## Downstream Analysis

# THE POINT - NORTH Package 3 

Rolesville, North Carolina

## DOWNSTREAM ANALYSIS

## CONSTRUCTION DRAWINGS

## Project Number:

Designed By:
AWH-20000
Daniel Wiebke, PE, CFM
Tommy Dabolt

DATE:
February 2024

## 플 <br> McAdAMS

2905 Meridian Parkway
Durham, North Carolina 27713
NC LIC. \# C-0293


# THE POINT - CD PACKAGE 3 <br> Downstream Analysis 

## GENERAL DESCRIPTION

The Point is a proposed residential development in Rolesville, North Carolina, located between Highway 401 and East Young Street/Rolesville Road. The development is approximately 300 acres, divided into a northern parcel and a southern parcel. This Stormwater Impact Analysis covers the development of the southern parcel only. The total development will consist of approximately 804 lots, a mixture of townhomes and various types of single-family housing, thirteen stormwater control measures, sidewalks, roadways, greenway trail, and associated infrastructure.

Of the total site area, approximately 34.1 acres of CD Package 3 will be treated by primary stormwater control measures (SCMs). The outflow from the SCM discharges into Harris Creek and leaves the property just upstream of the flooded quarry on the neighboring properties. The overall Harris Creek watershed to the southern property boundary extends northeast of the site to the other side of Highway 401 up to South Main Street as well as east of East Young Street, enveloping the previous two CD Packages, totaling approximately 822.22 acres. The CD Package 3 project site is 47.48 acres (including bypass areas), making up approximately $5 \%$ of the total watershed at this point, which is less than the $10 \%$ rule requirement. This point along Reedy Creek is referred to as the Downstream Point of Analysis (POA \#6) in this report.

A hydrologic analysis was performed for the 10-year storm event using both pre- and post-development conditions to determine if the proposed project will cause any impacts to flooding or channel degradation downstream of the project site, in accordance with the stormwater management performance standards for development set forth in the Rolesville Unified Development Ordinance Article 7, Section 7.5.4 - Standards.

The downstream analysis regulations are as follows:

## (B) General Standards

(1) Downstream Impact Analysis The downstream impact analysis must be performed in accordance with the "ten percent rule," and a copy of the analysis must be provided with the permit application. The purpose of the downstream impact analysis is to determine if the project will cause any impacts on flooding or channel degradation downstream of the project site. The analysis must include the assumptions, results and supporting calculations to show safe passage of post-Development design flows downstream. This analysis shall be performed at the outlet(s) of the site, and downstream at each tributary junction to the point(s) in the conveyance system where the area of the portion of the site draining into the system is less than or equal to ten percent of the total drainage area above that point.

## CALCULATION METHODOLOGY

- The SCS Curve Number Method was used to estimate direct runoff. A composite curve number was calculated for each subbasin using soils and land cover data.
- Depth-Duration Frequency (DDF) rainfall data was obtained from NOAA Atlas 14. Synthetic rainfall hyetographs were generated using frequency-based hypothetical storms assuming a storm duration of 1 day, intensity duration of 5 minutes, intensity position of $50 \%$, and a uniform distribution for all subbasins. Rainfall depths were input into the meteorological model within PondPack for peak flow rate calculations. Please reference the rainfall data section within this report for additional information.
- Hydrologic soil groups within each subbasin were determined using NRCS Web Soil Survey.
- Land cover conditions for the pre-development condition were based on survey provided by McAdams and aerial imagery for the site. Land cover conditions for the post-development condition were taken from the proposed layout. Offsite cover conditions were based onQL2 LiDAR, aerial imagery for the site, and Town of Apex Official Zoning Map.
- Existing conditions survey data and proposed grading was used for onsite topography. QL2 LiDAR topography data was obtained from North Carolina Spatial Data Download and used for offsite areas.
- The pre-development and post-development times of concentration were calculated using the SCS TR-55 Segmental Approach. The flow path was divided into the following segments where applicable: overland flow, concentrated flow, pipe flow, and channel flow. The travel time was then computed for each segment, from which the overall time of concentration was determined by taking the sum of each segmental time.
- The time of concentration to the proposed stormwater control measures was conservatively assumed to be 5 minutes.
- PondPack Version V8i was used in determining the pre- and post-development peak flow rates and routing calculations for the proposed stormwater control measures.
- Design and routing information for the proposed SCMs can be found in The Point - CD Package 3 Stormwater Impact Analysis report dated February 2023.


## DISCUSSION OF RESULTS

 project site makes up approximately $5 \%$ of the total watershed. Our analysis shows that peak flows in the post development 10 -year storm are less than in the pre development at point of analysis 6 . As shown in the Summary of Results section of this report, the proposed stormwater control measures provide the peak runoff control for the proposed development such that there are no adverse impacts to the existing downstream infrastructure. Therefore, the proposed project will not cause any impacts to flooding or channel degradation downstream of the project site, in accordance with the stormwater management performance standards for development set forth in the Rolesville Unified Development Ordinance Article 7, Section 7.5.4 - Standards.

## CONCLUSION

If the development on this tract is built as proposed within this report, then the requirements set forth in Town of Rolesville regulations will be met. However, modifications to the proposed development may require that this analysis be revised. Some modifications that would require this analysis to be revised include:

1. The proposed site impervious surface exceeds the amount accounted for in this report.
2. The post-development watershed breaks change significantly from those used to prepare this report.

The above modifications may result in the assumptions within this report becoming invalid. The computations within this report will need to be revisited if any of the above conditions become apparent as development of the proposed site moves forward.

| 1 | SUMMARYOF RESULTS |
| :---: | :--- |
| $\mathbf{2}$ | MISGELANEOUSSTEINFORMATION |
| $\mathbf{3}$ | PRE-DEVEOPMENTHMROLOGICCALOLLATIONS |
| $\mathbf{4}$ | POST-DEVEOPMENTHDROLOGICCALOLLATIONS |

SUMMARY OF RESULTS

RELEASE RATE MANAGEMENT RESULTS

| POINT OF ANALYSIS \#1 |  |  |  |
| :---: | :---: | :---: | :---: |
| Return Period | $\begin{gathered} \text { Pre-Dev } \\ \text { [cfs] } \end{gathered}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 283.5 | 219.4 | -23\% |
| 10-Year | 545.3 | 514.0 | -6\% |
| POINT OF ANALYSIS \#2 |  |  |  |
| Return Period | $\begin{gathered} \text { Pre-Dev } \\ \text { [cfs] } \end{gathered}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 77.3 | 70.0 | -9\% |
| 10-Year | 153.0 | 138.6 | -9\% |
| POINT OF ANALYSIS \#3 |  |  |  |
| Return Period | $\begin{aligned} & \text { Pre-Dev } \\ & \text { [cfs] } \end{aligned}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 8.7 | 6.7 | -23\% |
| 10-Year | 22.3 | 16.6 | -26\% |
| POINT OF ANALYSIS \#4 |  |  |  |
| Return Period | $\begin{gathered} \text { Pre-Dev } \\ \text { [cfs] } \end{gathered}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 344.2 | 248.9 | -28\% |
| 10-Year | 704.1 | 636.3 | -10\% |
| POINT OF ANALYSIS \#5 |  |  |  |
| Return Period | $\begin{aligned} & \text { Pre-Dev } \\ & \text { [cfs] } \end{aligned}$ | $\begin{aligned} & \text { Post-Dev } \\ & \text { [cfs] } \end{aligned}$ | \% Increase [\%] |
| 1-Year | 359.7 | 241.2 | -33\% |
| 10-Year | 787.2 | 611.7 | -22\% |
| POINT OF ANALYSIS \#6 |  |  |  |
| Return Period | $\begin{aligned} & \text { Pre-Dev } \\ & \text { [cfs] } \end{aligned}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 704.0 | 485.3 | -31\% |
| 10-Year | 1494.8 | 1231.6 | -18\% |
| POINT OF ANALYSIS \#7 |  |  |  |
| Return Period | $\begin{gathered} \text { Pre-Dev } \\ \text { [cfs] } \end{gathered}$ | $\begin{aligned} & \text { Post-Dev } \\ & \text { [cfs] } \end{aligned}$ | \% Increase [\%] |
| 1-Year | 2.5 | 2.5 | 0\% |
| 10-Year | 17.2 | 8.6 | -50\% |
| POINT OF ANALYSIS \#8 |  |  |  |
| Return Period | $\begin{gathered} \text { Pre-Dev } \\ \text { [cfs] } \end{gathered}$ | $\begin{gathered} \text { Post-Dev } \\ \text { [cfs] } \end{gathered}$ | \% Increase [\%] |
| 1-Year | 0.2 | 0.6 | 200\% |
| 10-Year | 4.3 | 3.7 | -14\% |

## MISCELLANEOUS SITE INFORMATION




| Name of Stream | Description | Class | Class Date | Index No. |
| :---: | :---: | :---: | :---: | :---: |
| NEUSE RIVER | From a point 0.5 mile upstream of Town of Wake Forest proposed water supply intake to Town of Wake Forest proposed water supply intake | WS-IV;NSW,CA | 07/01/04 | 27-(22) |
| NEUSE RIVER | From Town of Wake Forest proposed water supply intake to mouth of Beddingfield Creek | C;NSW | 08/03/92 | 27-(22.5) |
| Smith Creek | From source to a point 0.3 mile downstream of Franklin-Wake County Line | WS-II;HQW,NSW | 08/03/92 | 27-23-(1) |
| Smith Creek (Wake Forest Reservoir) | From a point 0.3 mile downstream of Franklin-Wake County Line to dam at Wake Reservoir | WS-II;HQW,NSW, CA | 08/03/92 | 27-23-(1.5) |
| Smith Creek | From dam at Wake Forest Reservoir to Neuse River | C;NSW | 05/01/88 | 27-23-(2) |
| Austin Creek (Mitchell Pond) | From source to Smith Creek | C;NSW | 07/01/96 | 27-23-3 |
| Hatters Branch | From source to Smith Creek | C;NSW | 05/01/88 | 27-23-4 |
| Spring Branch | From source to Hatters Branch | C;NSW | 05/01/88 | 27-23-4-1 |
| Sanford Creek | From source to Smith Creek | C;NSW | 05/01/88 | 27-23-5 |
| Toms Creek (Mill Creek) | From source to Neuse River | C;NSW | 05/01/88 | 27-24 |
| Perry Creek (Greshams Lake) | From source to dam at Greshams Lake | B;NSW | 05/01/88 | 27-25-(1) |
| Perry Creek | From dam at Greshams Lake to Neuse River | C;NSW | 05/01/88 | 27-25-(2) |
| Unnamed Tributary near Neuse | From source to dam at Camp Durant | B;NSW | 05/01/88 | 27-25-3-(1) |
| Unnamed Tributary near Neuse | From dam at Camp Durant to Perry Creek | C;NSW | 05/01/88 | 27-25-3-(2) |
| Harris Creek (Peeples Creek) | From source to Neuse River | C;NSW | 05/01/88 | 27-26 |
| (Wake Crossroads Lake) <br> Hodges Mill Creek (Lake Mirl) | From source to water intake at Lake Mirl | B;NSW | 05/01/88 | 27-26-1-(1) |
| Hodges Mill Creek | From water intake at Lake Mirl to Harris Creek | C;NSW | 05/01/88 | 27-26-1-(2) |
| Beaverdam Creek (west side of Neuse River) | From source to Neuse River | C;NSW | 05/01/88 | 27-27 |
| Rocky Creek | From source to Neuse River | C;NSW | 05/01/88 | 27-28 |
| Beaverdam Creek (east side of Neuse River) (Neuseco Lake, Beaverdam Lake) | From soruce to Neuse River | C;NSW | 05/01/88 | 27-29 |
| Bridges Creek (Bridges Lake) | From source to Neuse River | C;NSW | 05/01/88 | 27-30 |
| Milburnie Creek (Milburnie Lake) | From source to Neuse River | C;NSW | 05/01/88 | 27-31 |
| Mango Creek | From source to Neuse River | C;NSW | 05/01/88 | 27-32 |
| Crabtree Creek | From source to backwaters of Crabtree Lake | C;NSW | 05/01/88 | 27-33-(1) |
| Turkey Creek | From source to Crabtree Creek | C;NSW | 05/01/88 | 27-33-2 |
| Coles Branch | From source to Crabtree Creek | C;NSW | 05/01/88 | 27-33-3 |
| South Fork Coles Branch | From source to Coles Branch | C;NSW | 05/01/88 | 27-33-3-1 |
| Crabtree Creek (Crabtree Lake) | From backwaters of Crabtree Lake to mouth of Richlands Creek | B;NSW | 04/01/94 | 27-33-(3.5) |



FLOOD HAZARD INFORMATION

## SEE FIS REPORT FOR ZONE DESCRIPTIONS AND INDEX MAP

 FOR FIRM PANEL LAYOUT DOCUMENTATION ARE ALSO AVAILABLE IN DINGITAL FORMAT AT HITPS://FRIS.NC.GOV/FRI| SHZARD AEAS | Without Base Flood Elevation (BFE) With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway |
| :---: | :---: |
| OTHER AREAS OF FLOOD HAZARD | $0.2 \%$ Annual Chance Flood Hazard, Areas of 1\% Annual Chance Flood with Average Depth Less Than One Foot or With Drainage Areas of Less Than One Square Mile Zone $X$ Future Conditions 1\% Annual Chance Flood Hazard Zone $X$ Area with Reduced Flood Risk due to Levee See Notes Zone $X$ |
| $\begin{aligned} & \text { OTHER } \\ & \text { AREAS } \end{aligned}$ | Areas Determined to be Outside the 0.2\% Annual Chance Flood plain Zone X |
| GENERALSTRUCTURES | Channel, Culvert, or Storm Sewer Levee, Dike, or Floodwall |
|  | 012-18-2- Cross Sections with 1\% Annual Chance Water Surface Elevation (BFE) <br> (8) ----- Coastal Transect |
|  | ---- Coastal Transect Baseline $\qquad$ Profile Baseline |
|  | Hydrographic Feature |
|  | Limit of Study |
| features | Jurisdiction Boundary |



## VNGU

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FLOOD HAZARD INFORMATION SEE FIS REPORT FOR ZONE DESCRIPTIONS AND INDEX MAP SEE FIS REPORT FOR ZONE DEECRRPTIIONS
FOR PANEL LAYOUT THE INFORMATION DEPICTED ON THIS MAP AND SUPPORTING
DOCUMENTATION ARE AISO AVALIABLEIN HTTPS://FRIS.NC.GOV/FRIS HTPS.//MSC FEMA.GOV


NOTES TO USERS


















LLimit of Moderate Wave Action (LiMWA)

SCALE
0

 1 inch $=500$ feet $\quad 1: 6,000$ \begin{tabular}{cccc}

0 \& 250 \& 500 \& | 1,000 |
| :---: |
| Feet | <br>

\hline 0 \& 75 \& 150 \& Meters
\end{tabular}

PANEL LOCATOR


## VWGコ 窥

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2.3 .3 .2
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20175800K MAP NUMBER
3720175800 K



## MAP LEGEND

| Area of Interest (AOI) | $\square$ | C |
| :---: | :---: | :---: |
| Area of Interest (AOI) | $\square$ | C/D |
| Soils $\square$ |  |  |
| Soil Rating Polygons $\square$ |  |  |
| ] A | $\square$ | Not rated or not available |
| A/D | Water Fea | ures |
|  | $\sim$ | Streams and Canals |
| B |  |  |
|  | Transpo | tion |
| B/D | H+ | Rails |
| C | - | Interstate Highways |
| C/D | - | US Routes |
| D | $\approx$ | Major Roads |
| Not rated or not available | $\geq$ | Local Roads |
| Soil Rating Lines | Backgro |  |
| $\cdots$ A |  | Aerial Photography |
| $\cdots$ A/D |  |  |
| $\cdots B$ |  |  |
| $\cdots$ B/D |  |  |
| $\cdots \mathrm{C}$ |  |  |
| $\cdots$ C/D |  |  |
| $\cdots$ D |  |  |
| * Not rated or not available |  |  |
| Soil Rating Points |  |  |
| $\square \quad \mathrm{A}$ |  |  |
| $\square \quad \mathrm{A} / \mathrm{D}$ |  |  |
| $\square \quad \mathrm{B}$ |  |  |
| $\square \quad \mathrm{B} / \mathrm{D}$ |  |  |

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements
Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)
Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
Soil Survey Area: Wake County, North Carolina
Survey Area Data: Version 18, Sep 16, 2019
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 11, 2019-Oct 19, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
| :---: | :---: | :---: | :---: | :---: |
| AaA | Altavista fine sandy loam, 0 to 4 percent slopes, rarely flooded | C | 4.1 | 1.4\% |
| AuA | Augusta fine sandy loam, 0 to 2 percent slopes, rarely flooded | C/D | 10.1 | 3.3\% |
| ChA | Chewacla and Wehadkee soils, 0 to 2 percent slopes, frequently flooded | B/D | 27.0 | 8.9\% |
| HeB | Helena sandy loam, 2 to 6 percent slopes | D | 7.1 | 2.4\% |
| RgB | Rawlings-Rion complex, 2 to 6 percent slopes | C | 43.5 | 14.4\% |
| RgC | Rawlings-Rion complex, 6 to 10 percent slopes | C | 45.6 | 15.1\% |
| RgD | Rawlings-Rion complex, 10 to 15 percent slopes | C | 15.0 | 5.0\% |
| W | Water |  | 9.7 | 3.2\% |
| WaB | Wake-Rolesville complex, 2 to 6 percent slopes, very rocky | D | 7.4 | 2.4\% |
| WaC | Wake-Rolesville complex, 6 to 10 percent slopes, very rocky | D | 29.0 | 9.6\% |
| WaD | Wake-Rolesville complex, 10 to 15 percent slopes, very rocky | D | 51.1 | 16.9\% |
| WaE | Wake-Rolesville complex, 15 to 25 percent slopes, very rocky | D | 28.9 | 9.6\% |
| WeB | Wedowee sandy loam, 2 to 6 percent slopes | D | 0.2 | 0.1\% |
| WfB | Wedowee-Saw complex, 2 to 6 percent slopes | D | 22.9 | 7.6\% |
| WgC | Wedowee-Urban land complex, 6 to 15 percent slopes | D | 0.1 | 0.0\% |
| Totals for Area of Interest |  |  | 301.6 | 100.0\% |

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified
Tie-break Rule: Higher

NOAA Atlas 14, Volume 2, Version 3
Location name: Wake Forest, North Carolina, USA* Latitude: $\mathbf{3 5 . 9 0 5 3}^{\circ}$, Longitude: $\mathbf{- 7 8 . 4 5 2}^{\circ}$

Elevation: $354.67 \mathrm{ft}^{* *}$

* source: ESRI Maps
** source: USGS



## POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland
PF tabular | PF_graphical | Maps \& aerials

## PF tabular

| PDS-based point precipitation frequency estimates with 90\% confidence intervals (in inches) ${ }^{1}$ |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Duration | Average recurrence interval (years) |  |  |  |  |  |  |  |  |  |
|  | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | $\mathbf{0 . 4 0 3}$ <br> $(0.369-0.442)$ | $\mathbf{0 . 4 6 8}$ <br> $(0.429-0.512)$ | $\mathbf{0 . 5 3 4}$ <br> $(0.489-0.582)$ | $\mathbf{0 . 6 0 0}$ <br> $(0.548-0.654)$ | $\mathbf{0 . 6 6 6}$ <br> $(0.606-0.726)$ | $\mathbf{0 . 7 1 8}$ <br> $(0.651-0.783)$ | 0.765 <br> $(0.690-0.834)$ | $\mathbf{0 . 8 0 7}$ <br> $(0.723-0.881)$ | $\mathbf{0 . 8 5 3}$ <br> $(0.758-0.932)$ | 0.895 <br> $(0.789-0.979)$ |
| 10-min | $\mathbf{0 . 6 4 4}$ <br> $(0.590-0.705)$ | 0.749 <br> $(0.687-0.818)$ | $\mathbf{0 . 8 5 5}$ <br> $(0.783-0.933)$ | 0.959 <br> $(0.877-1.05)$ | 1.06 <br> $(0.966-1.16)$ | $\begin{gathered} 1.14 \\ (1.04-1.25) \\ \hline \end{gathered}$ | $\begin{gathered} 1.22 \\ (1.10-1.33) \\ \hline \end{gathered}$ | $\begin{gathered} 1.28 \\ (1.15-1.40) \\ \hline \end{gathered}$ | $\begin{gathered} 1.35 \\ (1.20-1.47) \\ \hline \end{gathered}$ | $\begin{gathered} 1.41 \\ (1.24-1.54) \\ \hline \end{gathered}$ |
| 15-min | $\mathbf{0 . 8 0 6}$ <br> $(0.738-0.882)$ | $\begin{gathered} \hline \boldsymbol{0 . 9 4 2} \\ (0.863-1.03) \\ \hline \hline \end{gathered}$ | $\begin{gathered} 1.08 \\ (0.991-1.18) \\ \hline \end{gathered}$ | $\begin{gathered} 1.21 \\ (1.11-1.32) \\ \hline \end{gathered}$ | $\begin{gathered} 1.35 \\ (1.22-1.47) \\ \hline \end{gathered}$ | $\begin{gathered} 1.45 \\ (1.31-1.58) \\ \hline \end{gathered}$ | $\begin{gathered} 1.54 \\ (1.39-1.68) \\ \hline \end{gathered}$ | $\begin{gathered} 1.61 \\ (1.45-1.76) \\ \hline \end{gathered}$ | $\begin{gathered} 1.70 \\ (1.51-1.86) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.77 \\ (1.56-1.94) \\ \hline \end{gathered}$ |
| 30-min | $\begin{gathered} 1.10 \\ (1.01-1.21) \end{gathered}$ | $\begin{gathered} 1.30 \\ (1.19-1.42) \end{gathered}$ | $\begin{gathered} 1.54 \\ (1.41-1.68) \end{gathered}$ | $\begin{gathered} 1.76 \\ (1.61-1.92) \end{gathered}$ | $\begin{gathered} 1.99 \\ (1.81-2.17) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 1 8} \\ (1.98-2.38) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 3 5} \\ (2.12-2.57) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 5 1} \\ (2.25-2.74) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 7 0} \\ (2.40-2.95) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 8 7} \\ (2.52-3.14) \end{gathered}$ |
| 60-min | $\begin{gathered} 1.38 \\ (1.26-1.51) \\ \hline \end{gathered}$ | $\begin{gathered} 1.63 \\ (1.50-1.78) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.97 \\ (1.81-2.15) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 2 9} \\ (2.09-2.50) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 6 5} \\ (2.42-2.89) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 9 6} \\ (2.68-3.22) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.24 \\ (2.92-3.53) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.52 \\ (3.16-3.85) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.88 \\ (3.45-4.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.18 \\ (3.69-4.58) \\ \hline \end{gathered}$ |
| 2-hr | $\begin{gathered} \hline 1.61 \\ (1.46-1.78) \\ \hline \end{gathered}$ | $\begin{gathered} 1.92 \\ (1.75-2.10) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 3 4} \\ (2.13-2.56) \\ \hline \end{gathered}$ | $\begin{gathered} 2.75 \\ (2.49-3.01) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.23 \\ (2.91-3.54) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.66 \\ (3.28-4.00) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.07 \\ (3.63-4.45) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.49 \\ (3.98-4.91) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.04 \\ (4.42-5.51) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.52 \\ (4.80-6.05) \\ \hline \end{gathered}$ |
| 3-hr | $\begin{gathered} 1.71 \\ (1.55-1.89) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 0 3} \\ (1.85-2.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 4 9} \\ (2.26-2.74) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 . 9 4} \\ (2.67-3.24) \\ \hline \end{gathered}$ | $\begin{gathered} 3.50 \\ (3.15-3.84) \\ \hline \end{gathered}$ | $\begin{gathered} 3.99 \\ (3.58-4.39) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.49 \\ (3.98-4.92) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.00 \\ (4.41-5.48) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.69 \\ (4.96-6.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.32 \\ (5.45-6.95) \\ \hline \end{gathered}$ |
| 6-hr | $\begin{gathered} 2.05 \\ (1.87-2.26) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 4 4} \\ (2.23-2.68) \end{gathered}$ | $\begin{gathered} 2.99 \\ (2.72-3.28) \end{gathered}$ | $\begin{gathered} 3.54 \\ (3.22-3.88) \\ \hline \end{gathered}$ | $\begin{gathered} 4.22 \\ (3.82-4.62) \end{gathered}$ | $\begin{gathered} 4.84 \\ (4.35-5.29) \end{gathered}$ | $\begin{gathered} \mathbf{5 . 4 6} \\ (4.86-5.96) \end{gathered}$ | $\begin{gathered} 6.12 \\ (5.39-6.67) \end{gathered}$ | $\begin{gathered} 7.00 \\ (6.10-7.64) \end{gathered}$ | $\begin{gathered} 7.82 \\ (6.72-8.55) \end{gathered}$ |
| 12-hr | $\begin{gathered} \hline \mathbf{2 . 4 1} \\ (2.21-2.66) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 . 8 7} \\ (2.64-3.15) \\ \hline \end{gathered}$ | $\begin{gathered} 3.54 \\ (3.24-3.88) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.21 \\ (3.84-4.62) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.07 \\ (4.59-5.53) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{5 . 8 5} \\ (5.26-6.36) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.64 \\ (5.91-7.22) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.49 \\ (6.59-8.14) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.66 \\ (7.50-9.41) \\ \hline \end{gathered}$ | $\begin{gathered} 9.76 \\ (8.32-10.6) \\ \hline \end{gathered}$ |
| 24-h | $\begin{gathered} \hline \mathbf{2 . 8 6} \\ (2.66-3.08) \\ \hline \end{gathered}$ | $\begin{gathered} 3.46 \\ (3.22-3.73) \\ \hline \end{gathered}$ | $\begin{gathered} 4.35 \\ (4.04-4.69) \\ \hline \end{gathered}$ | $\begin{gathered} 5.06 \\ (4.69-5.44) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{6 . 0 2} \\ (5.57-6.49) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{6 . 8 0} \\ (6.27-7.32) \\ \hline \hline \end{gathered}$ | $\begin{gathered} \hline 7.60 \\ (6.98-8.19) \\ \hline \hline \end{gathered}$ | $\begin{gathered} 8.43 \\ (7.71-9.09) \\ \hline \end{gathered}$ | $\begin{gathered} 9.58 \\ (8.71-10.3) \\ \hline \end{gathered}$ | $\begin{gathered} 10.5 \\ (9.50-11.3) \\ \hline \end{gathered}$ |
| 2-day | $\begin{gathered} 3.32 \\ (3.09-3.57) \\ \hline \end{gathered}$ | $\begin{gathered} 3.99 \\ (3.72-4.30) \\ \hline \end{gathered}$ | $\begin{gathered} 4.98 \\ (4.64-5.37) \\ \hline \end{gathered}$ | $\begin{gathered} 5.77 \\ (5.35-6.21) \\ \hline \end{gathered}$ | $\begin{gathered} 6.83 \\ (6.32-7.36) \\ \hline \end{gathered}$ | $\begin{gathered} 7.68 \\ (7.09-8.27) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.56 \\ (7.87-9.22) \\ \hline \end{gathered}$ | $\begin{gathered} 9.46 \\ (8.66-10.2) \\ \hline \end{gathered}$ | $\begin{gathered} 10.7 \\ (9.74-11.6) \\ \hline \end{gathered}$ | $\begin{gathered} 11.7 \\ (10.6-12.7) \\ \hline \end{gathered}$ |
| 3-day | $\begin{gathered} 3.52 \\ (3.28-3.77) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.23 \\ (3.94-4.54) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{5 . 2 5} \\ (4.89-5.63) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.06 \\ (5.64-6.50) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.17 \\ (6.64-7.69) \\ \hline \end{gathered}$ | 8.05 <br> $(7.44-8.64)$ <br> 8.42 | $\begin{gathered} \hline \hline 8.96 \\ (8.25-9.62) \\ \hline \hline \end{gathered}$ | $\begin{gathered} 9.89 \\ (9.07-10.6) \\ \hline \end{gathered}$ | $\begin{gathered} 11.2 \\ (10.2-12.1) \\ \hline \end{gathered}$ | $\begin{gathered} 12.2 \\ (11.1-13.2) \\ \hline \end{gathered}$ |
| 4-day | $\begin{gathered} 3.72 \\ (3.47-3.98) \end{gathered}$ | $\begin{gathered} \hline 4.46 \\ (4.17-4.77) \end{gathered}$ | $\begin{gathered} 5.52 \\ (5.15-5.90) \end{gathered}$ | $\begin{gathered} \mathbf{6 . 3 5} \\ (5.92-6.79) \end{gathered}$ | $\begin{gathered} \hline 7.50 \\ (6.96-8.01) \end{gathered}$ | $\begin{gathered} 8.42 \\ (7.79-9.00) \end{gathered}$ | $\begin{gathered} \hline 9.36 \\ (8.63-10.0) \\ \hline \end{gathered}$ | $\begin{gathered} 10.3 \\ (9.49-11.1) \end{gathered}$ | $\begin{gathered} 11.7 \\ (10.7-12.5) \end{gathered}$ | $\begin{gathered} 12.7 \\ (11.6-13.7) \\ \hline \end{gathered}$ |
| 7-day | $\begin{gathered} 4.31 \\ (4.04-4.61) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{5 . 1 5} \\ (4.82-5.50) \end{gathered}$ | $\begin{gathered} 6.29 \\ (5.88-6.71) \\ \hline \end{gathered}$ | $\begin{gathered} 7.19 \\ (6.72-7.68) \\ \hline \end{gathered}$ | $\begin{gathered} 8.43 \\ (7.85-9.00) \\ \hline \end{gathered}$ | $\begin{gathered} 9.42 \\ (8.75-10.1) \\ \hline \end{gathered}$ | $\begin{gathered} 10.4 \\ (9.66-11.2) \\ \hline \end{gathered}$ | $\begin{gathered} 11.5 \\ (10.6-12.3) \\ \hline \end{gathered}$ | $\begin{gathered} 12.9 \\ (11.8-13.9) \end{gathered}$ | $\begin{gathered} 14.1 \\ (12.8-15.1) \end{gathered}$ |
| 10-day | $\begin{gathered} \hline 4.91 \\ (4.61-5.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.85 \\ (5.48-6.23) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.04 \\ (6.60-7.50) \\ \hline \end{gathered}$ | $\begin{gathered} 7.99 \\ (7.47-8.50) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 9.26 \\ (8.64-9.86) \\ \hline \end{gathered}$ | $\begin{gathered} 10.3 \\ (9.55-10.9) \\ \hline \end{gathered}$ | $\begin{gathered} 11.3 \\ (10.5-12.0) \\ \hline \end{gathered}$ | $\begin{gathered} 12.3 \\ (11.4-13.2) \end{gathered}$ | $\begin{gathered} 13.7 \\ (12.6-14.7) \\ \hline \end{gathered}$ | $\begin{gathered} 14.8 \\ (13.6-15.9) \end{gathered}$ |
| 20-day | $\begin{gathered} \hline 6.59 \\ (6.20-7.02) \\ \hline \end{gathered}$ | $\begin{gathered} 7.79 \\ (7.32-8.29) \\ \hline \end{gathered}$ | $\begin{gathered} 9.23 \\ (8.67-9.81) \\ \hline \end{gathered}$ | $\begin{gathered} 10.4 \\ (9.72-11.0) \end{gathered}$ | $\begin{gathered} 11.9 \\ (11.1-12.7) \end{gathered}$ | $\begin{gathered} 13.1 \\ (12.2-14.0) \end{gathered}$ | $\begin{gathered} 14.3 \\ (13.3-15.3) \end{gathered}$ | $\begin{gathered} 15.6 \\ (14.5-16.6) \end{gathered}$ | $\begin{gathered} 17.3 \\ (16.0-18.5) \end{gathered}$ | $\begin{gathered} 18.6 \\ (17.1-19.9) \end{gathered}$ |
| 30-day | $\begin{gathered} \hline 8.18 \\ (7.72-8.69) \\ \hline \end{gathered}$ | $\begin{gathered} 9.63 \\ (9.08-10.2) \\ \hline \end{gathered}$ | $\begin{gathered} 11.2 \\ (10.6-11.9) \\ \hline \end{gathered}$ | $\begin{gathered} 12.5 \\ (11.7-13.2) \\ \hline \end{gathered}$ | $\begin{gathered} 14.1 \\ (13.2-15.0) \\ \hline \end{gathered}$ | $\begin{gathered} 15.4 \\ (14.4-16.3) \\ \hline \end{gathered}$ | $\begin{gathered} 16.6 \\ (15.5-17.7) \\ \hline \end{gathered}$ | $\begin{gathered} 17.9 \\ (16.7-19.0) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 19.5 \\ (18.1-20.9) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 0 . 8} \\ (19.3-22.3) \\ \hline \end{gathered}$ |
| 45-day | $\begin{gathered} 10.4 \\ (9.89-11.0) \\ \hline \end{gathered}$ | $\begin{gathered} 12.2 \\ (11.6-12.9) \\ \hline \end{gathered}$ | $\begin{gathered} 14.0 \\ (13.3-14.8) \\ \hline \end{gathered}$ | $\begin{gathered} 15.4 \\ (14.6-16.2) \\ \hline \end{gathered}$ | $\begin{gathered} 17.2 \\ (16.3-18.1) \\ \hline \end{gathered}$ | $\begin{gathered} 18.6 \\ (17.5-19.6) \\ \hline \end{gathered}$ | $\begin{gathered} 19.9 \\ (18.7-21.0) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 1 . 2} \\ (19.9-22.5) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 3 . 0} \\ (21.5-24.4) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 4 . 3} \\ (22.7-25.8) \\ \hline \end{gathered}$ |
| 60-day | $\begin{gathered} 12.5 \\ (11.9-13.1) \\ \hline \end{gathered}$ | $\begin{gathered} 14.6 \\ (13.9-15.4) \\ \hline \end{gathered}$ | $\begin{gathered} 16.6 \\ (15.7-17.4) \\ \hline \end{gathered}$ | $\begin{gathered} 18.1 \\ (17.1-19.0) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 0 . 0} \\ (19.0-21.1) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 1 . 5} \\ (20.3-22.6) \\ \hline \end{gathered}$ | $\begin{gathered} 22.9 \\ (21.6-24.1) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 4 . 2} \\ (22.9-25.6) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 6 . 0} \\ (24.5-27.5) \\ \hline \end{gathered}$ | $\begin{gathered} 27.4 \\ (25.7-29.0) \\ \hline \end{gathered}$ |

[^0]
## PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: $35.9053^{\circ}$, Longitude: $-78.4520^{\circ}$


| Average recurrence <br> interval <br> (years) |
| :---: |
| -1 |
| -2 |
| -5 |
| -10 |
| -25 |
| -50 |
| — 100 |
| — 200 |
| — 500 |
| -1000 |



| Duration |  |
| :---: | :---: |
|  | — 2 -day — 3 -day — 4 -day — ${ }^{\text {-day }}$ — 10 -day — ${ }^{20 \text {-day }}$ — 40 -day — 60 -day |

NOAA Atlas 14, Volume 2, Version 3
Created (GMT): Wed Jun 3 19:09:38 2020
Back to Top
Maps \& aerials

## Small scale terrain



Large scale aerial


Back to Top

## US Department of Commerce

National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov
Disclaimer

## PRE-DEVELOPMENT HYDROLOGIC CALCULATIONS

| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Total Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Crops | B | 78 | Yes | 0 | 0.00 | 23.79 |
| Crops | B/D | 89 | Yes | 0 | 0.00 | 0.13 |
| Crops | D | 89 | Yes | 0 | 0.00 | 6.70 |
| Low Density Residential | B | 68 |  | 20 | 0.06 | 0.29 |
| Low Density Residential | D | 84 |  | 20 | 4.53 | 22.63 |
| Medium Density Residential | B | 70 |  | 25 | 0.29 | 1.14 |
| Medium Density Residential | D | 85 |  | 25 | 0.02 | 0.08 |
| Mixed Use Neighborhood | B | 85 |  | 65 | 9.26 | 14.25 |
| Mixed Use Neighborhood | B/D | 92 |  | 65 | 2.68 | 4.12 |
| Mixed Use Neighborhood | C | 90 |  | 65 | 1.16 | 1.79 |
| Mixed Use Neighborhood | D | 92 |  | 65 | 10.60 | 16.30 |
| Open | B | 61 | Yes | 0 | 0.00 | 16.63 |
| Open | B/D | 80 | Yes | 0 | 0.00 | 0.38 |
| Open | D | 80 | Yes | 0 | 0.00 | 8.88 |
| Pond | B | 100 | Yes | 0 | 0.00 | 0.13 |
| Pond | B/D | 100 | Yes | 0 | 0.00 | 0.02 |
| Pond | D | 100 | Yes | 0 | 0.00 | 6.78 |
| Roadway |  | 98 |  | 100 | 7.98 | 7.98 |
| Roof | B | 98 | Yes | 100 | 0.35 | 0.35 |
| Roof | D | 98 | Yes | 100 | 0.00 | 0.00 |
| School | B | 88 |  | 72 | 12.90 | 17.91 |
| School | B/D | 93 |  | 72 | 6.11 | 8.49 |
| School | D | 93 |  | 72 | 43.84 | 60.89 |
| Trail | B | 82 | Yes | 100 | 0.52 | 0.52 |
| Trail | B/D | 89 | Yes | 100 | 0.01 | 0.01 |
| Trail | D | 89 | Yes | 100 | 0.11 | 0.11 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 8.52 |
| Wooded | B/D | 77 | Yes | 0 | 0.00 | 6.40 |
| Wooded | C | 70 | Yes | 0 | 0.00 | 0.03 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 1.61 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Crops | B | 78 | Yes | 0 | 0.00 | 1.34 |
| Crops | D | 89 | Yes | 0 | 0.00 | 0.55 |
| Low Density Residential | D | 84 |  | 20 | 3.59 | 17.96 |
| Mixed Use Neighborhood | B | 85 |  | 65 | 3.44 | 5.30 |
| Mixed Use Neighborhood | B/D | 92 |  | 65 | 0.58 | 0.89 |
| Mixed Use Neighborhood | D | 92 |  | 65 | 8.11 | 12.48 |
| Open | B | 61 | Yes | 0 | 0.00 | 0.92 |
| Open | D | 80 | Yes | 0 | 0.00 | 0.75 |
| Roadway |  | 98 |  | 100 | 13.35 | 13.35 |
| Trail | B | 82 | Yes | 100 | 0.07 | 0.07 |
| Trail | D | 89 | Yes | 100 | 0.22 | 0.22 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 2.53 |
| Wooded | B/D | 77 | Yes | 0 | 0.00 | 1.21 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 2.33 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 100 | ft |
| Top Elev = | 427.00 | ft |
| Bot Elev = | 426.00 | ft |
| Height = | 1 | ft |
| Slope $=$ | 0.0100 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.17 | cultivated soils, residue cover |
| $\mathrm{P}(2$-year/24-hour) $=$ | 3.46 | inches (Rolesville, NC) |
| Segment Time $=$ | 13.74 | minutes |

Segment 2: Concentrated Flow

| Length $=$ | 2541 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 426.00 | ft |
| Bot Elev $=$ | 368.00 | ft |
| Height $=$ | 58 | ft |
| Slope $=$ | 0.0228 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? $=$ | No |  |
| Velocity $=$ | 2.44 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 17.37 | minutes |


| Time of Concentration $=$ | 31.12 | minutes |
| ---: | :---: | :--- |
| SCS Lag Time $=$ | 18.67 | minutes (SCS Lag = 0.6* Tc) |
| Time Increment $=$ | 5.41 | minutes $\left(=0.29^{*}\right.$ SCS Lag) |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open | C | 74 | Yes | 0 | 0.00 | 1.23 |
| Open | D | 80 | Yes | 0 | 0.00 | 1.45 |
| Wooded | C | 70 | Yes | 0 | 0.00 | 0.85 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 4.59 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |  | Segment 2: Concentrated Flow |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 100 | ft |  | Length $=$ | 698 | ft |
| Top Elev = | 368.00 | ft |  | Top Elev = | 363.00 | ft |
| Bot Elev = | 363.00 | ft |  | Bot Elev = | 336.00 | ft |
| Height = | 5 | ft |  | Height = | 27 | ft |
| Slope $=$ | 0.0500 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope = | 0.0387 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |  | Paved ? = | No |  |
| P (2-year/24-hour) = | 3.46 | inches (Rolesville |  | Velocity = | 3.17 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 9.51 | minutes |  | Segment Time $=$ | 3.67 | minutes |
| Segment 3: Channel Flow |  |  |  |  |  |  |
| Length $=$ | 249 | ft |  |  |  |  |
| Top Elev = | 336.00 | ft |  |  |  |  |
| Bot Elev = | 333.00 | ft |  |  |  |  |
| Height $=$ | 3 | ft |  |  |  |  |
| Slope $=$ | 0.0120 | $\mathrm{ft} / \mathrm{ft}$ |  |  |  |  |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |  |  |  |  |
| Flow Area = | 15.00 | sf (assume 5'w x | channel) |  |  |  |
| Wetted Perimeter = | 11.00 | If (assume $5^{\prime} \mathrm{w} \times 3$ | hannel) |  |  |  |
| Channel Velocity = | 4.47 | $\mathrm{ft} / \mathrm{sec}$ |  |  |  |  |
| Segment Time $=$ | 0.93 | minutes |  |  |  |  |
| Time of Concentration = |  |  | 14.11 | minutes |  |  |
|  |  | SCS Lag Time = | 8.46 | minutes (SCS Lag $=0.6^{*} \mathrm{Tc}$ ) |  |  |
|  |  | Time Increment = | 2.45 | minutes ( $=0.29 *$ SCS Lag) |  |  |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Crops | B | 78 | Yes | 0 | 0.00 | 6.35 |
| Crops | B/D | 89 | Yes | 0 | 0.00 | 0.16 |
| Crops | D | 89 | Yes | 0 | 0.00 | 3.19 |
| Mixed Use Neighborhood | B | 85 |  | 65 | 1.18 | 1.82 |
| Mixed Use Neighborhood | B/D | 92 |  | 65 | 0.56 | 0.86 |
| Mixed Use Neighborhood | C | 90 |  | 65 | 0.68 | 1.04 |
| Mixed Use Neighborhood | D | 92 |  | 65 | 5.08 | 7.81 |
| Open | A | 39 | Yes | 0 | 0.00 | 1.44 |
| Open | B | 61 | Yes | 0 | 0.00 | 10.63 |
| Open | B/D | 80 | Yes | 0 | 0.00 | 8.78 |
| Open | C | 74 | Yes | 0 | 0.00 | 4.91 |
| Open | D | 80 | Yes | 0 | 0.00 | 20.40 |
| Roadway |  | 98 |  | 100 | 7.47 | 7.47 |
| Roof | B | 98 | Yes | 100 | 0.11 | 0.11 |
| Roof | B/D | 98 | Yes | 100 | 0.00 | 0.00 |
| Roof | D | 98 | Yes | 100 | 0.03 | 0.03 |
| Trail | A | 72 | Yes | 100 | 0.17 | 0.17 |
| Trail | B | 82 | Yes | 100 | 0.39 | 0.39 |
| Trail | B/D | 89 | Yes | 100 | 0.19 | 0.19 |
| Trail | D | 89 | Yes | 100 | 0.69 | 0.69 |
| Wooded | A | 30 | Yes | 0 | 0.00 | 0.00 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 3.07 |
| Wooded | B/D | 77 | Yes | 0 | 0.00 | 9.47 |
| Wooded | C | 70 | Yes | 0 | 0.00 | 7.15 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 12.43 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |  | Segment 2: Concentrated Flow |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 100 | ft |  |  | Length = | 1855 | ft |
| Top Elev = | 389.00 | ft |  |  | Top Elev = | 388.00 | ft |
| Bot Elev = | 388.00 | ft |  |  | Bot Elev = | 339.00 | ft |
| Height = | 1 | ft |  |  | Height = | 49 | ft |
| Slope = | 0.0100 | $\mathrm{ft} / \mathrm{ft}$ |  |  | Slope = | 0.0264 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.17 | cultivated soils, | e cover |  | Paved ? = | No |  |
| P (2-year/24-hour) = | 3.46 | inches (Rolesville |  |  | Velocity = | 2.62 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 13.74 | minutes |  |  | Segment Time $=$ | 11.79 | minutes |
| Segment 3: Channel Flow |  |  |  |  |  |  |  |
| Length $=$ | 2366 | ft |  |  |  |  |  |
| Top Elev = | 339.00 | ft |  |  |  |  |  |
| Bot Elev = | 302.00 | ft |  |  |  |  |  |
| Height $=$ | 37 | ft |  |  |  |  |  |
| Slope = | 0.0156 | $\mathrm{ft} / \mathrm{ft}$ |  |  |  |  |  |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |  |  |  |  |  |
| Flow Area = | 15.00 | sf (assume 5'w x | channel) |  |  |  |  |
| Wetted Perimeter = | 11.00 | If (assume $5^{\prime} \mathrm{w} \times 3$ | channel) |  |  |  |  |
| Channel Velocity = | 5.09 | $\mathrm{ft} / \mathrm{sec}$ |  |  |  |  |  |
| Segment Time $=$ | 7.74 | minutes |  |  |  |  |  |
| Time of Concentration = |  |  | 33.28 | minutes |  |  |  |
|  |  | SCS Lag Time $=$ | 19.97 | minutes (SCS Lag $=0.6^{*} \mathrm{Tc}$ ) |  |  |  |
|  |  | Time Increment = | 5.79 | minutes ( $=0.29 *$ SCS Lag) |  |  |  |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Business Park | A | 89 |  | 85 | 0.01 | 0.01 |
| Business Park | B | 92 |  | 85 | 0.02 | 0.02 |
| Business Park | D | 95 |  | 85 | 0.03 | 0.03 |
| Commercial | D | 95 |  | 85 | 0.86 | 1.01 |
| High Density Residential | B | 85 |  | 65 | 46.03 | 70.82 |
| High Density Residential | D | 92 |  | 65 | 33.44 | 51.45 |
| Low Density Residential | B | 65 |  | 20 | 0.18 | 0.92 |
| Low Density Residential | D | 82 |  | 20 | 0.37 | 1.85 |
| Medium Density Residential | A | 54 |  | 25 | 0.00 | 0.01 |
| Medium Density Residential | B | 70 |  | 25 | 11.98 | 47.92 |
| Medium Density Residential | D | 85 |  | 25 | 10.74 | 42.98 |
| Mixed Use Neighborhood | B | 85 |  | 65 | 4.36 | 6.71 |
| Mixed Use Neighborhood | B/D | 92 |  | 65 | 2.15 | 3.31 |
| Mixed Use Neighborhood | D | 92 |  | 65 | 7.50 | 11.55 |
| Open | A | 49 | Yes | 0 | 0.00 | 1.90 |
| Open | B | 69 | Yes | 0 | 0.00 | 10.33 |
| Open | D | 84 | Yes | 0 | 0.00 | 11.86 |
| Preserved Open Space | A | 39 |  | 0 | 0.00 | 0.39 |
| Preserved Open Space | B | 61 |  | 0 | 0.00 | 4.48 |
| Preserved Open Space | D | 80 |  | 0 | 0.00 | 34.05 |
| Roadway |  | 98 |  | 100 | 49.05 | 49.05 |
| School | B | 88 |  | 72 | 0.34 | 0.47 |
| School | D | 93 |  | 72 | 5.12 | 7.11 |
| Town Center | B | 92 |  | 85 | 5.55 | 6.53 |
| Town Center | D | 95 |  | 85 | 1.48 | 1.74 |
| Trail | A | 72 | Yes | 100 | 0.02 | 0.02 |
| Trail | B | 82 | Yes | 100 | 0.43 | 0.43 |
| Trail | D | 89 | Yes | 100 | 1.93 | 1.93 |
| Wooded | A | 30 | Yes | 0 | 0.00 | 0.70 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 4.50 |
| Wooded | B/D | 77 | Yes | 0 | 0.00 | 0.15 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 50.68 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

Segment 1: Overland Flow |  |  |  |
| ---: | :---: | :--- |
| Length $=$ | 100 | ft |
| Top Elev $=$ | 440.00 | ft |
| Bot Elev $=$ | 438.00 | ft |
| Height $=$ | 2 | ft |
| Slope $=$ | 0.0200 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |
| $\mathrm{P}(2$-year/24-hour $)$ | $=$ | 3.46 |
| Segment Time $=$ | 13.72 | inches (Rolesville, NC$)$ |
| minutes |  |  |

## Segment 2: Concentrated Flow

| Length $=$ | 2989 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 438.00 | ft |
| Bot Elev $=$ | 372.00 | ft |
| Height $=$ | 66 | ft |
| Slope $=$ | 0.0221 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? $=$ | No |  |
| Velocity $=$ | 2.40 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 20.78 | minutes |

Segment 3: Open Water Flow

| Length $=$ | 655 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 372.00 | ft |
| Bot Elev $=$ | 372.00 | ft |
| Segment Time $=$ | 0.00 | minutes |

Segment 4: Concentrated Flow

| Length $=$ | 2379 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 372.00 | ft |
| Bot Elev $=$ | 320.00 | ft |
| Height $=$ | 52 | ft |
| Slope $=$ | 0.0219 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? $=$ | No |  |
| Velocity $=$ | 2.39 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 16.62 | minutes |

Segment 5: Channel Flow

| Length $=$ | 3730 | ft |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Top Elev = | 320.00 | ft |  |  |
| Bot Elev = | 292.00 | ft |  |  |
| Height $=$ | 28 | ft |  |  |
| Slope $=$ | 0.0075 | $\mathrm{ft} / \mathrm{ft}$ |  |  |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |  |  |
| Flow Area = | 32.00 | sf (assume 8'w x | channel) |  |
| Wetted Perimeter = | 16.00 | If (assume $8^{\prime} w x$ | hannel) |  |
| Channel Velocity = | 4.55 | $\mathrm{ft} / \mathrm{sec}$ |  |  |
| Segment Time = | 13.65 | minutes |  |  |
|  | Time of Concentration = |  | 64.78 | minutes |
|  |  | SCS Lag Time = | 38.87 | minutes (SCS Lag $=0.6$ * Tc ) |
|  |  | Time Increment = | 11.27 | minutes ( $=0.29 *$ SCS Lag) |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Trail | D | 89 | Yes | 100 | 0.16 | 0.16 |
| Wooded | A | 30 | Yes | 0 | 0.00 | 0.37 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 8.04 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow | Segment 2: Concentrated Flow |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 100 | ft |  | Length $=$ | 599 | ft |
| Top Elev = | 386.00 | ft |  | Top Elev = | 380.00 | ft |
| Bot Elev = | 380.00 | ft |  | Bot Elev = | 288.00 | ft |
| Height $=$ | 6 | ft |  | Height $=$ | 92 | ft |
| Slope $=$ | 0.0600 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope = | 0.1536 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.40 | wooded |  | Paved ? = | No |  |
| P (2-year/24-hour) $=$ | 3.46 | inches (Rolesvill |  | Velocity = | 6.32 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 13.31 | minutes |  | Segment Time $=$ | 1.58 | minutes |
|  | Time | Concentration = | 14.89 | minutes |  |  |
|  |  | SCS Lag Time = | 8.93 | minutes (SCS Lag $=0.6^{*} \mathrm{Tc}$ ) |  |  |
|  |  | Time Increment = | 2.59 | minutes ( $=0.29 *$ SCS Lag) |  |  |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open | A | 39 | Yes | 0 | 0.00 | 0.70 |
| Open | B | 61 | Yes | 0 | 0.00 | 1.00 |
| Open | D | 80 | Yes | 0 | 0.00 | 0.06 |
| Trail | B | 82 | Yes | 100 | 0.15 | 0.15 |
| Wooded | A | 30 | Yes | 0 | 0.00 | 0.66 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 7.85 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 5.24 |
| Total Area | 15.67 ac |  |  |  |  |  |
| Total Impervious Area | 0.15 ac |  |  |  |  |  |
| Onsite Area | 15.67 ac |  |  |  |  |  |
| Onsite Impervious Area | 0.15 ac |  |  |  |  |  |
| Percent Impervious | 1 \% |  |  |  |  |  |
| Composite Curve Number | 61 |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 100 | ft |
| Top Elev = | 418.00 | ft |
| Bot Elev = | 417.00 | ft |
| Height = | 1 | ft |
| Slope = | 0.0100 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |
| P (2-year/24-hour) = | 3.46 | inches (Rolesville, NC) |
| Segment Time $=$ | 18.11 | minutes |

Segment 2: Concentrated Flow

| Length $=$ | 593 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 417.00 | ft |
| Bot Elev $=$ | 338.00 | ft |
| Height $=$ | 79 | ft |
| Slope $=$ | 0.1332 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? $=$ | No |  |
| Velocity $=$ | 5.89 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 1.68 | minutes |

Segment 3: Channel Flow

| Length $=$ | 285 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 338.00 | ft |
| Bot Elev $=$ | 324.00 | ft |
| Height $=$ | 14 | ft |
| Slope $=$ | 0.0491 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Flow Area $=$ | 10.00 | sf (assume 5'w $\times$ 2'h channel) |
| Wetted Perimeter $=$ | 9.00 | If (assume 5'w $\times$ 2'h channel) |
| Channel Velocity $=$ | 7.87 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 0.60 | minutes |


| Time of Concentration $=$ | 20.39 | minutes |
| ---: | :---: | :--- |
| SCS Lag Time $=$ | 12.23 | minutes (SCS Lag = 0.6* Tc) |
| Time Increment $=$ | 3.55 | minutes ( $=0.29^{* S C S ~ L a g) ~}$ |


| Land Use | HSG | CN | Onsite | Percent Impervious (\%) | Impervious Area (ac) | Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open | A | 39 | Yes | 0 | 0.00 | 0.67 |
| Open | B | 61 | Yes | 0 | 0.00 | 1.24 |
| Open | D | 80 | Yes | 0 | 0.00 | 0.02 |
| Trail | A | 72 | Yes | 100 | 0.04 | 0.04 |
| Trail | B | 82 | Yes | 100 | 0.07 | 0.07 |
| Wooded | A | 30 | Yes | 0 | 0.00 | 0.71 |
| Wooded | B | 55 | Yes | 0 | 0.00 | 3.22 |
| Wooded | D | 77 | Yes | 0 | 0.00 | 0.26 |
| Total Area |  |  |  |  |  |  |
| Total Impervious Area |  |  |  |  |  |  |
| Onsite Area |  |  |  |  |  |  |
| Onsite Impervious Area |  |  |  |  |  |  |
| Percent Impervious |  |  |  |  |  |  |
| Composite Curve Number |  |  |  |  |  |  |

Time of Concentration Information
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

Segment 1: Overland Flow |  |  |  |
| ---: | :---: | :--- |
| Length | $=$ | 100 |
| Top Elev | $=$ | 398.00 |
| ft |  |  |
| ft |  |  |
| Bot Elev | $=$ | 393.00 |
| ft |  |  |
| Height $=$ | 5 | ft |
| Slope $=$ | 0.0500 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |
| $\mathrm{P}(2$-year/24-hour $)$ | $=$ | 3.46 |
| Segment Time | $=$ | 9.51 |

Segment 2: Concentrated Flow

| Length $=$ | 697 | ft |
| ---: | :---: | :--- |
| Top Elev $=$ | 393.00 | ft |
| Bot Elev $=$ | 365.00 | ft |
| Height $=$ | 28 | ft |
| Slope $=$ | 0.0402 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? $=$ | No |  |
| Velocity $=$ | 3.23 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 3.59 | minutes |


| Time of Concentration $=$ | 13.10 | minutes |
| ---: | :---: | :--- |
| SCS Lag Time $=$ | 7.86 | minutes (SCS Lag = 0.6* Tc) |
| Time Increment $=$ | 2.28 | minutes ( $=0.29^{*}$ SCS Lag) |

## REACH DATA

| Reach 1-POA 1 to POA 4 |  |  |
| ---: | :---: | :--- |
| Length $=$ | 2988 | ft |
| Top Elev $=$ | 344.00 | ft |
| Bot Elev $=$ | 302.00 | ft |
| Height $=$ | 42 | ft |
| Slope $=$ | 0.0141 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Flow Area $=$ | 10.00 | sf (assume 5'w $\times$ 2'h channel) |
| Wetted Perimeter $=$ | 9.00 | If (assume 5'w $\times$ 2'h channel) |
| Channel Velocity $=$ | 4.21 | ft/sec |
| Reach Travel Time $=$ | 11.83 | minutes |



# FlexTable: Catchment <br> Table (AWH20000Master.ppc) 

## Current Time: 0.00 min

| Label | Area <br> $\left(\mathrm{ft}^{2}\right)$ | SCS CN |  | Time of <br> Concentration <br> $(\mathrm{min})$ |
| :--- | ---: | ---: | ---: | ---: |
| SUB 1 | $10,318,492.80$ | 89.0 | 40.80 | PRE |
| SUB 2 | $2,609,244.00$ | 87.0 | 31.12 | PRE |
| SUB 5 | $18,508,208.41$ | 84.0 | 48.15 | PRE |
| SUB 4 | $4,729,309.20$ | 78.0 | 33.28 | PRE |
| SUB 7 | $682,585.20$ | 20.39 | PRE |  |
| SUB 8 | $271,269.00$ | 61.0 | 13.10 | PRE |
| SUB 6 | $372,873.60$ | 53.0 | 14.89 | PRE |
| SUB 3 | $353,271.60$ | 75.0 | 14.11 | PRE |

Subsection: Master Network Summary

## Catchments Summary

| Label | Scenario | Return Event (years) | Hydrograph Volume (ac-ft) | Time to Peak (min) | Peak Flow $\left(\mathrm{ft}^{3} / \mathrm{s}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| SUB 1 | Pre-Dev 10 yr | 10 | 74.868 | 745.00 | 545.3 |
| SUB 2 | Pre-Dev 10 yr | 10 | 17.950 | 738.00 | 153.0 |
| SUB 3 | Pre-Dev 10 yr | 10 | 1.740 | 728.00 | 22.3 |
| SUB 4 | Pre-Dev 10 yr | 10 | 24.755 | 741.00 | 204.9 |
| SUB 5 | Pre-Dev 10 yr | 10 | 116.278 | 751.00 | 787.2 |
| SUB 6 | Pre-Dev 10 yr | 10 | 1.774 | 728.00 | 22.0 |
| SUB 7 | Pre-Dev 10 yr | 10 | 1.820 | 734.00 | 17.2 |
| SUB 8 | Pre-Dev 10 yr | 10 | 0.458 | 729.00 | 4.3 |

## Node Summary

| Label | Scenario | Return Event (years) | Hydrograph Volume (ac-ft) | Time to Peak (min) | $\begin{aligned} & \text { Peak Flow } \\ & \left(\mathrm{ft}^{3} / \mathrm{s}\right) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| POA 1 | Pre-Dev 10 yr | 10 | 74.868 | 745.00 | 545.3 |
| POA 2 | Pre-Dev 10 yr | 10 | 17.950 | 738.00 | 153.0 |
| POA 3 | Pre-Dev 10 yr | 10 | 1.740 | 728.00 | 22.3 |
| POA 4 | Pre-Dev 10 yr | 10 | 99.357 | 752.00 | 704.1 |
| POA 5 | Pre-Dev 10 yr | 10 | 116.278 | 751.00 | 787.2 |
| POA 6 | Pre-Dev 10 yr | 10 | 216.993 | 758.00 | 1,494.8 |
| POA 7 | Pre-Dev 10 yr | 10 | 1.820 | 734.00 | 17.2 |
| POA 8 | Pre-Dev 10 yr | 10 | 0.458 | 729.00 | 4.3 |



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PHoN: 191.232 .239
CONTACT: BOB MSHLLER


REVISIONS
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plan information



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| :--- |


PRE

## POST-DEVELOPMENT HYDROLOGIC CALCULATIONS

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 2.20 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.87 | 0 | 0.00 |
| Pond | B | 100 | Yes | 0.26 | 99 | 0.26 |
| Road | B | 98 | Yes | 1.80 | 100 | 1.80 |
| Road | D | 98 | Yes | 0.57 | 100 | 0.57 |
| Sidewalk | B | 98 | Yes | 1.28 | 100 | 1.28 |
| Sidewalk | D | 98 | Yes | 0.52 | 100 | 0.52 |
| Singlefam40 | B | 84 | Yes | 1.85 | 62 | 1.15 |
| Singlefam40 | D | 91 | Yes | 0.16 | 62 | 0.10 |
| Singlefam60 | B | 80 | Yes | 0.53 | 50 | 0.27 |
| Sidewalk | B | 98 | No | 0.02 | 100 | 0.02 |
| Sidewalk | D | 98 | No | 0.07 | 100 | 0.07 |
| Mixed Use Neighborhood | B | 85 | No | 0.42 | 65 | 0.27 |
| Mixed Use Neighborhood | D | 92 | No | 1.81 | 65 | 1.18 |


| Onsite Area | 10.03 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 5.93 | ac |
| Offsite Area | 2.33 | ac |
| Offsite Impervious Area | 1.55 | ac |
| Total Area | 12.35 | ac |
| Total Impervious Area | 7.48 | ac |
| Percent Impervious | 61 | $\%$ |
| Composite Curve Number | 86 |  |

## time of concentration

Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes ( $=0.29^{*}$ SCS Lag) |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.05 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.04 | 0 | 0.00 |
| Open Space, Good Condition | C | 74 | Yes | 0.00 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.19 | 0 | 0.00 |
| Road | D | 98 | Yes | 0.46 | 100 | 0.46 |
| Sidewalk | A | 98 | Yes | 0.00 | 100 | 0.00 |
| Sidewalk | B | 98 | Yes | 0.00 | 100 | 0.00 |
| Sidewalk | C | 98 | Yes | 0.00 | 100 | 0.00 |
| Sidewalk | D | 98 | Yes | 0.00 | 100 | 0.00 |


| Onsite Area | 0.74 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 0.47 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 0.74 | ac |
| Total Impervious Area | 0.47 | ac |
| Percent Impervious | 63 | $\%$ |
| Composite Curve Number | 87 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes ( $=0.29 *$ SCS Lag) |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 1.65 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 1.34 | 0 | 0.00 |
| Pond | B | 100 | Yes | 0.13 | 99 | 0.13 |
| Pond | D | 100 | Yes | 0.08 | 100 | 0.08 |
| Road | B | 98 | Yes | 0.66 | 100 | 0.66 |
| Road | D | 98 | Yes | 0.90 | 100 | 0.90 |
| Sidewalk | B | 98 | Yes | 0.20 | 100 | 0.20 |
| Sidewalk | D | 98 | Yes | 0.24 | 100 | 0.24 |
| Townhome 22 | B | 95 | Yes | 1.56 | 91 | 1.42 |
| Townhome 22 | D | 96 | Yes | 1.53 | 91 | 1.39 |
| Mixed Use Neighborhood | B | 85 | No | 1.61 | 65 | 1.05 |
| Mixed Use Neighborhood | D | 92 | No | 0.32 | 65 | 0.21 |


| Onsite Area | 8.29 | ac |
| :--- | :--- | :--- |
| Onsite Impervious Area | 5.02 | ac |
| Offsite Area | 1.93 | ac |
| Offsite Impervious Area | 1.25 | ac |
| Total Area | 10.22 | ac |
| Total IIpervious Area | 6.28 | ac |
| Percent Impervious | 61 | $\%$ |
| Composite Curve Number | 87 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes $\left(\right.$ SCS Lag $\left.=0.6^{*} \mathrm{Tc}\right)$ |
| Time Increment $=$ | 0.87 | minutes $\left(=0.29^{* S C S ~ L a g) ~}\right.$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 1.45 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 1.18 | 0 | 0.00 |
| Pond | B | 100 | Yes | 0.03 | 99 | 0.03 |
| Pond | D | 100 | Yes | 0.32 | 100 | 0.32 |
| Road | A | 98 | Yes | 0.00 | 100 | 0.00 |
| Road | B | 98 | Yes | 1.68 | 100 | 1.68 |
| Road | D | 98 | Yes | 0.93 | 100 | 0.93 |
| Sidewalk | B | 98 | Yes | 0.51 | 100 | 0.51 |
| Sidewalk | D | 98 | Yes | 0.30 | 100 | 0.30 |
| Singlefam39 | B | 85 | Yes | 2.07 | 0 | 0.00 |
| Singlefam39 | D | 80 | Yes | 1.83 | 0 | 0.00 |
| Singlefam40 | B | 84 | Yes | 3.25 | 62 | 2.02 |
| Singlefam40 | D | 91 | Yes | 0.90 | 62 | 0.56 |
| Singlefam60 | B | 80 | Yes | 1.00 | 50 | 0.50 |
| Singlefam60 | D | 89 | Yes | 0.24 | 50 | 0.12 |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


|  |  |  |
| :--- | :---: | :--- |
| Onsite Area | 15.68 | ac |
| Onsite Impervious Area | 6.96 | ac |
| Offite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 15.68 | ac |
| Total Impervious Area | 6.96 | ac |
| Percent Impervious | 44 | $\%$ |
| Composite Curve Number | 85 |  |

time of concentration
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $\left.=0.6^{*} \mathrm{Tc}\right)$ |
| Time Increment $=$ | 0.87 | minutes $\left(=0.29^{*}\right.$ SCS Lag $)$ |

CURVE NUMBER CALCULATIONS

|  | Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Open Space, Good Condition | B | 61 | Yes | 1.07 | 0 | 0.00 |
|  | Open Space, Good Condition | D | 80 | Yes | 1.22 | 0 | 0.00 |
|  | Pond | B | 100 | Yes | 0.20 | 99 | 0.20 |
|  | Pond | D | 100 | Yes | 0.35 | 100 | 0.35 |
|  | Road | B | 98 | Yes | 1.37 | 100 | 1.37 |
|  | Road | D | 98 | Yes | 1.48 | 100 | 1.48 |
|  | Sidewalk | B | 98 | Yes | 0.37 | 100 | 0.37 |
|  | Sidewalk | D | 98 | Yes | 0.42 | 100 | 0.42 |
|  | Singlefam39 | B | 85 | Yes | 1.32 | 0 | 0.00 |
|  | Singlefam39 | D | 80 | Yes | 1.14 | 0 | 0.00 |
|  | Singlefam40 | B | 84 | Yes | 2.25 | 62 | 1.40 |
|  | Singlefam40 | D | 91 | Yes | 2.36 | 62 | 1.47 |
|  | Singlefam60 | B | 80 | Yes | 2.00 | 50 | 1.00 |
|  | Singlefam60 | D | 89 | Yes | 1.80 | 50 | 0.90 |
|  | Onsite Area | 17.35 | ac |  |  |  |  |
|  | Onsite Impervious Area | 8.95 | ac |  |  |  |  |
|  | Offsite Area | 0.00 | ac |  |  |  |  |
|  | Offsite Impervious Area | 0.00 | ac |  |  |  |  |
|  | Total Area | 17.35 | ac |  |  |  |  |
|  | Total Impervious Area | 8.95 | ac |  |  |  |  |
|  | Percent Impervious | 52 | \% |  |  |  |  |
|  | Composite Curve Number | 87 |  |  |  |  |  |
| time of Concentration |  |  |  |  |  |  |  |
| Time of concentration is assumed to be 5 minutes. |  |  |  |  |  |  |  |
|  |  | Time of Concentration = SCS Lag Time = Time Increment = |  | $\begin{aligned} & \hline 5.00 \\ & 3.00 \\ & 0.87 \\ & \hline \end{aligned}$ | minutes <br> minutes (SCS Lag $=0.6^{*} \mathrm{Tc}$ ) <br> minutes ( $=0.29 *$ SCS Lag) |  |  |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 1.62 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.08 | 0 | 0.00 |
| Pond | B | 100 | Yes | 0.45 | 99 | 0.44 |
| Road | B | 98 | Yes | 1.65 | 100 | 1.65 |
| Road | D | 98 | Yes | 0.07 | 100 | 0.07 |
| Sidewalk | B | 98 | Yes | 0.57 | 100 | 0.57 |
| Sidewalk | D | 98 | Yes | 0.03 | 100 | 0.03 |
| Singlefam60 | B | 80 | Yes | 8.33 | 50 | 4.17 |
| Singlefam60 | D | 89 | Yes | 0.39 | 50 | 0.20 |


| Onsite Area | $\mathbf{1 3 . 2 0}$ | ac |
| :--- | :---: | :---: |
| Onsite Impervious Area | 7.13 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 13.20 | ac |
| Total Impervious Area | $\mathbf{7 . 1 3}$ | ac |
| Percent Impervious | 54 | $\%$ |
| Composite Curve Number | 82 |  |
|  |  |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 1.24 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 4.15 | 0 | 0.00 |
| Pond | D | 100 | Yes | 0.37 | 100 | 0.37 |
| Road | B | 98 | Yes | 1.18 | 100 | 1.18 |
| Road | D | 98 | Yes | 1.73 | 100 | 1.73 |
| Sidewalk | B | 98 | Yes | 0.31 | 100 | 0.31 |
| Sidewalk | D | 98 | Yes | 0.64 | 100 | 0.64 |
| Townhome 22 | B | 95 | Yes | 0.23 | 91 | 0.21 |
| Townhome 22 | D | 96 | Yes | 1.30 | 91 | 1.18 |
| Singlefam39 | B | 85 | Yes | 2.08 | 0 | 0.00 |
| Singlefam39 | D | 80 | Yes | 0.86 | 0 | 0.00 |
| Singlefam40 | B | 84 | Yes | 1.97 | 62 | 1.22 |
| Singlefam40 | D | 91 | Yes | 1.97 | 62 | 1.22 |
| Singlefam60 | D | 89 | Yes | 0.60 | 50 | 0.30 |


| Onsite Area | 18.63 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 8.36 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 18.63 | ac |
| Total Impervious Area | 8.36 | ac |
| Percent Impervious | 45 | $\%$ |
| Composite Curve Number | 87 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes $\left(=0.29^{* S C S ~ L a g) ~}\right.$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 0.92 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 2.65 | 0 | 0.00 |
| Pond | D | 100 | Yes | 0.36 | 100 | 0.36 |
| Road | B | 98 | Yes | 0.71 | 100 | 0.71 |
| Road | D | 98 | Yes | 1.60 | 100 | 1.60 |
| Sidewalk | B | 98 | Yes | 0.26 | 100 | 0.26 |
| Sidewalk | D | 98 | Yes | 0.50 | 100 | 0.50 |
| Townhome 22 | B | 95 | Yes | 1.30 | 91 | 1.18 |
| Townhome 22 | D | 96 | Yes | 2.40 | 91 | 2.19 |
| Singlefam50 | B | 83 | Yes | 0.58 | 59 | 0.35 |
| Singlefam50 | D | 91 | Yes | 2.38 | 59 | 1.40 |


| Onsite Area | $\mathbf{1 3 . 6 6}$ | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 8.54 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 13.66 | ac |
| Total Impervious Area | 8.54 | ac |
| Percent Impervious | 63 | $\%$ |
| Composite Curve Number | 90 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6 *$ Tc) |
| Time Increment $=$ | 0.87 | minutes $(=0.29 * S C S ~ L a g)$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | C | 74 | Yes | 0.96 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.94 | 0 | 0.00 |
| Pond | D | 100 | Yes | 0.76 | 100 | 0.76 |
| Road | C | 98 | Yes | 1.31 | 100 | 1.31 |
| Road | D | 98 | Yes | 0.99 | 100 | 0.99 |
| Sidewalk | C | 98 | Yes | 0.41 | 100 | 0.41 |
| Sidewalk | D | 98 | Yes | 0.27 | 100 | 0.27 |
| Singlefam50 | C | 89 | Yes | 7.71 | 59 | 4.55 |
| Singlefam50 | D | 91 | Yes | 2.85 | 59 | 1.68 |
| Woods, Good Condition | D | 77 | Yes | 0.22 | 0 | 0.00 |


| Onsite Area | 16.40 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 9.96 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 16.40 | ac |
| Total Impervious Area | 9.96 | ac |
| Percent Impervious | 61 | $\%$ |
| Composite Curve Number | 90 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes ( $=0.29^{* S C S ~ L a g) ~}$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.15 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.04 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 1.31 | 0 | 0.00 |
| Pond | D | 100 | Yes | 0.18 | 10 | 0.18 |
| Road | A | 98 | Yes | 0.26 | 100 | 0.26 |
| Road | B | 98 | Yes | 0.08 | 100 | 0.08 |
| Road | D | 98 | Yes | 0.77 | 100 | 0.77 |
| Sidewalk | A | 98 | Yes | 0.09 | 100 | 0.09 |
| Sidewalk | B | 98 | Yes | 0.03 | 100 | 0.03 |
| Sidewalk | D | 98 | Yes | 0.27 | 100 | 0.27 |
| Singlefam50 | A | 74 | Yes | 0.91 | 59 | 0.54 |
| Singlefam50 | B | 83 | Yes | 0.69 | 59 | 0.41 |
| Singlefam50 | D | 91 | Yes | 3.58 | 59 | 2.11 |


| Onsite Area | 8.37 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 4.75 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 8.37 | ac |
| Total Impervious Area | 4.75 | ac |
| Percent Impervious | 57 | $\%$ |
| Composite Curve Number | 87 |  |

## TIME OF CONCENTRATION

Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes $(=0.29 * S C S$ Lag) |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.56 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.64 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 2.51 | 0 | 0.00 |
| Poond | D | 100 | Yes | 0.31 | 100 | 0.31 |
| Road | A | 98 | Yes | 0.20 | 100 | 0.20 |
| Road | B | 98 | Yes | 0.42 | 100 | 0.42 |
| Road | D | 98 | Yes | 1.68 | 100 | 1.68 |
| Sidewalk | A | 98 | Yes | 0.07 | 100 | 0.07 |
| Sidewalk | B | 98 | Yes | 0.13 | 100 | 0.13 |
| Sidewalk | D | 98 | Yes | 0.54 | 100 | 0.54 |
| Townhome 22 | B | 95 | Yes | 0.51 | 91 | 0.47 |
| Townhome 22 | D | 96 | Yes | 1.30 | 91 | 1.19 |
| Singlefam50 | A | 74 | Yes | 1.08 | 59 | 0.64 |
| Singlefam50 | B | 83 | Yes | 0.44 | 59 | 0.26 |
| Singlefam50 | D | 91 | Yes | 3.83 | 59 | 2.26 |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


|  |  |  |
| :--- | :---: | :---: |
| Onsite Area | 14.20 | ac |
| Onsite Impervious Area | 8.14 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 14.20 | ac |
| Total Impervious Area | 8.14 | ac |
| Percent Impervious | 57 | $\%$ |
| Composite Curve Number | 86 |  |

time of concentration
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $\left.=0.6^{*} \mathrm{Tc}\right)$ |
| Time Increment $=$ | 0.87 | minutes $\left(=0.29^{*}\right.$ SCS Lag $)$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.15 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.00 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 5.38 | 0 | 0.00 |
| Pond | D | 100 | Yes | 0.73 | 100 | 0.73 |
| Road | D | 98 | Yes | 1.94 | 100 | 1.94 |
| Sidewalk | D | 98 | Yes | 0.53 | 100 | 0.53 |
| Townhome 22 | A | 93 | Yes | 0.00 | 91 | 0.00 |
| Townhome 22 | D | 96 | Yes | 4.75 | 91 | 4.33 |
| Singlefam50 | A | 74 | Yes | 1.08 | 59 | 0.64 |
| Singlefam50 | B | 83 | Yes | 1.19 | 59 | 0.70 |
| Singlefam50 | D | 91 | Yes | 0.42 | 59 | 0.25 |


| Onsite Area | 16.19 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 9.12 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 16.19 | ac |
| Total Impervious Area | 9.12 | ac |
| Percent Impervious | 56 | $\%$ |
| Composite Curve Number | 88 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

| Time of Concentration $=$ | 5.00 | minutes |
| ---: | :--- | :--- | :--- |
| SCS Lag Time $=$ | 3.00 | minutes (SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 0.87 | minutes ( $=0.29^{* S C S ~ L a g) ~}$ |

## CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 1.24 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 2.96 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.64 | 0 | 0.00 |
| Pond | B | 100 | Yes | 0.37 | 99 | 0.37 |
| Pond | D | 100 | Yes | 0.42 | 100 | 0.42 |
| Road | A | 98 | Yes | 0.06 | 100 | 0.06 |
| Road | B | 98 | Yes | 1.24 | 100 | 1.24 |
| Road | D | 98 | Yes | 0.29 | 100 | 0.29 |
| Sidewalk | A | 98 | Yes | 0.02 | 100 | 0.02 |
| Sidewalk | B | 98 | Yes | 0.36 | 100 | 0.36 |
| Sidewalk | D | 98 | Yes | 0.11 | 100 | 0.11 |
| Singlefam50 | A | 74 | Yes | 0.60 | 59 | 0.36 |
| Singlefam50 | B | 83 | Yes | 5.41 | 59 | 3.19 |
| Singlefam50 | D | 91 | Yes | 2.10 | 59 | 1.24 |
| Woods, Good Condition | A | 30 | Yes | 0.09 | 0 | 0.00 |
| Wooss, Good Condition | B | 55 | Yes | 0.11 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 0.01 | 0 | 0.00 |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


| Onsite Area | 16.04 | ac |
| :--- | :---: | :--- |
| Onsite Impervious Area | 7.65 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Tota I Area | 16.04 | ac |
| Total Impervious Area | 7.65 | ac |
| Percent Impervious | 48 | $\%$ |
| Composite Curve Number | 78 |  |

TIME OF CONCENTRATION
Time of concentration is assumed to be 5 minutes.

CURVE NUMBER CALCULATIONS

| Land Use | HsG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 12.28 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 4.82 | 0 | 0.00 |
| Pond | в | 100 | Yes | 0.09 | 99 | 0.09 |
| Pond | D | 100 | Yes | 6.80 | 100 | 6.80 |
| Road | B | 98 | Yes | 0.18 | 100 | 0.18 |
| Sidewalk | B | 98 | Yes | 0.22 | 100 | 0.22 |
| Sidewalk | D | 98 | Yes | 0.00 | 100 | 0.00 |
| Singlefam60 | B | 80 | Yes | 0.30 | 50 | 0.15 |
| Singlefam60 | D | 89 | Yes | 0.05 | 50 | 0.03 |
| Woods, Good Condition | B | 55 | Yes | 3.23 | 0 | 0.00 |
| Woods, Good Condition | C | 70 | Yes | 0.03 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 5.78 | 0 | 0.00 |
| Mixed Use Neighborhood | в | 85 | No | 12.22 | 65 | 7.94 |
| Mixed Use Neighborhood | c | 90 | No | 1.79 | 65 | 1.16 |
| Mixed Use Neighborhood | D | 92 | No | 18.30 | 65 | 11.89 |
| Low Density Residential | B | 68 | No | 0.29 | 20 | 0.06 |
| Low Density Residential | D | 84 | No | 22.63 | 20 | 4.53 |
| Medium Density Residential | B | 70 | No | 1.14 | 25 | 0.29 |
| Medium Density Residential | D | 85 | No | 0.08 | 25 | 0.02 |
| Right-Of-Way | B | 89 | No | 4.08 | 90 | 3.67 |
| Right-Of-Way | D | 93 | No | 3.89 | 90 | 3.50 |
| School | B | 88 | No | 17.91 | 72 | 12.90 |
| School | D | 93 | No | 69.37 | 72 | 49.95 |
| Onsite Area | 33.79 | ac |  |  |  |  |
| Onsite Impervious Area | 7.47 | ac |  |  |  |  |
| Offsite Area | 151.71 | ac |  |  |  |  |
| Offsite Impervious Area | 95.91 | ac |  |  |  |  |
| Total Area | 185.50 | ac |  |  |  |  |
| Total Impervious Area | 103.38 | ac |  |  |  |  |
| Percent Impervious | 56 | \% |  |  |  |  |
| Composite Curve Number | 87 |  |  |  |  |  |

TIME OF CONCENTRATION
Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 100 | ft |
| Top Elev = | 426.00 | ft |
| Bot Elev = | 425.00 | ft |
| Height $=$ | 1 | ft |
| Slope $=$ | 0.0100 | $\mathrm{ft} / \mathrm{tt}$ |
| Manning's $\mathrm{n}=$ | 0.17 | cultivated soils, residue cover |
| $\mathrm{P}(2$-year/24-hour) $=$ | 3.46 | inches (Rolesville, NC) |


| Segment 2: Concentrated |  |  |
| ---: | :---: | :--- |
| Flow |  |  |
| Tength $=$ | 2645 | ft |
| Top Elev $=$ | 425.00 | ft |
| Bot Elev $=$ | 374.00 | ft |
| Height $=$ | 51 | ft |
| Slope $=$ | 0.0193 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? | No |  |
| Velocity $=$ | 2.24 | $\mathrm{ft} /$ sec |
| Segment Time $=$ | 19.68 | minutes |

## Segment 3: Open Water Flow $\begin{aligned} & \text { Length }=580 \mathrm{ft}\end{aligned}$ <br> Top Elev $=374.00 \mathrm{ft}$

$\begin{array}{llll}\text { Segment Time } & = & 372.00 & \mathrm{ft} \\ 0.00 & \text { minutes }\end{array}$

| Segment 4: Channel Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 2088 | ft |
| Top Elev = | 372.00 | $f t$ |
| Bot Elev = | 344.00 | ft |
| Height = | 28 | $f t$ |
| Slope $=$ | 0.0134 | ft/ft |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Flow Area $=$ | 15.00 | sf (assume $5^{\prime \prime} \times \times 3^{\prime} h$ channel) |
| Wetted Perimeter = | 11.00 | If (assume $5^{\prime} \mathrm{w} \times 3^{\prime} \mathrm{h}$ channel) |
| Channel Velocity = | 4.72 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 7.38 | minutes |


| Time of Concentration $=$ | 40.80 | minutes |
| ---: | :---: | :--- |
| SCS Lag Time $=$ | 24.48 | minutes $\left(\right.$ SCS Lag $=0.6^{*}$ Tc) |
| Time Increment $=$ | 7.10 | minutes $\left(=0.29^{* S C S ~ L a g) ~}\right.$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | B | 61 | Yes | 1.27 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.71 | 0 | 0.00 |
| Sidewalk | B | 98 | Yes | 0.14 | 100 | 0.14 |
| Sidewalk | D | 98 | Yes | 0.07 | 100 | 0.07 |
| Woods Good Condition | B | 55 | Yes | 0.43 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 1.65 | 0 | 0.00 |
| Mixed Use Neighborhood | B | 85 | No | 5.30 | 65 | 3.44 |
| Mixed Use Neighborhood | D | 92 | No | 13.37 | 65 | 8.69 |
| Low Density Residential | D | 84 | No | 17.96 | 20 | 3.59 |
| Risht-Of-Way | B | 89 | No | 2.31 | 90 | 2.08 |
| Right-Of-Way | D | 93 | No | 11.03 | 90 | 9.93 |
|  |  |  |  |  |  |  |


| Onsite Area |  |  |
| :--- | :---: | :---: |
| Ons | ac |  |
| Onsite Impervious Area | 0.22 | ac |
| Offsite Area | 4.97 | ac |
| Offsite Impervious Area | 27.74 | ac |
| Total Area | 54.25 | ac |
| Total Impervious Area | 27.95 | ac |
| Percent Impervious | 52 | $\%$ |
| Composite Curve Number | 87 |  |

TIME OF CONCENTRATION
Time of concentration is calculated using the SCS Segmental Approach (TR-55).


CURVE NUMBER CALCULATIONS


CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.00 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.00 | 0 | 0.00 |
| Open Space, Good Condition | c | 74 | Yes | 1.50 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 4.88 | 0 | 0.00 |
| Singlefam50 | c | 89 | Yes | 0.00 | 59 | 0.00 |
| Singlefam60 | D | 89 | Yes | 0.02 | 50 | 0.01 |
| Woods, Good Condition | c | 70 | Yes | 1.79 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 9.53 | 0 | 0.00 |
| Mixed Use Neighborhood | в | 85 | No | 1.78 | 65 | 1.16 |
| Mixed Use Neighborhood | c | 90 | No | 1.04 | 65 | 0.68 |
| Mixed Use Neighborhood | D | 92 | No | 8.00 | 65 | 5.20 |
| Onsite Area | 17.72 | ac |  |  |  |  |
| Onsite Impervious Area | 0.01 | ac |  |  |  |  |
| Offsite Area | 10.83 | ac |  |  |  |  |
| Offsite Impervious Area | 7.04 | ac |  |  |  |  |
| Total Area | 28.55 | ac |  |  |  |  |
| Total Impervious Area | 7.05 | ac |  |  |  |  |
| Percent Impervious | 25 | \% |  |  |  |  |
| Composite Curve Number | 82 |  |  |  |  |  |

TIME OF CONCENTRATION
Time of concentration is calculated using the SCS Segmental Approach (TR-55),

| Segment 1: Overland Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 100 | ft |
| Top Elev = | 394.00 | ft |
| Bot Elev = | 392.00 | ft |
| Height $=$ | 2 | ft |
| Slope $=$ | 0.0200 | $\mathrm{ft} / \mathrm{tt}$ |
| Manning's $\mathrm{n}=$ | 0.17 | cultivated soils, residue cov |
| $\mathrm{P}(2$-year/24-hour) $=$ | 3.46 | inches (Rolesville, NC) |


| Segment 2: Concentrated Flow |  |  |
| ---: | :---: | :--- |
| Length $=$ | 1223 | ft |
| Top Elev $=$ | 392.00 | ft |
| Bot Elev $=$ | 341.00 | ft |
| Height $=$ | 51 | ft |
| Slope $=$ | 0.0417 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? | No |  |
| Velocity $=$ | 3.29 | $\mathrm{ft} /$ sec |
| Segment Time $=$ | 6.19 | minutes |

Segment 3: Channel Flow
Length $=2660 \mathrm{ft}$
Top Elev $=341.00 \mathrm{ft}$
Bot Elev $=302.00 \mathrm{ft}$

$\begin{array}{rll}\text { Slope } & =0.0147 & \mathrm{ft} / \mathrm{ft} \\ \text { Hanning's } \mathrm{n} & =0.045 & \text { natural channel }\end{array}$
Flow Area $=\begin{array}{ll}15.00 & \text { sf (assume 5'w } \times 3^{\prime} \text { 'h channel) }\end{array}$
etted Perimeter = $\begin{aligned} & 11.00 \\ & \text { If (assume } 5^{\prime} \mathrm{w} \times 3^{\prime} \mathrm{h} \text { channel) }\end{aligned}$
Channel Velocity $=4.93 \mathrm{ft} / \mathrm{sec}$
segment Time $=8.99 \quad$ minutes

| Time of Concentration $=$ | 25.59 | minutes |
| ---: | :--- | :--- |
| SCS Lag Time $=$ | 15.36 | minutes (SCS Lag $=0.6^{*}$ TC) |
| Time Increment $=$ | 4.45 | minutes $\left(=0.29^{*}\right.$ SCS Lag $)$ |

CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.43 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 2.06 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 9.69 | 0 | 0.00 |
| Sidewalk | A | 98 | Yes | 0.02 | 100 | 0.02 |
| Sidewalk | B | 98 | Yes | 0.26 | 100 | 0.26 |
| Sidewalk | D | 98 | Yes | 0.86 | 100 | 0.86 |
| Townhome 22 | D | 96 | Yes | 0.03 | 91 | 0.03 |
| Woods, Good Condition | B | 55 | Yes | 0.03 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 6.25 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | No | 9.58 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | No | 9.24 | 0 | 0.00 |
| Mixed Use Neighborhood | B | 85 | No | 6.75 | 65 | 4.39 |
| Mixed Use Neighborhood | D | 92 | No | 15.53 | 65 | 10.09 |
| Low Density Residential | B | 68 | No | 0.92 | 20 | 0.18 |
| Low Density Residential | D | 84 | No | 1.85 | 20 | 0.37 |
| Medium Density Residential | A | 54 | No | 0.01 | 25 | 0.00 |
| Medium Density Residential | B | 70 | No | 47.92 | 25 | 11.98 |
| Medium Density Residential | D | 85 | No | 42.98 | 25 | 10.74 |
| High Density Residential | B | 72 | No | 70.82 | 30 | 21.24 |
| High Density Residential | D | 86 | No | 51.45 | 30 | 15.43 |
| Right-Of-Way | A | 83 | No | 0.76 | 90 | 0.68 |
| Right-Of-Way | B | 89 | No | 23.84 | 90 | 21.46 |
| Right-Of-Way | D | 93 | No | 31.95 | 90 | 28.75 |
| School | B | 88 | No | 0.47 | 72 | 0.34 |
| School | D | 93 | No | 7.11 | 72 | 5.12 |
| Preserved Open | A | 39 | No | 0.39 | 0 | 0.00 |
| Preserved Open | B | 61 | No | 4.48 | 0 | 0.00 |
| Preserved Open | D | 80 | No | 34.04 | 0 | 0.00 |
| Woods, Good Condition | B | 55 | No | 3.27 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | No | 23.30 | 0 | 0.00 |
| Business Park | A | 89 | No | 0.01 | 85 | 0.01 |
| Business Park | B | 92 | No | 0.02 | 85 | 0.02 |
| Business Park | D | 95 | No | 0.03 | 85 | 0.03 |
| Commercial | D | 95 | No | 1.01 | 85 | 0.86 |
| Town Center | B | 92 | No | 6.53 | 85 | 5.55 |
| Town Center | C | 94 | No | 0.00 | 85 | 0.00 |
| Town Center | D | 95 | No | 1.74 | 85 | 1.48 |


| Onsite Area | 19.63 | ac |
| :--- | :---: | :---: |
| Onsite Impervious Area | 1.16 | ac |
| Offsite Area | 395.98 | ac |
| Offsite Impervious Area | 138.72 | ac |
| Total Area | 415.61 | ac |
| Total Inpervious Area | 139.88 | ac |
| Percent Impervious | 34 | $\%$ |
| Composite Curve Number | 80 |  |

## time of concentration

Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 100 | $f t$ |
| Top Elev = | 440.00 | $f t$ |
| Bot Elev = | 438.00 | $f t$ |
| Height $=$ | 2 | $f t$ |
| Slope $=$ | 0.0200 | $\mathrm{ft} / \mathrm{tt}$ |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |
| $\mathrm{P}(2$-year $/ 24$-hour $)=$ | 3.46 | inches (Rolesville, NC) |
| Segment Time $=$ | 13.72 | minutes |

$\begin{array}{cc}\text { Segment 2: } \text { Concentrated Flow } \\ \text { Length }= & 2989 \\ \text { Top Elev }= & 438.00\end{array}$
$\begin{array}{lll}\text { Length }= & 2989 & \mathrm{ft} \\ \text { Top Elev }= & 438.00 & \mathrm{ft}\end{array}$
$\begin{array}{ll}\text { Bot Elev }= & 372.0\end{array}$
Slope $=0.022$
Paved? = No ft/t

| Velocity $=$ | 2.40 | $\mathrm{ft} / \mathrm{sec}$ |
| ---: | :---: | :---: |
| Segment Time $=$ | 20.78 | minute |

Segment 3: Open Water Flow
Length $=655 \mathrm{ft}$ Top Elev $=372.00 \mathrm{ft}$
Bot Elev $=372.00 \quad \mathrm{ft}$
Segment Time $=0.00$ minutes

| Segment 4: Concentrated Flow |  |  |
| ---: | :---: | :--- |
| Length $=$ | 2379 | ft |
| Top Elev $=$ | 372.00 | ft |
| Bot Elev $=$ | 320.00 | ft |
| Height $=$ | 52 | ft |
| Stope $=$ | 0.0219 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? | No |  |
| Velocity $=$ | 2.39 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 16.62 | minutes |

Segment 5: Channel Flow
Length $=3029 \mathrm{ft}$
Top Elev $=320.00 \mathrm{ft}$
Bot Elev $=39200$
$\begin{array}{ccc}\text { Bot Elev }= & 292.00 & \mathrm{ft} \\ \text { Height }= & 28 & \mathrm{ft}\end{array}$
$\begin{array}{ll}\text { Height }= & 28 \\ \text { Slope }= & 0.0092 \\ \mathrm{ft} / \mathrm{ft}\end{array}$
Manning's $\mathrm{n}=0.045$ natural channel
Flow Area $=32.00 \quad$ sf (assume $8^{\prime} w \times 4^{\prime} h$ channel)
Wetted Perimeter $=16.00$ If (assume $8^{\prime} \mathbf{w} \times 4^{\prime} \mathrm{h}$ channel)
Channel Velocity $=\begin{array}{ll}5.05 & \mathrm{ft} / \mathrm{sec}\end{array}$
Segment Time $=9.99 \quad$ minutes


CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | D | 80 | Yes | 1.69 | 0 | 0.00 |
| Road | D | 98 | Yes | 0.01 | 100 | 0.01 |
| Sidewalk | D | 98 | Yes | 0.14 | 100 | 0.14 |
| Townhome 22 | D | 96 | Yes | 0.04 | 91 | 0.04 |
| Woods, Good Condition | D | 77 | Yes | 0.23 | 0 | 0.00 |


| Onsite Area | 2.11 | ac |
| :--- | :---: | :---: |
| Onsite Impervious Area | 0.19 | ac |
| Offite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 2.11 | ac |
| Total Impervious Area | $\mathbf{0 . 1 9}$ | ac |
| Percent Impervious | 9 | \% |
| Composite Curve Number | $\mathbf{8 1}$ |  |

## time of concentration

Time of concentration is assumed to be 5 minutes.
$\begin{array}{rlll}\text { Time of Concentration }= & 5.00 & \text { minutes } \\ \text { SCS Lag Time }= & 3.00 & \text { minutes (SCS Lag }=0.6^{*} \text { Tc) }\end{array}$


CURVE NUMBER CALCULATIONS

| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.01 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.10 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.63 | 0 | 0.00 |
| Woods, Good Condition | A | 30 | Yes | 0.05 | 0 | 0.00 |
| Woods Good Condition | B | 55 | Yes | 0.17 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 1.16 | 0 | 0.00 |


| Onsite Area | $\mathbf{2 . 1 2}$ | ac |
| :--- | :---: | :---: |
| Onsite Impervious Area | 0.00 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 2.12 | ac |
| Total Impervious Area | 0.00 | ac |
| Percent Impervious | 0 | $\%$ |
| Composite Curve Number | 74 |  |

## TIME OF CONCENTRATION

Time of concentration is calculated using the SCS Segmental Approach (TR-55).

| Segment 1: Overland Flow |  |  | Segment 2: Concentrated Flow |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 100 | ft | Length $=$ | 115 | ft |
| Top Elev = | 352.00 | ft | Top Elev = | 345.00 | ft |
| Bot Elev = | 345.00 | ft | Bot Elev = | 324.00 | $f t$ |
| Height $=$ | 7 | $f$ ft | Height $=$ | 21 | ft |
| Slope $=$ | 070 | ft/ft | Slope $=$ | . 1826 | ft/ft |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses | Paved ? $=$ | No |  |
| $\mathrm{P}(2$-year/24-hour) $=$ | 3.46 | inches (Rolesville, NC) | Velocity $=$ | 6.89 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 8.31 | minutes | Segment Time $=$ | 0.28 | minutes |


| Time of Concentration $=$ | 8.59 | minutes |
| ---: | ---: | ---: | :--- |
| SCS Lag Time $=$ | 5.16 | minutes (SCS Lag $=0.6^{*}$ TC) |
| Time Increment $=$ | 1.50 | minutes $\left(=0.29^{*}\right.$ SCS Lag) |


| Land Use | HSG | CN | Onsite | Area (ac) | Percent Impervious (\%) | Impervious Area (ac) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Open Space, Good Condition | A | 39 | Yes | 0.25 | 0 | 0.00 |
| Open Space, Good Condition | B | 61 | Yes | 0.94 | 0 | 0.00 |
| Open Space, Good Condition | D | 80 | Yes | 0.10 | 0 | 0.00 |
| Sidewalk | A | 98 | Yes | 0.03 | 100 | 0.03 |
| Sidewalk | B | 98 | Yes | 0.13 | 100 | 0.13 |
| Sidewalk | D | 98 | Yes | 0.02 | 100 | 0.02 |
| Woods, Good Condition | A | 30 | Yes | 0.02 | 0 | 0.00 |
| Woods, Good Condition | B | 55 | Yes | 0.56 | 0 | 0.00 |
| Woods, Good Condition | D | 77 | Yes | 0.08 | 0 | 0.00 |


| Onsite Area | 2.13 | ac |
| :--- | :---: | :---: |
| Onsite Impervious Area | 0.18 | ac |
| Offsite Area | 0.00 | ac |
| Offsite Impervious Area | 0.00 | ac |
| Total Area | 2.13 | ac |
| Total Impervious Area | 0.18 | ac |
| Percent Impervious | 9 | $\%$ |
| Composite Curve Number | 61 |  |

TIME OF CONCENTRATION
Time of concentration is calculated using the SCS Seqmental Approach (TR-55).

| Segment 1: Overland Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 37 | $f t$ |
| Top Elev = | 398.00 | ft |
| Bot Elev = | 396.00 | ft |
| Height $=$ | 2 | ft |
| Slope $=$ | 0.0541 | ft/f |
| Manning's $\mathrm{n}=$ | 0.24 | dense grasses |
| $\mathrm{P}(2$-year/ 24 -hour $)=$ | 3.46 | inches (Rolesville, NC) |
| Segment Time $=$ | 4.16 | minutes |


| Segment 2: Concentrated Flow |  |  |
| ---: | :---: | :--- |
| Length $=$ | 575 | ft |
| Top Elev $=$ | 396.00 | ft |
| Bot Elev $=$ | 365.00 | ft |
| Height $=$ | 31 | ft |
| Slope $=$ | 0.0539 | $\mathrm{ft} / \mathrm{tt}$ |
| Paved ? | No |  |
| Velocity $=$ | 3.75 | $\mathrm{ft} /$ sec |
| Segment Time $=$ | 2.56 | minutes |

## REACH DATA <br> Reach \#1-POA 1 to POA 4

| Segment 1: Concentrated Flow |  |  |
| ---: | :---: | :--- |
| Length $=$ | 2988 | ft |
| Top Elev $=$ | 344.00 | ft |
| Bot Elev $=$ | 302.00 | ft |
| Height $=$ | 42 | ft |
| Slope $=$ | 0.0141 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Flow Area $=$ | 10.00 | sf (assume $5^{\prime} \mathrm{w} \times 2^{\prime} \mathrm{h}$ channel) |
| Wetted Perimeter $=$ | 9.00 | If (assume $5^{\prime} \mathrm{w} \times 2^{\prime} \mathrm{h}$ channel) |
| Channel Velocity $=$ | 4.21 | $\mathrm{ft} / \mathrm{sec}$ |
| Reach Travel Time $=$ | 11.83 | minutes |

Reach \#2 - SCM A TO POA 1


Reach \#4 - SCM F TO POA 4

| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel Flow |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 77 | ft |  | Length $=$ | 2292 | ft |
| Top Elev = | 339.00 | ft |  | Top Elev = | 336.00 | ft |
| Bot Elev = | 336.00 | ft |  | Bot Elev = | 302.00 | ft |
| Height = | 3 | ft |  | Height = | 34 | ft |
| Slope $=$ | 0.0390 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0148 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? = | No |  |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Velocity = | 3.18 | $\mathrm{ft} / \mathrm{sec}$ |  | Flow Area = | 10.00 | sf (assume 5'w $\times 2$ 'h channel) |
|  |  |  |  | Wetted Perimeter $=$ | 9.00 | If (assume $5^{\prime} \mathrm{w} \times 2 \mathrm{~h}$ channel) |
|  |  |  |  | Channel Velocity = | 4.33 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 0.40 | minutes |  | Reach Travel Time $=$ | 8.83 | minutes |
| Total Travel Time $=9.23$ minutes |  |  |  |  |  |  |
| Reach \#5 - SCM B TO POA 1 |  |  |  |  |  |  |
| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel Flow |  |  |
| Length $=$ | 161 | ft |  | Length $=$ | 1850 | ft |
| Top Elev = | 375.00 | ft |  | Top Elev = | 363.00 | ft |
| Bot Elev = | 363.00 | ft |  | Bot Elev = | 344.00 | ft |
| Height $=$ | 12 | ft |  | Height = | 19 | ft |
| Slope $=$ | 0.0745 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0103 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? = | No |  |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Velocity = | 4.40 | $\mathrm{ft} / \mathrm{sec}$ |  | Flow Area $=$ | 12.00 | sf (assume 6'w $\times 2$ 'h channel) |
|  |  |  |  | Wetted Perimeter = | 10.00 | If (assume $6^{\prime} \mathrm{w} \times 2 \mathrm{~h}$ channel) |
|  |  |  |  | Channel Velocity = | 3.79 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 0.61 | minutes |  | Reach Travel Time = | 8.14 | minutes |
| Total Travel Time $=8.75$ minutes |  |  |  |  |  |  |

## Reach \#6-SCM G TO POA 4

| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel Flow |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 75 | ft |  | Length $=$ | 1566 | ft |
| Top Elev = | 338.00 | ft |  | Top Elev = | 337.00 | ft |
| Bot Elev = | 337.00 | ft |  | Bot Elev = | 303.00 | ft |
| Height = | 1 | ft |  | Height = | 34 | ft <br> $\mathrm{ft} / \mathrm{ft}$ |
| Slope $=$ | 0.0133 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0217 |  |
| Paved ? = | No | $\mathrm{ft} / \mathrm{sec}$ |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel sf (assume $6^{\prime} w \times 2$ 'h channel) |
| Velocity $=$ | 1.86 |  |  | Flow Area = | 12.00 |  |
| Segment Time $=$ |  |  |  | Wetted Perimeter = | 10.00 | If (assume $6^{\prime} w \times 2$ 'h channel) $\mathrm{ft} / \mathrm{sec}$ |
|  |  |  |  | Channel Velocity = | 5.51 |  |
|  | 0.67 | minutes |  | Reach Travel Time = | 4.74 | minutes |
| Total Travel Time $=5.41$ minutes |  |  |  |  |  |  |
| Reach \#7-SCM H TO POA 4 |  |  |  |  |  |  |
| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel Flow |  |  |
| Length = | 482 | ft |  | Length $=$ | 1567 | ft |
| Top Elev = | 374.00 | ft |  | Top Elev = | 361.00 | ft |
| Bot Elev = | 361.00 | ft |  | Bot Elev = | 344.00 | ft |
| Height $=$ | 13 | ft |  | Height $=$ | 17 | ft |
| Slope $=$ | 0.0270 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0108 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? = | No |  |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Velocity $=$ | 2.65 | $\mathrm{ft} / \mathrm{sec}$ |  | Flow Area = | 12.00 | sf (assume $6^{\prime} \mathrm{w} \times 2$ 'h channel) |
|  |  |  |  | Wetted Perimeter $=$ | 10.00 | If (assume $6^{\prime} w \times 2$ 'h channel) |
|  |  |  |  | Channel Velocity = | 3.89 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 3.03 | minutes |  | Reach Travel Time = | 6.71 | minutes |
|  |  |  | 9.74 minutes |  |  |  |

Reach \#8-SCM I TO POA 4

| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 482 | ft |  | Length $=$ | 805 | ft |
| Top Elev = | 308.00 | ft |  | Top Elev = | 296.00 | ft |
| Bot Elev = | 296.00 | ft |  | Bot Elev = | 292.00 | ft |
| Height $=$ | 12 | ft |  | Height = | 4 | ft |
| Slope = | 0.0249 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0050 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? = | No |  |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Velocity = | 2.55 | $\mathrm{ft} / \mathrm{sec}$ |  | Flow Area = | 12.00 | sf (assume $6^{\prime} w \times 2$ 'h channel) |
|  |  |  |  | Wetted Perimeter $=$ | 10.00 | If (assume $6^{\prime} \mathrm{w} \times 2^{\prime} \mathrm{h}$ channel) |
|  |  |  |  | Channel Velocity = | 2.64 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 3.16 | minutes |  | Reach Travel Time = | 5.09 | minutes |
|  |  | Total Travel Time $=$ | 8.25 minutes |  |  |  |
| Reach \#9 - CULVERT TO POA 5 |  |  |  |  |  |  |
| Segment 1: Channel Flow |  |  |  |  |  |  |
| Length $=$ | 650 | ft |  |  |  |  |
| Top Elev = | 296.00 | ft |  |  |  |  |
| Bot Elev = | 291.00 | ft |  |  |  |  |
| Height = | 5 | ft |  |  |  |  |
| Slope $=$ | 0.0077 | $\mathrm{ft} / \mathrm{ft}$ |  |  |  |  |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |  |  |  |  |
| Flow Area = | 12.00 | sf (assume $6^{\prime} w \times 2$ 'h channel) |  |  |  |  |
| Wetted Perimeter $=$ | 10.00 | If (assume 6 'w $\times 2$ 'h channel) |  |  |  |  |
| Channel Velocity = | 3.28 | $\mathrm{ft} / \mathrm{sec}$ |  |  |  |  |
| Reach Travel Time $=$ | 3.30 | minutes |  |  |  |  |
|  |  | Total Travel Time $=$ | 3.30 minutes |  |  |  |
| Reach \#10-POA 5 TO POA 6 |  |  |  |  |  |  |
| Segment 1: Channel Flow |  |  |  |  |  |  |
| Length $=$ | 822 | ft |  |  |  |  |
| Top Elev = | 291.00 | ft |  |  |  |  |
| Bot Elev = | 288.00 | ft |  |  |  |  |
| Height = | 3 | ft |  |  |  |  |
| Slope $=$ | 0.0036 | $\mathrm{ft} / \mathrm{ft}$ |  |  |  |  |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |  |  |  |  |
| Flow Area = | 12.00 | sf (assume $6^{\prime} w \times 2$ 'h channel) |  |  |  |  |
| Wetted Perimeter = | 10.00 | If (assume 6'w x 2'h channel) |  |  |  |  |
| Channel Velocity = | 2.26 | $\mathrm{ft} / \mathrm{sec}$ |  |  |  |  |
| Reach Travel Time $=$ | 6.07 | minutes |  |  |  |  |
|  |  | Total Travel Time $=$ | 6.07 minutes |  |  |  |

Reach \#11 - POA 4 TO POA 6

| Segment 1: Channel Flow |  |  |
| :---: | :---: | :---: |
| Length $=$ | 883 | ft |
| Top Elev = | 292.00 | ft |
| Bot Elev = | 288.00 | ft |
| Height $=$ | 4 | ft |
| Slope $=$ | 0.0045 | $\mathrm{ft} / \mathrm{ft}$ |
| Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Flow Area = | 12.00 | sf (assume 6'w $\times 2$ 'h channel) |
| Wetted Perimeter = | 10.00 | If (assume $6^{\prime} \mathbf{w} \times 2$ 'h channel) |
| Channel Velocity = | 2.52 | $\mathrm{ft} / \mathrm{sec}$ |
| Reach Travel Time $=$ | 5.85 | minutes |

Total Travel Time $=5.85$ minutes

Reach \#12-SCM L TO POA 7

| Segment 1: Concentrated Flow |  |  |  | Segment 2: Channel Flow |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Length $=$ | 85 | ft |  | Length $=$ | 68 | ft |
| Top Elev = | 338.00 | ft |  | Top Elev = | 327.00 | ft |
| Bot Elev = | 327.00 | ft |  | Bot Elev = | 324.00 | ft |
| Height = | 11 | ft |  | Height = | 3 | ft |
| Slope = | 0.1294 | $\mathrm{ft} / \mathrm{ft}$ |  | Slope $=$ | 0.0441 | $\mathrm{ft} / \mathrm{ft}$ |
| Paved ? = | No |  |  | Manning's $\mathrm{n}=$ | 0.045 | natural channel |
| Velocity = | 5.80 | $\mathrm{ft} / \mathrm{sec}$ |  | Flow Area $=$ | 12.00 | sf (assume 6'w $\times 2$ 'h channel) |
|  |  |  |  | Wetted Perimeter $=$ | 10.00 | If (assume 6 ' $\mathrm{w} \times 2$ 'h channel) |
|  |  |  |  | Channel Velocity = | 7.85 | $\mathrm{ft} / \mathrm{sec}$ |
| Segment Time $=$ | 0.24 | minutes |  | Reach Travel Time = | 0.14 | minutes |
| Total Travel Time $=0.39$ minutes |  |  |  |  |  |  |

## Scenario: Post-Dev 1 yr



# FlexTable: Catchment <br> Table (AWH20000Master.ppc) 

## Current Time: $\mathbf{0 . 0 0} \mathbf{~ m i n}$

| Label | Area <br> $\left(\mathrm{ft}^{2}\right)$ | SCS CN <br> Concentration <br> $(\mathrm{min})$ |  |  |
| :--- | ---: | ---: | ---: | :--- |
| Sub 5 Bypass | $18,106,649.00$ | 80.0 | 61.11 | POST |
| Sub 2 | $2,363,457.00$ | 87.0 | 31.12 | POST |
| Sub 1 Bypass | $8,081,866.00$ | 87.0 | 40.80 | POST |
| Sub 1 to SCM B | $445,000.00$ | 87.0 | 5.00 | POST |
| Sub 1 to SCM A | $538,121.00$ | 86.0 | 5.00 | POST |
| Sub 1 to SCM D | $574,908.00$ | 82.0 | 5.00 | POST |
| Sub 1 to SCM C | $683,139.00$ | 86.0 | 5.00 | POST |
| Sub 1 to SCM E | $755,874.00$ | 87.0 | 5.00 | POST |
| Sub 4 to SCM F | $811,326.00$ | 87.0 | 5.00 | POST |
| Sub 4 to SCM H | $714,539.00$ | 90.0 | 5.00 | POST |
| Sub 4 Bypass | $1,243,502.00$ | 82.0 | 25.59 | POST |
| Sub 4 to SCM G | $595,088.00$ | 90.0 | 5.00 | POST |
| Sub 5 to SCM J | $618,668.00$ | 86.0 | 5.00 | POST |
| Sub 4 to SCM I | $364,616.00$ | 87.0 | 5.00 | POST |
| Sub 6 to SCM K | $705,046.00$ | 88.0 | 5.00 | POST |
| Sub 7 to SCM L | $698,883.00$ | 78.0 | 5.00 | POST |
| Sub 7 Bypass | $92,480.00$ | 74.0 | 8.59 | POST |
| Sub 8 | $92,863.00$ | 61.0 | 6.72 | POST |
| Sub 1 to SCM P | $32,425.00$ | 87.0 | 5.00 | POST |
| Sub 6 Bypass | $92,122.00$ | 81.0 | 5.00 | POST |

## Catchments Summary

| Label | Scenario | Return Event (years) | Hydrograph Volume (ac-ft) | Time to Peak (min) | Peak Flow $\left(\mathrm{ft}^{3} / \mathrm{s}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Sub 1 Bypass | Post-Dev 10 yr | 10 | 55.470 | 745.00 | 407.2 |
| Sub 1 to SCM A | Post-Dev 10 yr | 10 | 3.622 | 721.00 | 65.5 |
| Sub 1 to SCM B | Post-Dev 10 yr | 10 | 3.081 | 721.00 | 55.5 |
| Sub 1 to SCM C | Post-Dev 10 yr | 10 | 4.598 | 721.00 | 83.2 |
| Sub 1 to SCM D | Post-Dev 10 yr | 10 | 3.441 | 721.00 | 62.8 |
| Sub 1 to SCM E | Post-Dev 10 yr | 10 | 5.234 | 721.00 | 94.4 |
| Sub 1 to SCM P | Post-Dev 10 yr | 10 | 0.225 | 721.00 | 4.0 |
| Sub 2 | Post-Dev 10 yr | 10 | 16.259 | 738.00 | 138.6 |
| Sub 3 | Post-Dev 10 yr | 10 | 1.195 | 726.00 | 16.6 |
| Sub 4 Bypass | Post-Dev 10 yr | 10 | 7.401 | 734.00 | 70.7 |
| Sub 4 to SCM F | Post-Dev 10 yr | 10 | 5.618 | 721.00 | 101.3 |
| Sub 4 to SCM G | Post-Dev 10 yr | 10 | 4.474 | 721.00 | 79.4 |
| Sub 4 to SCM H | Post-Dev 10 yr | 10 | 5.372 | 721.00 | 95.4 |
| Sub 4 to SCM I | Post-Dev 10 yr | 10 | 2.525 | 721.00 | 45.5 |
| Sub 5 Bypass | Post-Dev 10 yr | 10 | 100.289 | 758.00 | 596.8 |
| Sub 5 to SCM J | Post-Dev 10 yr | 10 | 4.164 | 721.00 | 75.4 |
| Sub 6 Bypass | Post-Dev 10 yr | 10 | 0.535 | 721.00 | 9.8 |
| Sub 6 to SCM K | Post-Dev 10 yr | 10 | 5.020 | 721.00 | 90.1 |
| Sub 7 Bypass | Post-Dev 10 yr | 10 | 0.426 | 724.00 | 6.5 |
| Sub 7 to SCM L | Post-Dev 10 yr | 10 | 3.688 | 721.00 | 67.2 |
| Sub 8 | Post-Dev 10 yr | 10 | 0.249 | 723.00 | 3.7 |

## Node Summary

| Label | Scenario | Return Event (years) | Hydrograph Volume (ac-ft) | Time to Peak (min) | Peak Flow $\left(\mathrm{ft}^{3} / \mathrm{s}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Post-Dev 10 yr | 10 | 2.205 | 728.00 | 28.8 |
|  | Post-Dev 10 yr | 10 | 3.010 | 730.00 | 24.2 |
|  | Post-Dev 10 yr | 10 | 3.738 | 726.00 | 51.5 |
|  | Post-Dev 10 yr | 10 | 4.686 | 725.00 | 73.1 |
|  | Post-Dev 10 yr | 10 | 3.287 | 729.00 | 37.1 |
|  | Post-Dev 10 yr | 10 | 1.478 | 730.00 | 16.5 |
|  | Post-Dev 10 yr | 10 | 2.665 | 734.00 | 19.7 |
| CULVERT | Post-Dev 10 yr | 10 | 103.144 | 758.00 | 611.7 |
| POA 1 | Post-Dev 10 yr | 10 | 70.939 | 743.00 | 514.0 |
| POA 2 | Post-Dev 10 yr | 10 | 16.259 | 738.00 | 138.6 |
| POA 3 | Post-Dev 10 yr | 10 | 1.195 | 726.00 | 16.6 |
| POA 4 | Post-Dev 10 yr | 10 | 90.143 | 753.00 | 636.3 |
| POA 5 | Post-Dev 10 yr | 10 | 103.033 | 761.00 | 611.7 |
| POA 6 | Post-Dev 10 yr | 10 | 195.937 | 763.00 | 1,231.6 |
| POA 7 | Post-Dev 10 yr | 10 | 2.441 | 753.00 | 8.6 |
| POA 8 | Post-Dev 10 yr | 10 | 0.249 | 723.00 | 3.7 |

## Pond Summary

| Label | Scenario | Return <br> Event <br> (years) | Hydrograph <br> Volume <br> $(\mathrm{ac}-\mathrm{ft})$ | Time to Peak <br> $(\mathrm{min})$ | Peak Flow <br> $\left(\mathrm{ft}^{3} / \mathrm{s}\right)$ | Maximum <br> Water <br> Surface <br> Elevation <br> (ft) | Maximum <br> Pond Storage <br> (ac-ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | ---: | ---: | ---: |

## Pond Summary

| Label | Scenario | Return Event (years) | Hydrograph Volume (ac-ft) | Time to Peak (min) | Peak Flow $\left(\mathrm{ft}^{3} / \mathrm{s}\right)$ | Maximum Water Surface Elevation (ft) | Maximum Pond Storage (ac-ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { SCM A } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 3.010 | 730.00 | 24.2 | 384.19 | 1.488 |
| SCM B (IN) | Post-Dev 10 yr | 10 | 3.081 | 721.00 | 55.5 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM B } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 2.205 | 728.00 | 28.8 | 377.88 | 1.176 |
| SCM C (IN) | Post-Dev 10 yr | 10 | 4.598 | 721.00 | 83.2 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM C } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 3.738 | 726.00 | 51.5 | 379.49 | 1.405 |
| SCM D (IN) | Post-Dev 10 yr | 10 | 3.441 | 721.00 | 62.8 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM D } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 2.382 | 783.00 | 3.9 | 357.55 | 2.006 |
| SCM E (IN) | Post-Dev 10 yr | 10 | 5.234 | 721.00 | 94.4 | (N/A) | (N/A) |
| SCM E (OUT) | Post-Dev 10 yr | 10 | 4.166 | 731.00 | 29.2 | 358.69 | 2.112 |
| SCM F (IN) | Post-Dev 10 yr | 10 | 5.618 | 721.00 | 101.3 | (N/A) | (N/A) |
| SCM F (OUT) | Post-Dev 10 yr | 10 | 4.686 | 725.00 | 73.1 | 347.79 | 1.625 |
| SCM G (IN) | Post-Dev 10 yr | 10 | 4.474 | 721.00 | 79.4 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM G } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 2.665 | 734.00 | 19.7 | 344.41 | 2.160 |
| SCM H (IN) | Post-Dev 10 yr | 10 | 5.372 | 721.00 | 95.4 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM H } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 3.287 | 729.00 | 37.1 | 334.37 | 2.463 |
| SCM I (IN) | Post-Dev 10 yr | 10 | 2.525 | 721.00 | 45.5 | (N/A) | (N/A) |
| SCM I (OUT) | Post-Dev 10 yr | 10 | 1.478 | 730.00 | 16.5 | 322.28 | 1.134 |
| SCM J (IN) | Post-Dev 10 yr | 10 | 4.164 | 721.00 | 75.4 | (N/A) | (N/A) |
| SCM J (OUT) | Post-Dev 10 yr | 10 | 2.855 | 732.00 | 21.0 | 314.19 | 1.851 |
| SCM K (IN) | Post-Dev 10 yr | 10 | 5.020 | 721.00 | 90.1 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM K } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 2.635 | 761.00 | 7.2 | 303.40 | 3.036 |
| SCM L (IN) | Post-Dev 10 yr | 10 | 3.688 | 721.00 | 67.2 | (N/A) | (N/A) |
| SCM L (OUT) | Post-Dev 10 yr | 10 | 2.016 | 757.00 | 6.8 | 343.71 | 2.036 |
| SCM P (1) <br> (IN) | Post-Dev 10 yr | 10 | 0.090 | 725.00 | 3.1 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM P (1) } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 0.047 | 734.00 | 1.3 | 400.59 | 0.037 |
| $\begin{aligned} & \text { SCM P (2) } \\ & \text { (IN) } \end{aligned}$ | Post-Dev 10 yr | 10 | 0.185 | 722.00 | 3.9 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM P (2) } \\ & \text { (OUT) } \end{aligned}$ | Post-Dev 10 yr | 10 | 0.090 | 725.00 | 3.1 | 402.06 | 0.046 |
| SCM P (3) <br> (IN) | Post-Dev 10 yr | 10 | 0.225 | 721.00 | 4.0 | (N/A) | (N/A) |
| $\begin{aligned} & \text { SCM P (3) } \\ & \text { (OUT) } \\ & \hline \end{aligned}$ | Post-Dev 10 yr | 10 | 0.185 | 722.00 | 3.9 | 403.06 | 0.010 |




[^0]:    ${ }^{1}$ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
    Numbers in parenthesis are PF estimates at lower and upper bounds of the $90 \%$ confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is $5 \%$. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

