, 41 minor, 0 delay	2 hours
Criteria - Total Approach Volume (veh in one hour)	800	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



Major Street Volume (Both Approaches)



AM Pe	ak Hour			
hdv	g/c	в	q	С
006	0.7	0.00004	0.0097	0.4284
961	0.7	4.0E-05	0.009192	0.460018
1080	0.7	0.00004	0.0082	0.5217

PM Pe	ak Hour			
hdv	g/c	е	q	c
1800	0.7	0.00004	0.0097	0.4284
1900	0.7	4.0E-05	0.008867	0.480233
1980	0.7	0.00004	0.0082	0.5217

ft	hdm	S	
8800	55	109.09	
Distance to Upstream Signal	Posted Speed Limit	Travel Time	



CVAF	1	
Conflicting Volume (vph)	1900	
Adjusted Conflicting (vph)	1900	
Turning Volume (vph)	41	







AM Pe	ak Hour			
hdv	g/c	в	q	c
720	0.7	0.00004	0.0108	0.2587
873	0.7	3.2E-05	0.009525	0.34557
006	0.7	0.00003	0.0093	0.3609

PM Pe	ak Hour			
hdv	g/c	e	q	J
1800	0.7	0.00004	0.0108	0.2587
1835	0.7	3.8E-05	0.010508	0.278572
1980	0.7	0.00003	0.0093	0.3609

ft	mph	s
8800	55	109.09
Distance to Upstream Signal	Posted Speed Limit	Travel Time

1	873	873	150	
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)	

1	1835	1835	138	
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)	





Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project Name	Hills at Harris Creek
Project/File #	20498 - 05
Scenario	2027 Build

	Intersection	Information	
Major Street (E/W Road)	US 401 Bypass	Minor Street (N/S Road)	Jonesville Road / WB Left-Over
Analyzed with	2 or more approach lanes	Analyzed with	1 Approach Lane
Total Approach Volume	2947 vehicles	Total Approach Volume	607 vehicles
Total Ped/Bike Volume	0 crossings	Total Ped/Bike Volume	0 crossings
Right turn reduction of	100 percent applied	Right turn reduction of	0 percent applied

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

	Warrant 1, Eight Ho	ur Vehicular Volume	
	Condition A	Condition B	Condition A+B*
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied
Required values reached for	2 hours	2 hours	2 (Cond. A) & 2 (Cond. B)
Criteria - Major Street (veh/hr)	420	630	336 (Cond. A) & 504 (Cond. B)
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Ho	ur Vehicular Volume
Condition Satisfied?	Not Satisfied
Required values reached for	2 hours
Criteria	See Figure Below

Warrant 3, Peak Ho	ur Vehicular Volume	
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	2019 total, 67 minor, 0 delay	2 hours
Criteria - Total Approach Volume (veh in one hour)	800	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



Major Street Volume (Both Approaches)

				▲ 204/82	▲ 1853/879	F 112/67		- -	16 27	e
n] [Build]		9	71,	/81	77	BYPASS	401	TOF	40/128	
et Left-Tu		1	995	995	112			1	1952	
esville Road [Major-Stre		CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)			CVAF	Conflicting Volume (vph)	
ass & Jone						1		_		r
01 Bypa		J	0.4284	0.477642	0.5217			J	0.4284	
US 4(q	0.0097	0.008908	0.0082			q	0.0097	
		е	0.00004	4.0E-05	0.00004			a	0.00004	
	ık Hour	g/c	0.7	0.7	0.7		hk Hour	g/c	0.7	
	AM Pea	hdv	006	995	1080		PM Pea	hdv	1800	

hgm	55	imit	ted Speed L
ft	0088	m Signal	to Upstrea
0.5217	0.0082	0.00004	0.7
0.507187	0.008433	4.0E-05	0.7

_			1
¥	hdm	s	
8800	55	109.09	
Distance to Upstream Signal	Posted Speed Limit	Travel Time	

1	1952	1952	67	
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)	





US 401 Bypass & Jonesville Road [Minor-Street Right-Turn] [Build]
US 401 Bypass & Jone
US 401 Bypass & .
US 401

AM Pe	ak Hour			
hdv	g/c	е	q	c
720	0.7	0.00004	0.0108	0.2587
873	0.7	3.2E-05	0.009525	0.34557
006	0.7	0.00003	0.0093	0.3609

ft	mph	S	
8800	55	109.09	
Distance to Upstream Signal	Posted Speed Limit	Travel Time	

CVAE	-
	+
Conflicting Volume (vph)	873
Adjusted Conflicting (vph)	873
Turning Volume (vph)	236

1	1835	1835	192
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)





Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Hills at H	arris Creek	
2049	98 - 05	
2027	lo-Build	
Intersectio	on Information	
US 401 Bypass	Minor Street (N/S Road)	Eastern U-Turn Location
2 or more approach lanes	Analyzed with	1 Approach Lane
2886 vehicles	Total Approach Volume	173 vehicles
0 crossings	Total Ped/Bike Volume	0 crossings
0 percent applied	Right turn reduction of	0 percent applied
	Hills at H 2049 2027 M 2027 M Intersection US 401 Bypass 2 or more approach lanes 2886 vehicles 0 crossings 0 percent applied	Hills at Harris Creek 20498 - 05 2027 No-Build Intersection Information US 401 Bypass Minor Street (N/S Road) 2 or more approach lanes Analyzed with 2886 vehicles Total Approach Volume 0 crossings Total Ped/Bike Volume 0 percent applied Right turn reduction of

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

	Warrant 1, Eight Hour Vehicular Volume				
	Condition A	Condition B	Condition A+B*		
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied		
Required values reached for	0 hours	2 hours	1 (Cond. A) & 2 (Cond. B)		
Criteria - Major Street (veh/hr)	420	630	336 (Cond. A) & 504 (Cond. B)		
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)		

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Ho	ur Vehicular Volume
Condition Satisfied?	Not Satisfied
Required values reached for	2 hours
Criteria	See Figure Below

Warrant 3, Peak Ho	ur Vehicular Volume	
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	2094 total, 100 minor, 0 delay	1 hour
Criteria - Total Approach Volume (veh in one hour)	650	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



US 401 Bypass & Eastern U-Turn Location [Major-Street U-Turn] [No-Build]

AM Pe	ak Hour			
hdv	g/c	е	q	c
1800	0.7	0.00003	0.0072	0.5106
1994	0.7	3.0E-05	0.006984	0.539484
1980	0.7	0.00003	0.007	0.5374

	_				
	J	0.5106	0.536209	0.5374	
	q	0.0072	0.007009	0.007	
	a	0.00003	3.0E-05	0.00003	
ik Hour	g/c	0.7	0.7	0.7	
PM Pea	hdv	720	892	006	

ft	mph	S
10000	55	123.97
Distance to Upstream Signal	Posted Speed Limit	Travel Time

h) 1994	100	
Adjusted Conflicting (vp	Turning Volume (vph)	

1	268	268	73
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)



1994 1994

Conflicting Volume (vph)

CVAF

-











Adjusted Floresate (vph)
Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project Name	Hills at Harris Creek
Project/File #	20498 - 05
Scenario	2027 Build

Intersection Information			
Major Street (E/W Road)	US 401 Bypass	Minor Street (N/S Road)	Eastern U-Turn Location
Analyzed with	2 or more approach lanes	Analyzed with	1 Approach Lane
Total Approach Volume	2929 vehicles	Total Approach Volume	266 vehicles
Total Ped/Bike Volume	0 crossings	Total Ped/Bike Volume	0 crossings
Right turn reduction of	0 percent applied	Right turn reduction of	0 percent applied

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

Warrant 1, Eight Hour Vehicular Volume			
	Condition A	Condition B	Condition A+B*
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied
Required values reached for	2 hours	2 hours	2 (Cond. A) & 2 (Cond. B)
Criteria - Major Street (veh/hr)	420	630	336 (Cond. A) & 504 (Cond. B)
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Hour Vehicular Volume		
Condition Satisfied?	Not Satisfied	
Required values reached for	2 hours	
Criteria	See Figure Below	

Warrant 3, Peak Hour Vehicular Volume		
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	2168 total, 157 minor, 0 delay	1 hour
Criteria - Total Approach Volume (veh in one hour)	650	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



Major Street Volume (Both Approaches)

US 401 Bypass & Eastern U-Turn Location [Major-Street U-Turn] [Build]

AM Pe	ak Hour			
hdv	g/c	е	q	J
1980	0.7	0.00003	0.007	0.5374
2011	0.7	3.0E-05	0.006966	0.544651
2160	0.7	0.00003	0.0068	0.5795

	_	_	_		
	С	0.5374	0.54161	0.5795	
	b	0.007	0.00698	0.0068	
	a	0.00003	3.0E-05	0.00003	
ik Hour	g/c	0.7	0.7	0.7	
PM Pea	чdл	006	918	1080	

ft	hdm	S
10000	55	123.97
Distance to Upstream Signal	Posted Speed Limit	Travel Time



1	918	918	109
CVAF	Conflicting Volume (vph)	Adjusted Conflicting (vph)	Turning Volume (vph)





Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project Name	Hills at]	
Project/File #	20498 - 05		
Scenario	2027 No-Build		
Intersection Information			
Major Street (E/W Road)	Mitchell Mill Road	Minor Street (N/S Road)	Jonesville Road
Analyzed with	1 approach lane	Analyzed with	1 Approach Lane
Total Approach Volume	1708 vehicles	Total Approach Volume	518 vehicles
Total Ped/Bike Volume	0 crossings	Total Ped/Bike Volume	0 crossings
Right turn reduction of	100 percent applied	Right turn reduction of	100 percent applied

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

Warrant 1, Eight Hour Vehicular Volume				
	Condition A	Condition B	Condition A+B*	
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied	
Required values reached for	2 hours	2 hours	2 (Cond. A) & 2 (Cond. B)	
Criteria - Major Street (veh/hr)	350	525	280 (Cond. A) & 420 (Cond. B)	
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)	

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Hour Vehicular Volume		
Condition Satisfied?	Not Satisfied	
Required values reached for	2 hours	
Criteria	See Figure Below	

Warrant 3, Peak Hour Vehicular Volume		
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	1178 total, 200 minor, 0 delay	1 hour
Criteria - Total Approach Volume (veh in one hour)	800	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



Traffic Signal Warrant Analysis

Warrants 1 - 3 (Volume Warrants)

Project/File #	22422 25
Project/ File #	20498 - 05
Scenario	2027 Build

Intersection Information				
Major Street (E/W Road)	Mitchell Mill Road	Minor Street (N/S Road)	Jonesville Road	
Analyzed with	1 approach lane	Analyzed with	1 Approach Lane	
Total Approach Volume	2087 vehicles	Total Approach Volume	675 vehicles	
Total Ped/Bike Volume	0 crossings	Total Ped/Bike Volume	0 crossings	
Right turn reduction of	100 percent applied	Right turn reduction of	100 percent applied	

No high speed or isolated community reduction applied to the Volume Warrant thresholds.

Warrant 1, Eight Hour Vehicular Volume			
	Condition A	Condition B	Condition A+B*
Condition Satisfied?	Not Satisfied	Not Satisfied	Not Satisfied
Required values reached for	2 hours	2 hours	2 (Cond. A) & 2 (Cond. B)
Criteria - Major Street (veh/hr)	350	525	280 (Cond. A) & 420 (Cond. B)
Criteria - Minor Street (veh/hr)	105	53	84 (Cond. A) & 42 (Cond. B)

* Should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

Warrant 2, Four Hour Vehicular Volume		
Condition Satisfied?	Not Satisfied	
Required values reached for	2 hours	
Criteria	See Figure Below	

Warrant 3, Peak Hour Vehicular Volume		
	Condition A	Condition B
Condition Satisfied?	Not Satisfied	Satisfied
Required values reached for	1450 total, 251 minor, 0 delay	2 hours
Criteria - Total Approach Volume (veh in one hour)	800	
Criteria - Minor Street High Side Volume (veh in one hour)	100	See Figure Below
Criteria - Minor Street High Side Delay (veh-hrs)	4	



Major Street Volume (Both Approaches)



The items below are required in order to complete your application and shall be submitted when the application if filed.

- 1. A complete copy of the last deed of record for proof of ownership
- 2. An annexation boundary plat/map for recordation at the Wake County Register of Deeds Office (mylar plat) prepared by a professional land surveyor showing the boundaries of the area or property for annexation into the Town of Rolesville.
- 3. A complete copy of the written metes and bounds description based on the annexation boundary plat/map.

SECTION 1 - LOCATION

Is the area contiguous with the existing primary corporate limits? Satellite corporate limits is not primary. \Box Yes or X No Note: If the land is contiguous to any existing corporate limits, the proposed annexation boundary will include all intervening right-of-ways for streets, easements, and other areas as stated in North Carolina General Statute §160-131(1).

SECTION 2 - VESTED RIGHTS

NC General Statues require petitioners of both contiguous and non-contiguous annexations to file a signed statement declaring whether vested rights have been established in accordance with G.S. 160A-385.1 or 153A-344.1 for properties subject to the petition. Do you declare vested rights for the property subject to this petition? \Box Yes or No No

SECTION 3 - PROPERTY DETAILS

PIN Number	Real Estate ID Number	Deed Book Number	Page Number	Acreage To Be Annexed	Wake County Assessed Value
1757761273	0443802	DB016701	PG 00363	10.48	\$ 196,750
and the second		DB	PG		\$
		DB	PG		S

SECTION 4 - SIGNATURES AND VERIFICATION

We, the undersigned owners of the real properties comained in the metes and bounds description and plat/map attached hereto, respectfully request that the area described above be annexed and made part of the Town of Rolesville, North Carolino. By signing below, we acknowledge that all information is correct.

If property owned by INDIVIDUALS (NOTE: All legal owners must sign including both husband and wife)

hutter		3/21/22
Signature of Owner #1	Randall & Laura Watkins 3544	Date Signed
Marto	Donlin Dr, Wake Forest NC 27587	21 Mar 2022
Signature of Owner #2		Date Signed

Signature of Owner #2

Name of Corporation

If property owned by a COMPANY OR CORPORATION (NOTE: The company or corporation must be legally registered with the State of North Carolina - Office of the Secretary of State)

Printed Name of Registered Agent Signature of Registered Agent Address, State, Zip of Registered Office: Cousty , a Natury Public for said County and State, do hereby certify that the above signed individual(s) appeared before me this day and signed the foregoing instrument. LEF_day of MURCH_209.7 day of MURCH, 2022 <u>Slaben</u> otary Public y commission expires: <u>10/7/</u>



The items below are required in order to complete your application and shall be submitted when the application if filed.

- 1. A complete copy of the last deed of record for proof of ownership
- 2. An annexation boundary plat/map for recordation at the Wake County Register of Deeds Office (mylar plat) prepared by a professional land surveyor showing the boundaries of the area or property for annexation into the Town of Rolesville.
- 3. A complete copy of the written metes and bounds description based on the annexation boundary plat/map.

SECTION 1 - LOCATION

Is the area contiguous with the existing primary corporate limits? Satellite corporate limits is not primary. \Box Yes or \boxtimes No Note: If the land is contiguous to any existing corporate limits, the proposed annexation boundary will include all intervening right-of-ways for streets, easements, and other areas as stated in North Carolina General Statute §160-131(1).

SECTION 2 - VESTED RIGHTS

NC General Statues require petitioners of both contiguous and non-contiguous annexations to file a signed statement declaring whether vested rights have been established in accordance with G.S. 160A-385.1 or 153A-344.1 for properties subject to the petition. Do you declare vested rights for the property subject to this petition? \Box Yes or \boxtimes No

SECTION 3 - PROPERTY DETAILS

PIN Number	Real Estate ID Number	Deed Book Number	Page Number	Acreage To Be Annexed	Wake County Assessed Value
1757778982	0443803	DB20-E	PG 114	97.41	\$ 1,954,590
1757750520 0074789	0074789	DB20-E	PG 114	0.69	\$ 179,871
		DB	PG		5

SECTION 4 - SIGNATURES AND VERIFICATION

We, the undersigned owners of the real properties contained in the metes and bounds description and plat/map attached hereto, respectfully request that the area described above be annexed and made part of the Town of Rolesville, North Carolina. By signing below, we acknowledge that all information is correct.

If property owned by INDIVIDUALS (NOTE: All legal owners must sign including both husband and wife)

Jank D With	**********	3/15/2022
Signature of Owner #1	3609 Rock Farm Rd, Wake	Date Signed
Donnantiapun	Forest NC 27587-6872	3-15-2022
Signature of Owner #2		Date Signed

 If property owned by a COMPANY OR CORPORATION (NOTE: The company or corporation must be legally registered with the State of North Carolina – Office of the Secretary of State)

Name of Corporation		
Printed Name of Registered Agent	Signature of Registered Agent	TEH DA
Address, State, Zip of Registered Office:		3 NOTARL C
h Carolinu, <u>Wake</u> County Kerriette H. Dogg es H, a Nytary Public for said Cou	av and State, ashe are the there are and state, appeared before un	e this day and statue the location instrument.
ness my hand and official seal, this <u>15</u> day of <u>Marco</u>	B NOTA Marriter	South Contraction
Noterre Sout	Notary Phylic My contraintsion expires: D	7 2 2 2 Hand COUNTINGAN
	COUN Current	



The items below are required in order to complete your application and shall be submitted when the application if filed.

- 1. A complete copy of the last deed of record for proof of ownership
- 2. An annexation boundary plat/map for recordation at the Wake County Register of Deeds Office (mylar plat) prepared by a professional land surveyor showing the boundaries of the area or property for annexation into the Town of Rolesville.
- 3. A complete copy of the written metes and bounds description based on the annexation boundary plat/map.

SECTION 1 - LOCATION

Is the area contiguous with the existing primary corporate limits? Satellite corporate limits is not primary. Dressor Row Note: If the land is contiguous to any existing corporate limits, the proposed annexation boundary will include all intervening right-of-ways for streets, easements, and other areas as stated in North Carolina General Statute §160-131(1).

SECTION 2 - VESTED RIGHTS

NC General Statuces require petitioners of both contiguous and non-contiguous annexations to file a signed statement declaring whether vested rights have been established in accordance with G.S. 160A-385.1 or 153A-344.1 for properties subject to the petition. Do you declare vested rights for the property subject to this petition? \Box Yes or \boxtimes No

SECTION 3 - PROPERTY DETAILS

PIN Number	Real Estate ID Number	Deed Book Number	Page Number	Acreage To Be Annexed	Wake County Assessed Value
1757778982	0443803	DB20-E	PG 114	97.41	\$ 1,954,590
1757750520 0074789	0074789	DB20-E	PG 114	0.69	\$ 179,871
		DB	PG		S

SECTION 4 - SIGNATURES AND VERIFICATION

1 Alt

111

We, the undersigned owners of the real properties contained in the metes and bounds description and plat/map attached hereto, respectfully request that the area described above be annexed and made part of the Toxyn of Rolesville, North Carolina. By signing below, we acknowledge that all information is correct.

If property owned by INDIVIDUALS (NOTE: All legal owners must sign including both husband and wife)

Man Mathine		
Signature of Owner #1		3609 Rock Farm Rd
Malady L. Lale	they mad	Wake Forest, NC
Signature of Owner #2		27587-6872

 $\frac{3/15/2022}{\text{Date Signed}}$ $\frac{3/15/2022}{\text{Date Signed}}$

 If property owned by a COMPANY OR CORPORATION (NOTE: The company or corporation must be legally registered with the State of North Carolina – Office of the Secretary of State)

Name of Corporation

Printed Name of Registered Agent

Signature of Registered Agent

Address, State, Zip of Registered Office: North Carolina. <u>Market</u> County <u>Address, State, Zip of Registered Office:</u> North Carolina. <u>Market</u> County <u>Address, Market County</u> <u>Address, State, Zip of Registered Office:</u> <u>Address, State, Zip of County</u> <u>Address, State, Zip of County, Z</u>	re signed individualis) appeared before me this day and signed the facegoing insurum.m. Market M Walter Notary Public My commission expires: 9/21/26
COUNT VIIII	



The items below are required in order to complete your application and shall be submitted when the application if filed.

- 1. A complete copy of the last deed of record for proof of ownership
- 2. An annexation boundary plat/map for recordation at the Wake County Register of Deeds Office (mylar plat) prepared by a professional land surveyor showing the boundaries of the area or property for annexation into the Town of Rolesville.
- 3. A complete copy of the written metes and bounds description based on the annexation boundary plat/map.

SECTION 1 - LOCATION

Is the area contiguous with the existing primary corporate limits? Satellite corporate limits is not primary. \Box Yes or \swarrow No Note: If the land is contiguous to any existing corporate limits, the proposed annexation boundary will include all intervening right-of ways for streets, easements, and other areas as stated in North Carelina General Statute §160–131(1).

SECTION 2 - VESTED RIGHTS

NC General Statues require petitioners of both contiguous and non-contiguous annexations to file a signed statement declaring whether vested rights have been established in accordance with G.S. 160A-385.1 or 153A-344.1 for properties subject to the petition. Do you declare vested rights for the property subject to this petition? \Box Yes or X No

SECTION 3 - PROPERTY DETAILS

PIN Number	Real Estate ID Number	Deed Book Number	Page Number	Acreage To Be Annexed	Wake County Assessed Value
1757758529	0493307	DB 018421	PG 00370	24.08	\$
		DB	PG		\$
		DB	PG		\$

SECTION 4 - SIGNATURES AND VERIFICATION

We, the undersigned owners of the real properties contained in the metes and bounds description and plat/map attached hereto, respectfully request that the area described above be annexed and made part of the Town of Rolesville. North Carolina_By signing below, we acknowledge that all information is correct.

• If property awned by INDIVIDUALS (NOTE: All legal owners must sign including both husband and wife)

Signature of

Date Signed

Signature of Owner #2

Date Signed

• If property owned by a COMPANY OR CORPORATION (NOTE: The company or corporation must be legally registered with the State of North Carolina – Office of the Secretary of State)

Land Investment Company Name of Corporation Stephen Ellis Signature of Registered Agent Printed Name of Registered Agent 6801 Falls of Neuse Rd. Suite 108 Roleigh, NC 27615 Address, State, Zip of Registered Office: anny anny Notary Public for said Court and Stars in Million, in the above signed individuality appeared before me this day and signed the foregoing instrument. day of August 4, 20 Million (1997) and and an and a second strument. North Carolina, ____Wake County Anne B. Oakler Witness my hand and official seal, this Votary Public Notary Seal Iv commission expires: TAR STORTER

NORTH CARCLINA



WARRANTY DEED

THIS DEED, made this 24th day of March, 1959, by Romie C. Watkins and wife, Irma Kirkland Watkins; Claiborne Watkins and wife, Elizabeth Watkins; Ione Ayscue and husband, Frvin R. Ayscue; Hilton Watkins and wife, Excell Watkins; Jane Watkins Puffin and husband, Carlie Ruffin; Louis Watkins and wife, NewdosfecWatkins; David M. Watkins, single, all of Wake County, North Carolina, and Lois Watkins Ward and husband, Leo Ward, of Durham County, North Carolina, of the first part, to Donnell Watkins and wife, Daisy Watkins, of Wake County, North Carolina, of the second part;

WITNESSETH:

That said parties of the first part, in consideration of One Hundred (5100.00) Dollars, and other valuable considerations to them paid by said parties of the second part, the receipt of which is hereby acknowledged, have bargained and sold, and by these presents do grant, bargain, sell, and convey to said parties of the second part, their heirs and assigns, a certain tract or parcel of land in Wake County, State of North Carolina, adjoining the lands of J. M. Jones, Mrs. A. P. Upchurch, and others, and bounded as follows:

BEGINNING at stake on the North side of the Traboro Road, corner of Lot No. 4; thence along the lines of Lot No. 4, N. 22° 30° E. 658 feet to a stake; thence N. 47° 30° E. 1223 feet to a stake; thence N. 3100 feet to a stake and pointers on the south side of Powell's Creek; thence up with the various courses of said Creek to a stake corner of Lot No. 2; thence South along the line of Lot No. 2, 4600 feet to a take on the Kelly Branch 50° feet West of J. M. Jones' corner; therca lown with the various courses of said branch to a stake corner of J. M. Jones; thence along Jones line S. 23° %. 966 feet to a stake on the Kelly Branch 50° feet Mest of J. M. Jones' corner; therca lown with the various courses of said branch to a stake corner of J. M. Jones; thence along Jones line S. 23° %. 966 feet to a stake on the North side of the Traboro Road; thence along said road N. 81° %. 600 feet to the BEGINNING, containing 114-3/4 acres and being Lot No. 3 of the division of the lards of the late John M. Watkins, a min of which is recorded in Book of Maps in the Negister of Deeds office for Wake County; ard being the same property conveyed by deed racorded in Book 752, page 599, Wake County Benls'try.

There is excepted from the above described tract of land a one (1) acre tract, more or less, and more particularly described as follows:

All that certain tract or parcel of land located in Wake Forest Townshir, Wake Forest, North Carolina, and being on the North side

ELLIS NASSIF



BCOK 1318 PAGE 335 (SEAL) (SEAL) (SEAL) (SEAL) (SEAL) David M. Watkins Ini. 911 (SEAL) - Ce (SEAL) Leo Ward NORTH CAROLINA WAKE COUNTY I, DUTTED OF MELGON, Notary Public, do hereby certify that Nomie O. Watkins, Irma Kirkland Watkins, Claiborne Watkins, Elizabeth Watkins, Ione Ayscue, Wilton Watkins, Excell Watkins, Jane Watkins Ruffin, Louis Watkins, NewSSW Watkins, David M. Watkins, Lois Watkins Ward, and Leo Ward, each personally appeared before me this day and acknowledged the due execution of the foregoing instrument. SITNESS my hand and Notarial seal, this the 31 day of March, Betti p pimego Notary Publid My commission extress Man - 23, 1959 NORTH CAROLINA DURHAM COUNTY mar S. Sellere I, ..., Notary Fublic, do hereby that Frvin R. Ayscue personally appeared before me this day and used the due execution of the foregoing instrument. ITNESS my hand and Notarial scal, this the 25 day of March, mae A. Sellens Notary Public My commission expires: 9-13-59

ANTH CAROLINA - Hanser COINTY CCCK 1318 PAGE 336 1. $\frac{7}{2}$ acc $\frac{7}{2}$ hard $\frac{1}{2}$, Notary Public, do hereby cartify that Carlie Suffin personally appeared before me this day and acknowledged the due execution of the foregoing instrument. WITNESS my hand and Notarial seal, this the 26 day of March, AC'S Thenry Si sac Phrase Notary Public mission expires: STATE OF NORTH CAROLINA: Wake County: The foregoing Certificate(s) of Bettie Jerniga A Notary Public of Wake County, State of North Carolina Mac A. Sellara A Notary Public of Durkey Bounty, State of North Carolina Henry Lanac Rhodes Attor Fitter Mundanyus an anis at the are adjudged to be in due form and correct. Let the instrument, with the certificate be registered. Witness my hand this the day of The no 195 8 Safet Clerk Superior Court, Wake County, N. C. Filed for registration at 1:100 Pm June 10V 8 _o'clock. G _day of. Rake and registered in the office of the Register of Deeds for County, in Book 1318 Page 333 Probate 254 Paid ke Register of Deeds Registration \$

	0 599
STATE OF NORTH CAROLINA	FROM KOMie O. Watking
WAKE COUNTY	Dr. King - Up di 1
THIS DEED, Made this 25th pay of Debus	ToIma_Turfland_ Wattens
Wark tomin O. Was	4kina -
Watking for the paried of har national life them	reparting to her children who during her
of County and State of OO	it Careling, of the second part:
in consideration of Och allars and natural	lever and ages time
him paid by Orma Kickland	Watking 10
the receipt of which is hereby acknowledged, hall_bargained and sold, and	by these presents do 2 st grant, bargain, sell and convey to said
parcel of land in force, Mrs. a. f. U.S.	uate of Alex T. Consoling and others, and bounded as follows, viz:
Beginning at stake on the N.side	of the l'Arboro Road, corner of lot No.4; thence along t
lines of lot No.4, N. 22 deg. 30' E. stake, thence N. 3100 ft. to a stake	658' to a stake; thence N. 47 deg. 30' E. 1223' to a and pointers on the south side of Fowell's Creek:
thence up with the various courses o	or said Creek to a stake corner of lot Nc.2; thence sout
along the line of lut No.2, 4600' to	a stake on the Kelly Branch 50' West of J. M. Jones'
corner, thence down with the various	courses of said Branch to a stake corner of T. M. Tone
thence along Jones line s. 23 deg. W	1. 886' to a stake on the north side of the Parboro Road
thence along said road N. 81 deg. W.	600' to the beginning, containing one wundred and
fourteen and three-fourths acres(114	(2) and being lot No.3 or the Division or the lands of
the late John M. Watkins, a map of w	thich is recorded in the book of maps in the Register of
Deeds office for Wake County -	
	121
	5.
	2
,	
TO HAVE AND TO HOLD the aforesaid eract or parcel of land, and all	privileges and appurtenances thereto belonging, to the said
And the said forme O. Walking	for him self
and heirs, executors and administrators, covenant & with	said aller Fucken Watking, her)
brances, and that de de la hereby forever warrant and w	I have not a sign to convey in fee simple, that the same are free and clear from all encum-
	as energy decent the said the to the same against the trains of an persons whomsever.
IN TESTIMONY WILFPEOF AL	O. Walking
IN TESTIMONT WHEREOF, He sala	
ha_ hereunto set_ his hand and scal, the day and y	year first above written.
MATR .	Jonie (Watterns (Seal)
Altest:	(Seal)
	(Seal)
STATE OF NOPTH CAPOLINA Wall	(Seal)
I, and Books Augustice of the	Jury do, hereby certify that
hafore me this day and acknowledged the fue execution of the anneyed Deed	f Convergence: and the said
being by me privately exemined, separate and epart from her seid-bushand, to	outhing her voluntary execution of the same, doth state that she signed the same fresh and
voluntarily, Without fear or compulsion of her said husband or any other pres	on, and that she doth still voluntarily assent theretor. Let the inclument, with this certifi
ento, borregistered.	mail
Witness my hand and y many seal, thisd	lay of A.D. 193_0
NODTH CADOLINA W. C.	(Seal)
The foregoing settificate of	
and toregoing germonic of the set	Susting at the Alla K.
County, State of 1/01/1 Carpenno, is adjudged to be	correct Let the instrument, with the certificates, be registered.
County, State of 1/0.14. Contraction, is adjudged to be Witness my hand, this 1. day of 10, and 1, A.D. 1	correct Let the instrument, with the certificates, be registered.
County, State of 1/2014 September is adjudged to be Witness my hand, this / 2 day of 10 action, A.D. I Filed for registration on the / 24 day of 10 action Deeds for Wake County, N. C., this 97th day of March	correct Let the instrument, with the certificates, be registered. 93. Mallis Whitlord, A. 194K. Clerk Superior Court. -, 1938., at 9
County, State of // O. A. Contraction, is adjudged to be Witness my hand, this / E. day of March, A.D. I Filed for registration on the / Aday of March Deeds for Wake County, N. C., this 9th day of March	correct. Let the instrument, with the certificates, be registered. 1930. Nallis Whithlord, A. Yerk, Clerk Superior Court. , 1932., at 9
County, State of 1. 2014	correct. Let the instrument, with the certificates, be registered. 1930. Mallie Millord, Alyth. Clerk Superior Court. 1930. at 9
County, State of 1. 2014. Spiteture, is adjudged to be Witness my hand, this 1. day of 1. day of 1. A.D. 1 Filed for registration on the 1. day of 1. March Deeds for Wake County, N. C., this 9. day of 1. March	correct. Let the instrument, with the certificates, be registered. (93) Mallis Millorle of yet, Clerk Superior Court. , 1930, at 9
County, State of 1/2015. Control is adjudged to be Witness my hand, this 1. day of 100000000000000000000000000000000000	correct. Let the instrument, with the certificates, be registered. (93) Mallis Mikillord, Alfred, Clerk Superior Court. , 1930, at 9



BKBM2005PG02287

WAKE COUNTY, NC TAMMY L. BRUNNER REGISTER OF DEEDS PRESENTED & RECORDED ON 02-16-2022 AT 12:53:17 STATE OF NC REAL ESTATE EXCISE TAX: \$1,445.00 BOOK: 018921 PAGE: 00370 - 00374

NORTH CAROLINA SPECIAL WARRANTY DEED

Excise Tax: \$1445.00

Wake County REID Out of 0046970

Mail/Box to: Grantee

This instrument was prepared by: Kenneth L. Eagle, 105 Weston Estates Way, Cary, NC 27513

Brief description for the Index: 25.125 Acres, north side of Mitchell Mill Road (NCSR 2224)

THIS DEED made this day of February 2022, by and between					
GRANTOR Mitchell Mill Road Investors LLC, a North Carolina limited liability company		GRANTEE	GRANTEE		
		Ellis Land Investment Company, LLC, a North Carolina limited liability company			
Address:	105 Weston Estates Way Cary, NC 27513	Address: 6801 Falls of Neuse Road, Suite Raleigh, NC 27615	: 108		

Enter in appropriate block for each Grantor and Grantee: name, mailing address, and, if appropriate, character of entity, e.g. corporation or partnership.

The designation Grantor and Grantee as used herein shall include said parties, their heirs, successors, and assigns, and shall include singular, plural, masculine, feminine or neuter as required by context.

WITNESSETH, that the Grantor, for a valuable consideration paid by the Grantee, the receipt of which is hereby acknowledged, has and by these presents does grant, bargain, sell and convey unto the Grantee in fee simple, all that certain lot or parcel of land situated in Wake Forest Township, Wake County, North Carolina and more particularly described as follows (the "Property"):

SEE EXHIBIT A ATTACHED HERETO AND INCORPORATED BY REFERENCE.

The Property was acquired by Grantor by instrument recorded in Book 11505, Page 2324.

None of the Property herein conveyed includes the primary residence of Grantor.

<u>Submitted electronically by Ellis & Winters LLP in compliance with North Carolina statutes governing recordable documents and the terms of the submitter agreement with the Wake County Register of Deeds.</u>

TO HAVE AND TO HOLD the Property and all privileges and appurtenances thereto belonging to the Grantee in fee simple.

And the Grantor covenants with the Grantee, that Grantor has done nothing to impair such title as Grantor received, and Grantor will warrant and defend the title against the lawful claims of all persons claiming by, under or through Grantor, other than the following exceptions:

- 1. Ad valorem property taxes for 2022 and subsequent years.
- 2. Right of way of Mitchell Mill Road (North Carolina Secondary Road 2224).
- 3. All rights of way, easements, restrictions, agreements, and other matters of record affecting the Property recorded in the office of the Register of Deeds for Wake County, North Carolina, including, without limitation, the following:
 - a. Easement to Carolina Power and Light Company recorded in Book 2830, Page 24.
 - b. Rights of others entitled thereto in and to the continued uninterrupted flow of any portion of Kelly Branch on or adjoining the Property.
 - c. Riparian or littoral rights incident to any branches, creeks, streams, or other waters on or adjoining the Property.
 - d. Rights of adjoining property owners in and to any ditches on or adjoining the Property.
 - e. Matters that would be shown by a current survey of the Property.

IN WITNESS WHEREOF, the Grantor has caused this Deed to be executed in its company name by its duly authorized official, as of the day and year first above written.

Mitchell Mill Road Investors LLC, a North Carolina limited liability company

By: Name: TIMOTHER & Smit

Title: Vice President

Wake County, North Carolina

I certify that the following person personally appear foregoing Deed: <u>TIMOTHY</u> R. SMITH	red before me this day and acknowledged to me that he executed the
Date: February 15, 2022 (affix seal or stamp here) $NOTAP_L$	Hamon J. Ward Notary Public Printed/Typed Name: Lauren L. Ward My Commission Expires: Aug. 3, 2024

EXHIBIT A PROPERTY DESCRIPTION

LYING AND BEING in Wake Forest Township, Wake County, North Carolina, adjoining the right of way of Mitchell Mill Road (North Carolina Secondary Road 2224), and being more particularly described as follows (all recording references are to the office of the Register of Deeds for Wake County, North Carolina):

BEGINNING at an iron pipe found in the north right-of-way line of Mitchell Mill Road, said point being a common corner with the southeast corner of property owned now or formerly by Donnell Watkens (see Deed Book 1318, Page 333); then running along the following lines:

With the eastern boundary line of Watkens, North 18 degrees 12 minutes 00 seconds East 893.03 feet to an iron pipe set;

Continuing with the eastern boundary line of Watkens, North 18 degrees 12 minutes 00 seconds East 50.00 feet to a point in the Kelly Branch;

The along center line of Kelly Branch:

North 39 degrees 14 minutes 44 seconds East 7.31 feet; South 56 degrees 54 minutes 35 seconds East 48.61 feet; South 70 degrees 54 minutes 35 seconds East 24.61 feet; North 70 degrees 48 minutes 45 seconds East 25 .45 feet; South 40 degrees 27 minutes 21 seconds East 28.19 feet; South 89 degrees 23 minutes 35 seconds East 32.88 feet; South 36 degrees 29 minutes 25 seconds East 17.15 feet; North 87 degrees 07 minutes 56 seconds East 15.72 feet; South 51 degrees 42 minutes 06 seconds East 21.14 feet; South 89 degrees 24 minutes 42 seconds East 33.05 feet; South 02 degrees 01 minutes 35 seconds East 17.48 feet; South 81 degrees 55 minutes 17 seconds East 15.31 feet; North 69 degrees 56 minutes 02 seconds East 14.47 feet; South 47 degrees 41 minutes 48 seconds East 9.88 feet; South 10 degrees 48 minutes 42 seconds West 17.64 feet; South 60 degrees 44 minutes 48 seconds East 46.31 feet; South 37 degrees 39 minutes 08 seconds East 49.81 feet; South 12 degrees 22 minutes 12 seconds East 24.12 feet; South 29 degrees 48 minutes 24 seconds East 30.28 feet; South 82 degrees 27 minutes 46 seconds East 31.99 feet; South 59 degrees 14 minutes 13 seconds East 17.52 feet: South 84 degrees 17 minutes 27 seconds East 25.35 feet; South 34 degrees 56 minutes 33 seconds East 75.35 feet; South 76 degrees 29 minutes 04 seconds East 27.94 feet; South 53 degrees 56 minutes 59 seconds East 23.48 feet; North 62 degrees 42 minutes 03 seconds East 27.98 feet; South 12 degrees 06 minutes 52 seconds East 39.01 feet; South 19 degrees 20 minutes 23 seconds East 50.44 feet;

South 77 degrees 09 minutes 16 seconds East 19.10 feet; South 07 degrees 23 minutes 00 seconds East 8.05 feet; South 27 degrees 33 minutes 46 seconds West 17.80 feet; South 02 degrees 52 minutes 45 seconds East 17.25 feet; South 56 degrees 32 minutes 15 seconds East 19.64 feet; South 12 degrees 52 minutes 10 seconds East 26.30 feet; South 77 degrees 21 minutes 17 seconds East 19.17 feet; South 22 degrees 18 minutes 41 seconds East 19.39 feet; South 65 degrees 53 minutes 52 seconds East 24.78 feet; South 19 degrees 31 minutes 10 seconds West 13.15 feet; South 71 degrees 14minutes 44 seconds East 11.65 feet; North 78 degrees 16 minutes 05 seconds East 30.35 feet; North 66 degrees 40 minutes 56 seconds East 29.69 feet; North 52 degrees 43 minutes 45 seconds East 52.03 feet; South 84 degrees 05 minutes 40 seconds East 20.63 feet; North 75 degrees 05 minutes 35 seconds East 16.99 feet; South 68 degrees 27 minutes 23 seconds East 16.87 feet; North 81 degrees 44 minutes 06 seconds East 14.34 feet; South 74 degrees 50 minutes 19 seconds East 19.97 feet; North 49 degrees 38 minutes 31 seconds East 44.60 feet; South 62 degrees 45 minutes 51 seconds East 22.37 feet; North 57 degrees 04 minutes 06 seconds East 21.98 feet; South 85 degrees 24 minutes 11 seconds East 37.57 feet; North 62 degrees 13 minutes 03 seconds East 23 .16 feet; South 54 degrees 59 minutes 08 seconds East 19.65 feet; South 15 degrees 17 minutes 54 seconds East 38.18 feet; South 05 degrees 38 minutes 36 seconds East 33.15 feet; South 15 degrees 59 minutes 03 seconds West 8.22 feet; South 53 degrees 28 minutes 36 seconds West 20. 78 feet; South 27 degrees 04 minutes 40 seconds East 79.74 feet; North 82 degrees 47 minutes 20 seconds East 21.90 feet; South 66 degrees 58 minutes 30 seconds East 28.16 feet; South 81 degrees 40 minutes 19 seconds East 27.96 feet; South 51 degrees 33 minutes 15 seconds East 16.75 feet; South 06 degrees 37 minutes 45 seconds West 10.90 feet; South 19 degrees 04 minutes 40 seconds West 14.50 feet; South 48 degrees 42 minutes 21 seconds East 17.71 feet; South 61 degrees 22 minutes 03 seconds East 39.43 feet; and

South 36 degrees 38 minutes 48 seconds East 39.44 feet to a point in the center line of Kelly Branch, in the western boundary line of property owned now or formerly by Joseph H. Wagner (see deeds recorded in Book 5816, Page 277 and Book 2619, Page 775 and maps recorded in Book of Maps 1985, Page 2212 and Book of Maps 1993, Page 446);

then leaving the center line of Kelly Creek and running with the western boundary line of said property of Wagner along the following two (2) lines:

South 01 degrees 27 minutes 33 seconds East 19.71 feet; and

South 00 degrees 27 minutes 33 seconds East 10.00 feet to a point in the eastern boundary line of the Property herein described and a common corner with the southwest corner of said property of Wagner and with the northwest corner of property owned now or formerly by Charles Spencer Jones (see deed recorded in Book 10112, Page 1534);

then with the western boundary line of said property of Jones, along the following two (2) lines:

South 00 degrees 27 minutes 33 seconds East 10.00 feet; and

South 00 degrees 27 minutes 33 seconds East 552.55 feet to a spike set in the centerline of Mitchell Mill Road;

then along the centerline of Mitchell Mill Road the following lines:

North 71 degrees 32 minutes 39 seconds West 24.07 feet; North 74 degrees 13 minutes 18 seconds West 51.42 feet; North 75 degrees 48 minutes 52 seconds West 259.87 feet; North 75 degrees 14 minutes 33 seconds West 152.68 feet; North 74 degrees 15 minutes 20 seconds West 51.96 feet; North 74 degrees 59 minutes 06 seconds West 50.07 feet: North 74 degrees 05 minutes 27 seconds West 50.77 feet; North 73 degrees 34 minutes 20 seconds West 99.95 feet; North 72 degrees 45 minutes 42 seconds West 52.88 feet; North 73 degrees 37 minutes 20 seconds West 98.57 feet; North 74 degrees 31 minutes 11 seconds West 100.00 feet; North 75 degrees 15 minutes 51 seconds West 96.96 feet; North 75 degrees 39 minutes 32 seconds West 154.21 feet; North 76 degrees 20 minutes 42 seconds West 100.91 feet; North 77 degrees 07 minutes 54 seconds West 104.34 feet; and North 77 degrees 21 minutes 31 seconds West 70.97 feet to a spike set in the centerline of

Mitchell Mill Road, a common corner with the southeast corner of the aforesaid property owned now or formerly by Watkens;

then with the eastern boundary line of said property of Watkens, North 18 degrees 12 minutes 00 seconds East 30.14 feet to the point and place of BEGINNING,

and being Tract 1, containing a total of 25.125 acres (1.043 acres in the right of way of Mitchell Mill Road and 24.082 outside of the right of way of Mitchell Mill Road), as shown on a survey entitled "Boundary Survey of Hampton and Cole Property", prepared by Kenneth Close, Inc., dated July 10, 2003.

BOOK: 016701 PAGE: 00363 - 00364

BK016701PG00363

Excise Tax: \$0 Parcel# 443802

Prepared By: Gwynn & Edwards, P.A. (without title exam or tax advice)

Mail After Recording to: GRANTEE

NORTH CAROLINA GENERAL WARRANTY DEED

This Deed made this <u>15</u> day of February, 2017, by and between **DAISY WATKINS**, *GRANTOR, to* **RANDALL WATKINS and wife, LAURA WATKINS, GRANTEES**, whose mailing address is 3278 Landing Falls Lane, Raleigh, NC 27616.

The designation Grantor and Grantee as used herein shall include said parties, their heirs, successors, and assigns, and shall include singular, plural, masculine, feminine or neuter as required by context.

WITNESSETH, that the Grantor, for valuable consideration paid by the Grantee, the receipt of which is hereby acknowledged, has and by these presents does grant, bargain, sell and convey unto the Grantee in fee simple, all that certain lot or parcel of land situated in Wake County, North Carolina and more particularly described as follows:

BEING ALL OF TRACT 1 containing 10.524 gross acres as shown on map entitled "Exempt Subdivision for Randall Watkins, Wake Forest Township, Wake County, North Carolina" by Williams-Pearce and Assoc., Professional Land Surveyors, P.A., dated 12/08/2016 and recorded in Book of Maps 2017, Page 218, Wake County Registry.

This deed was prepared without a title search and drafting attorney makes no representation as to title or estate.

This is not the primary residence of the Grantor.

TO HAVE AND TO HOLD the aforesaid lot or parcel of land and all privileges and appurtenances thereto belonging to the Grantee in fee simple.

And the Grantor covenants with the Grantee, that Grantor is seized of the premises in fee simple, has the right to convey the same in fee simple, that title is marketable and free and clear of all encumbrances, and that Grantor will warrant and defend the title against the lawful claims of all persons

Submitted electronically by "Gwynn & Edwards, PA" in compliance with North Carolina statutes governing recordable documents and the terms of the submitter agreement with the Wake County Register of Deeds. whomsoever except for the exceptions hereinafter stated. Title to the property hereinabove described is subject to the following exceptions:

- 1. 2017 ad valorem taxes; and
- 2. Any rights of way and easements of record, if any.

The property hereinabove described was conveyed to Grantor by instrument recorded in Book 1318, Page 333, Wake County Registry.

A map of the above-described property is recorded in Book of Maps 2017, Page 218, Wake County Registry.

IN TESTIMONY WHEREOF, the Grantor has hereunto set her hand and seal the day and year first above written.

APRILITE FRANKLER HotoSEAD)ic AND ATA CAROL DAISY WATKINS

STATE OF NORTH CAROLINA

I, a Notary Public of the County and State aforesaid, certify that DAISY WATKINS personally appeared before me this day and acknowledged the voluntary execution of the foregoing instrument. Witness my hand and official stamp or seal, this <u>/</u>S day of FEBRUARY, 2017.

Notary Public Pranklin Count xpires: Notary Public

NOTARY BLIC

CHANGETTE L. THOXTON



S-333-16

LEGAL DESCRIPTIONS

5333 Mitchell Mill Road, Wake Forest, NC 27587 (PIN: 1757750520; REID: 0074789)

BEING all of Lot 1 as shown on that plat titled "Minor Subdivision for Daisy Watkins" by Williams-Pearce & Assoc., P.A., recorded in Book of Maps 2005, Page 2287, Wake County Registry.

3645 Rock Farm Road, Wake Forest, NC 27587 (PIN: 1757761273; REID: 0443802)

BEING all of Tract 1 containing 10.524 gross acres as shown on map entitled "Exempt Subdivision for Randall Watkins, Wake Forest Township, Wake County, North Carolina" by Williams-Pearce and Assoc., Professional Land Surveyors, P.A., dated 12/08/2016 and recorded in Book of Maps 2017, Page 218, Wake County Registry.

<u>0 Mitchell Mill Road, Wake Forest, NC 27587</u> (PIN: 1757778982; REID: 0443803)

BEGINNING at stake on the North side of the Traboro Road corner of Lot No. 4; thence along the lines of Lot No. 4, N. 22° 30' E. 658 feet to a stake; thence N. 47° 30' E. 1223 feet to a stake; thence N. 3100 feet to a stake and pointers on the south side of Powell's Creek; thence up with the various courses of said Creek to a stake corner of Lot No. 2; thence South along the line of Lot No. 2, 4600 feet to a stake on the Kelly Branch 50 feet West of J.M. Jones' corner; thence down with the various courses of said branch to a stake corner of J.M. Jones; thence along stake on the North side of the Traboro Road; thence along said road N. 81° W. 600 feet to the BEGINNING, containing 114-3/4 acres and being Lot No. 3 of the division of the lands of the late John M. Watkins, a map of which is recorded in Book of Maps in the Register of Deeds office for Wake County; and being the same property conveyed by deed recorded in Book 752, Page 599, Wake County Registry.

There is excepted from the above described tract of land a one (1) acre tract, more or less, and more particularly described as follows:

All that certain tract or parcel of land located in Wake Forest Township, Wake Forest, North Carolina, and being on the North side of the Tarboro Road, and containing 1 acre, more or less, and more particularly described as follows:

BEGINNING at a point in the center of the Tarboro Road, corner with the land of J.M. Jones; running thence along the center of said Tarboro Road N. 18° W. 210 feet to a point, corner with the land of R.O. Watkins; running thence along the R.O. Watkins' line N. 23° E. 210 feet to a point, another corner with the lands of R.O. Watkins; running thence along R.O. Watkins' line S. 18° E. 210 feet to a point in the line of J.M. Jones; running thence with the line of J.M. Jones S. 23° W. 210 feet to the point and place of BEGINNING, and being a part of Lot No. 3 of the J.M. Watkins' farm, according to a map and survey made by Pittman Stell, Surveyor, dated April, 1925.

5326 Mitchell Mill Road, Wake Forest, NC 27587 (PIN: 1757738648; REID: 0046970)

LYING AND BEING in Wake Forest Township, Wake County, North Carolina, adjoining the right of way of Mitchell Mill Road (North Carolina State Road 2224), and being more particularly described as follows:

TRACT ONE:

BEGINNING at an iron pipe found in the north right-of-way line of Mitchell Mill Road, said point being the common corner of Lois Jones Merriman Heirs' property and the southeast corner of Donnell Watkens (Deed Book 1318, Page 333, Wake County Registry); thence North 18 degrees 12 minutes 00 seconds East 893.03 feet to an iron pipe set; thence North 18 degrees 12 minutes 00 seconds East 50.00 feet to a point in the Kelly Branch the following courses and distances: North 39 degrees 14 minutes 44 seconds East 7.31 feet; South 56 degrees 54 minutes 35 seconds East 48.61 feet; South 70 degrees 54 minutes 35 seconds East 24.61 feet; North 70 degrees 48 minutes 45 seconds East 25.45 feet; South 40 degrees 27 minutes 21 seconds East 28.19 feet; South 89 degrees 23 minutes 35 seconds East 32.88 feet; South 36 degrees 29 minutes 25 seconds East 17.15 feet; North 87 degrees 07 minutes 56 seconds East 15.72 feet; South 51 degrees 42 minutes 06 seconds East 21.14 feet; South 89 degrees 24 minutes 42 seconds East 33.05 feet; South 02 degrees 01 minutes 35 seconds East 17.48 feet; South 81 degrees 55 minutes 17 seconds East 15.31 feet; North 69 degrees 56 minutes 02 seconds East 14.47 feet; South 47 degrees 41 minutes 48 seconds East 9.88 feet; South 10 degrees 48 minutes 42 seconds West 17.64 feet; South 60 degrees 44 minutes 48 seconds East 46.31 feet; South 37 degrees 39 minutes 08 seconds East 49.81 feet; South 12 degrees 22 minutes 12 seconds East 24.12 feet; South 29 degrees 48 minutes 24 seconds East 30.28 feet; South 82 degrees 27 minutes 46 seconds East 31.99 feet; South 59 degrees 14 minutes 13 seconds East 17.52 feet; South 84 degrees 17 minutes 27 seconds East 25.35 feet; South 34 degrees 56 minutes 33 seconds East 75.35 feet; South 76 degrees 29 minutes 04 seconds East 27.94 feet; South 53 degrees 56 minutes 59 seconds East 23.48 feet; North 62 degrees 42 minutes 03 seconds East 27.98 feet; South 12 degrees 06 minutes 52 seconds East 39.01 feet; South 19 degrees 20 minutes 23 seconds East 50.44 feet; South 77 degrees 09 minutes 16 seconds East 19.10 feet; South 07 degrees 23 minutes 00 seconds East 8.05 feet; South 27 degrees 33 minutes 46 seconds West 17.80 feet; South 02 degrees 52 minutes 45 seconds East 17.25 feet; South 56 degrees 32 minutes 15 seconds East 19.64 feet; South 12 degrees 52 minutes 10 seconds East 26.30 feet; South 77 degrees 21 minutes 17 seconds East 19.17 feet; South 22 degrees 18 minutes 41 seconds East 19.39 feet; South 65 degrees 53 minutes 52 seconds East 24.78 feet; South 19 degrees 31 minutes 10 seconds West 13.15 feet; South 71 degrees 14 minutes 44 seconds East 11.65 feet; North 78 degrees 16 minutes 05 seconds East 30.35 feet; North 66 degrees 40 minutes 56 seconds East 29.69 feet; North 52 degrees 43 minutes 45 seconds East 52.03 feet; South 84 degrees 05 minutes 40 seconds East 20.63 feet; North 75 degrees 05 minutes 35 seconds East 16.99 feet; South 68 degrees 27 minutes 23 seconds East 16.87 feet; North 81 degrees 44 minutes 06 seconds East 14.34 feet; South 74 degrees 50 minutes 19 seconds East 19.97 feet; North 49 degrees 38 minutes 31 seconds East 44.60 feet; South 62 degrees 45 minutes 51 seconds East 22.37 feet; North 57 degrees 04 minutes 06 seconds East 21.98 feet; South 85 degrees 24 minutes 11 seconds East 37.57 feet; North 62 degrees 13 minutes 03 seconds East 23.16 feet; South 54 degrees 59 minutes 08 seconds East 19.65 feet; South 15 degrees 17 minutes 54 seconds East 38.18 feet; South 05 degrees 38 minutes 36 seconds East 33.15 feet; South 15 degrees 59 minutes 03 seconds West 8.22 feet; South 53 degrees 28 minutes 36 seconds West 20.78 feet; South 27 degrees 04 minutes 40 seconds East 79.74 feet; North 82 degrees 47 minutes 20 seconds East 21.90 feet; South 66 degrees 58 minutes 30 seconds East 28.16 feet; South 81 degrees 40 minutes 19 seconds East 27.96 feet; South 51 degrees 33 minutes 15 seconds East 16.75 feet; South 06 degrees 37 minutes 45 seconds West 10.90 feet; South 19 degrees 04 minutes 40 seconds West 14.50 feet; South 48 degrees 42 minutes 21 seconds East 17.71 feet; South 61 degrees 22 minutes 03 seconds East 39.43 feet; South 36 degrees 38 minutes 48 seconds East 39.44 feet; South 00 degrees 27 minutes 33 seconds East 19.71 feet; South 00 degrees 27 minutes 33 seconds East 10.00 feet; South 00 degrees 27 minutes 33 seconds East 10.00 feet; then South 00 degrees 27 minutes 33 seconds East 552.55 feet to a spike set in the centerline of Mitchell Mill Road; thence along the centerline of Mitchell Mill Road the following courses and distances: North 71 degrees 32 minutes 39 seconds West 24.07 feet; North 74 degrees 13 minutes 18 seconds West 51.42 feet; North 75 degrees 48 minutes 52 seconds West 259.87 feet; North 75 degrees 14 minutes 33 seconds West 152.68 feet; North 74 degrees 15 minutes 20 seconds West 51.96 feet; North 74 degrees 59 minutes 06 seconds West 50.07 feet; North 74 degrees 05 minutes 27 seconds West 50.77 feet; North 73 degrees 34 minutes 20 seconds West 99.95 feet; North 72 degrees 45 minutes 42 seconds West 52.88 feet; North 73 degrees 37 minutes 20 seconds West 98.57 feet; North 74 degrees 31 minutes 11 seconds West 100.00 feet; North 75 degrees 15 minutes 51 seconds West 96.96 feet; North 75 degrees 39 minutes 32 seconds West 154.21 feet; North 76 degrees 20 minutes 42 seconds West 100.91 feet; North 77 degrees 07 minutes 54 seconds West 104.34 feet; North 77 degrees 21 minutes 31 seconds West 70.97 feet to a spike set in the centerline of Mitchell Mill Road; thence North 18 degrees 12 minutes 00 seconds East 30.14 feet to the point and place of BEGINNING and being Tract 1 containing a total of 25.125 acres (1.043 acres in the right-of-way of Mitchell Road and 24.082 outside of the rightof-way of Mitchell Mill Road), as shown on a survey entitled "Boundary Survey of Hampton and Cole Property", prepared by Kenneth Close, Inc., dated July 10, 2003.



ATTACHMENT 14 - ANX-22-04 Certificate of Sufficiency

CERTIFICATE OF SUFFICIENCY ANX22-04 - Hills at Harris Creek

To the Board of Commissioners of the Town of Rolesville, North Carolina:

I, <u>Robin E. Peyton</u>, Town Clerk, do hereby certify that I have investigated the attached petition and hereby make the following findings:

- a. The petition contains an adequate property description of the area(s) proposed for annexation.
- b. The area described in the petition is contiguous to the Town of Rolesville primary corporate limits as required by G.S. 160A-31.
- c. The petition is signed by all owners of real property lying in the area described therein.

In witness whereof, I have hereunto set my hand and affixed the seal of the Town of Rolesville, this 5th day of October 2022



Robin C, Peyton

Robin E. Peyton Town Clerk



Hills at Harris Creek: Board of Commissioners Meeting REZ-23-03

September 5, 2023 Rolesville Town Hall



Proposed Rezoning	PROPOSED CHANGES	Zoning Change to Neighborhood Center- Conditional Zoning and Residential High Density Conditional Zoning		Proposed construction of 152 single family homes and 115 townhomes (2.3 units/acre)	Approximately 2-acre commercial site (proposed day care facility)
	CURRENT STATUS	Rural residential land	Acreage: 115.94 acres	Current Zoning: Wake County R-30 (allows approximately 75 lots)	

Dronorad Razoning



Site Plan





Bike & Greenway Plan







Greenway Plan







Community Features

COMMUNITY POOL AND CABANA





Community Features

DOG PARK





Community Features

GREENWAY TRAILS




Community Features

POLLINATOR/WILDFLOWER GARDEN





Community Features

LANDSCAPED BOULEVARD STREETSCAPES







Community Features

PICKLEBALL COURT

INTEGRATED BIKE PATHS







Commercial Parcel





Site Plan

